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COLD LAKE - BEAVER RIVER WATER MANAGEMENT STUDY

VOLUME 2 WATER SUPPLY

Appendix A . . . Surface Water Resources
Appendix B . . . Ground Water Resources
Appendix C . . . Ground Water Modelling



Water Resources

Alberta

**ENVIRONMENT
Planning Division**

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COLD LAKE — BEAVER RIVER
WATER MANAGEMENT STUDY

VOLUME 2
WATER SUPPLY

PREPARED BY
PLANNING DIVISION
ALBERTA ENVIRONMENT
December 1983

Submitted to: Hon. F. Bradley
Minister of Environment

APPENDIX A

SURFACE WATER RESOURCES

A Report Prepared By
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Our File Number AE-COLD(2)/673

June 18, 1982

Mr. Ron Hicks, P.Eng.
Alberta Environment, Planning
9th Floor, Oxbridge Place
9820 - 106 Street
EDMONTON, AB
T5K 2J6

Dear Sir:

Re: Second Draft - Beaver River Cold Lake Report

We are pleased to submit our final report, updated to include the latest model run of April 27, 1982. You will note that the principle conclusions of the report are that additional hydrometric data will be required to confirm the study results and that the flow estimates can be readily updated as additional data become available.

We thank you for the opportunity to have carried out a most challenging assignment and trust that the report assists your branch in assessing the water resource development opportunities in the Beaver River Cold Lake basin.

Yours truly

NORTHWEST HYDRAULIC CONSULTANTS LTD.

L.E. Bodnaruk, P.Eng.

LEB/smd

Encl.

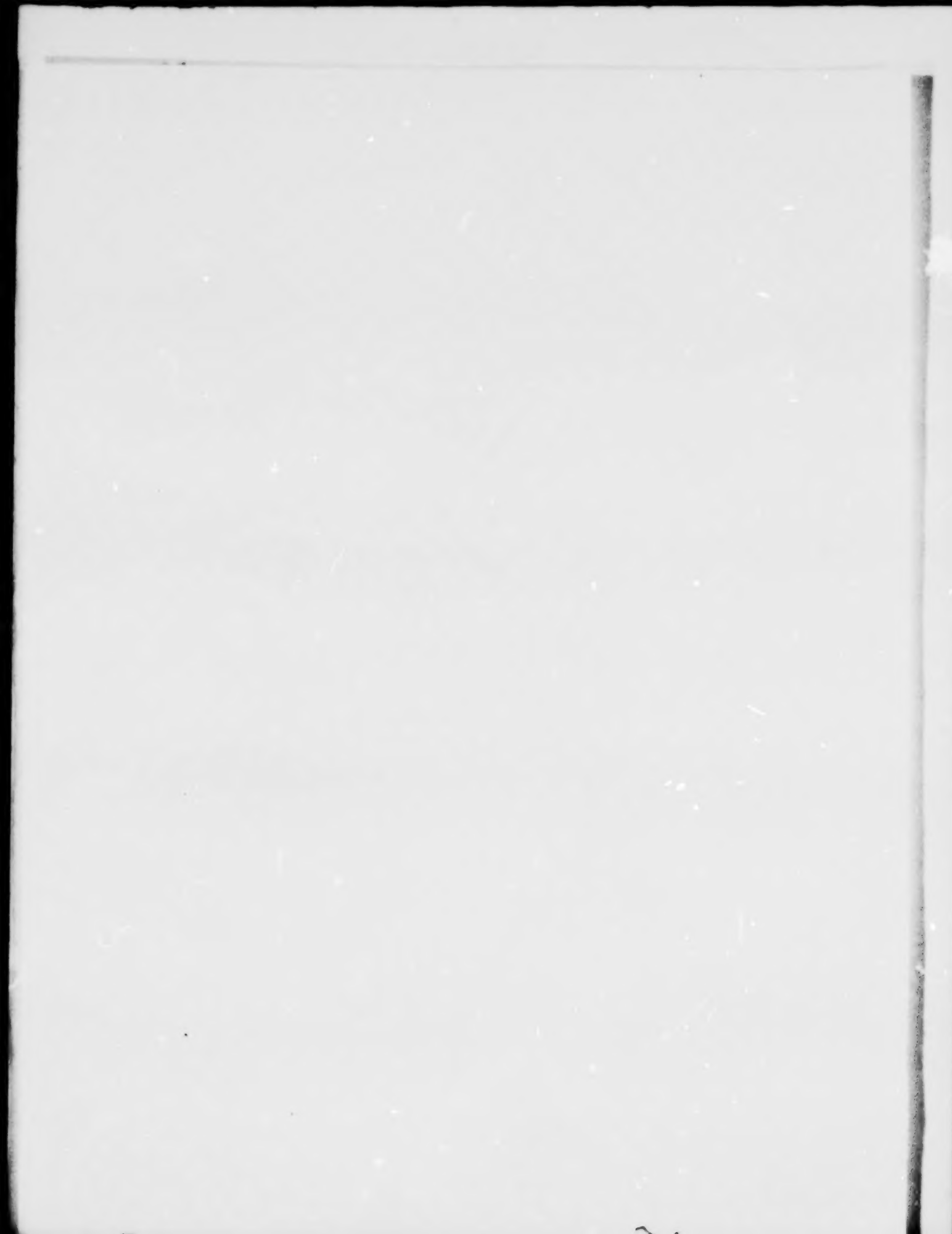


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SUMMARY

A surface water supply study was undertaken to estimate streamflows and lake water levels at 27 points in the Beaver River and Cold Lake basins, as a continuous time series over the period 1956 - 1980.

The required data were estimated from;

- existing hydrometric data where available;
- correlation of hydrometric data to extend the available record for several short-term stations;
- a computer model analysis for study points that had little or no hydrometric data.

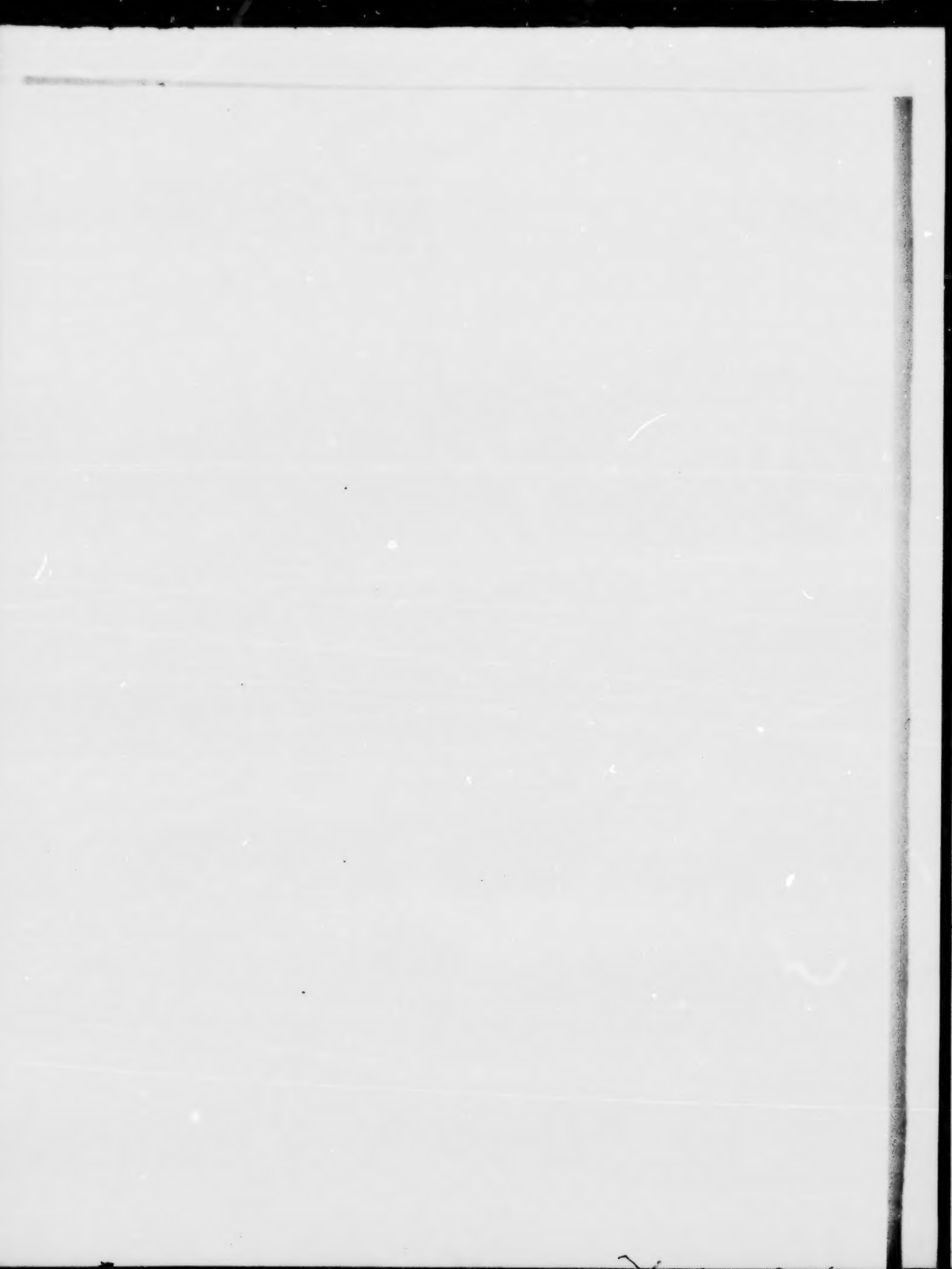
In addition, a brief survey was conducted to gather data on the outlets of several lakes.

Results of the analysis compared well with the limited data available on water levels and discharges within the basin. Historic effects of beaver damming activities were shown to be significant at many sites.

CREDITS AND ACKNOWLEDGEMENTS

Thanks are expressed to Mr. Ron Hicks, P.Eng. of Alberta Environment, Planning Branch, for his direction of the project and to Mr. Ron Bothe, P.Eng., Alberta Environment, Hydrology Branch, for his assistance in data assembly and for his technical reviews which have helped to ensure the quality of the results.

This study was conducted by staff of Northwest Hydraulic Consultants Ltd. under the direction of Mr. L.E. Bodnaruk, P.Eng. Mr. G.V. Gehmlich and Mr. S. Madsen were responsible for computer model applications and Mr. B. Berry was responsible for computer model design. The field survey was led by Mr. S. Seidel. Technical reviews were provided by Mr. C.R. Neill and Mr. E.K. Yaremko.



1. INTRODUCTION

Alberta Environment has undertaken a study of water resources in the Beaver River and Cold Lake basins of eastern Alberta. The purpose of the regional study is to develop a framework plan for the long-term management of the surface and groundwater resources in the region likely to be affected by development of the Cold Lake oil sands.

The regional water resources study includes as one component the analysis of surface water supply. The Surface Water Supply Study was commissioned to Northwest Hydraulic Consultants Ltd. and has as its general objectives the analysis of natural streamflows within the study basin and the synthesis of a 25-year record of streamflows and water levels at a number of selected locations. Specifically, the terms of reference of the Surface Water Supply Study were as follows:

- (a) Select study points where streamflows will be required in the analysis of water quantity demands (i.e. at existing or potential withdrawal points for industrial users, at potential regulated lakes, etc.) and in the analysis of water quality demands (i.e. at existing or potential waste discharge points).
- (b) Review available streamflow, lake level, precipitation and evaporation data; screen and select suitable data.
- (c) Convert streamflow data to natural flow data where necessary.

- (d) Extend natural flows at gauged points to the specified period 1956-80.
- (e) Determine natural flows at ungauged selected study points as per (c).
- (f) Determine the accuracy of the extended and estimated natural flows calculated at each of the selected study points.
- (g) Using long term station data for adjacent and hydrologically similar watersheds, determine the frequency of occurrence of dry and wet periods within the period 1956-80.
- (h) Prepare a report outlining the work program and results, and detailing all data, methodologies, and assumptions used to derive the study results.

A further objective of the study was to develop a methodology that could be readily refined in future years as additional streamflow data became available within the watershed. Since it was recognized that only a first estimate of the required hydrological data would be initially possible with the existing data base, it was important that the methodology could be easily updated in the future.

The methodology that was developed to meet these objectives involved the synthetic generation of sub-catchment flow estimates and the routing of these flow estimates through a computer model of the river system to compute flows and water levels at the required sites.

The principal results of the study are the computer routing model and a data file containing flow data at the selected study points. The model and the data file has been provided to Alberta Environment separately from the present report and includes the following:

- all model data;
- tables and plots of stream discharges and lake elevations;
- summary statistics of discharges and lake elevations;
- streamflow data for input into the water balance model (by Nanuk Engineering and Development Ltd.) for the assessment of water management alternatives.

This report is intended to complete the requirements of the study, specifically to fulfill objective (h) above; to detail all methodologies and assumptions used to derive the study results, and objective (f); to discuss the accuracy and limitations of the derived flows.

2. STUDY AREA

2.1 Physical Features of the Study Area

A general view of the study area is shown in Figure 1. The study area includes the drainage basins of the Beaver and Cold Rivers in the east-central region of Alberta. It has an area of 22,000 km², of which 15,500 km² lies within the Beaver River basin and 6,500 km² within the Cold River basin.

The Beaver River has its headwaters in Beaver Lake near Lac La Biche and a length of some 150 km to the Alberta-Saskatchewan border. Its major tributary is the Sand River which drains about one-third of the basin but contributes one-half of the total Beaver River flow. Other smaller tributaries (from west to east in the basin) are the Mooselake River, Manatokan Creek, Jackfish Creek, Marie Creek, Muriel Creek, Reita Creek, and Redspring Creek.

The distribution of drainage area within the Beaver River watershed is approximately as follows:

<u>Tributary</u>	<u>Drainage Area (km²)</u>	<u>% of Total</u>
Beaver R. above Sand R.	5260	33%
Sand River	5040	34%
Mooselake River	900	5.9%
Manatokan Creek	410	2.7%
Jackfish Creek	490	3.2%
Marie Creek	730	4.7%
Muriel Creek	790	5.2%
Reita Creek	270	1.7%
Redspring Creek	520	3.4%
other	920	6.0%

Within the Cold River basin the primary water features are Primrose and Cold Lakes and the Martineau River connecting the two. Primrose Lake is the largest lake in the study area (445 km^2) and Cold Lake is second largest (350 km^2). This basin includes the watershed of the Medley River as shown in Figure 2.

Primrose Lake drains just over one-half of the Cold River basin within the study area and the Medley River drains about seven percent.

The Cold River is the outlet from Cold Lake. It eventually joins the Beaver River some distance further downstream in Saskatchewan.

A schematic plan showing the more significant elements in the watershed is shown in Figure 3.

Several major physiographic regions are apparent in Figures 2 and 3 as follows:

- the Beaver River basin upstream of the Sand River confluence (local areas 101 and 102) and the adjacent watersheds of Punk, Seibert, and Spencer Creeks (local area 108). This region consists of irregular, poorly drained topography; much of it, especially in the west, is dotted with numerous small lakes which would be expected to reduce runoff from the area by evaporation from the lake surface. Further downstream this area has more intensive agricultural use. Ground elevations in this part of the watershed vary from about 580 to 640 metres above sea level;
- the northern portion of the watershed, in the headwaters of the Sand, Wolf, and Medley Rivers,

Marie Creek, and Primrose and Cold Lake. This area is characterized by extensive areas of treed muskeg with moderate relief. It includes local areas 104, 105, 107, 118 119, 120, 131, 132 and 133 and extends as far south as the upper headwaters of Manatokan and Jackfish Creeks. Much of this area is uninhabited and lies within the Air Weapons Range rising to an elevation of about 760 m;

- the central portion of the watershed along the main stem of the Beaver River. This area has little relief and low runoff and is largely developed for agricultural purposes. It generally lies at an elevation of 550 to 580 metres above sea level. It includes several local areas tributary to the Beaver River (local areas 115, 122, 130, and 134) and the downstream portions of the watersheds of the smaller tributaries;
- the southern part of the watershed, within the upper watersheds of Mooselake River, Muriel Creek, and Reita Creek. This area is notable for large areas of lake relative to the drainage area. Larger lakes in this area are Moose Lake, Muriel Lake, Charlotte Lake, Reita Lake, and Angling Lake. Higher elevations, in the range of 580 to 760 m above sea level, occur in this part of the basin.

These differences in physiography have significant implications for watershed runoff as will be discussed later.

2.2 Climate

The following is a brief overview of the climate of the Cold Lake area, abstracted from Reference 1.

The climate of the Cold Lake area is typically continental, featuring large annual, seasonal, day-to-day, and diurnal temperature variations along with a summer precipitation maximum. The summer precipitation maximum is due mainly to convective activity associated with modified maritime polar air, the most frequent summer air mass.

The average annual precipitation is 470 mm; annual variations are from a maximum of 630 mm in 1960 to a minimum of 310 mm in 1967. The greatest monthly precipitation occurs in July, with an average of 90 mm. July also holds the record for the most precipitation in one month: a total of 179 mm in 1971. The greatest one-day rainfall (94 mm) occurred in July 1962.

Thunderstorms occur from April to October, although only a few occurrences have been reported in the months of April or October. Thunderstorm activity reaches a maximum in July, occurring on an average once in 7.7 days.

Seasonal snowfall, based on the months September to May, averages 135 cm¹. This has varied from a maximum of 236 cm to a minimum of 70 cm.

¹ The water equivalent of 1 cm of snow is approximately 1 mm.

The first measurable snowfall usually occurs in early October; the first measurable snowfall that lasts throughout the winter usually occurs in mid-November and remains until early April. December has the highest monthly snowfall, with an average of 26 cm.

Large annual variations in temperature illustrate the continental type of climate of Cold Lake. The mean monthly temperature varies from -19.1°C in January to 17.2°C in July; the mean annual temperature is 1.2°C . The record maximum temperature of 36.1°C occurred in July 1964 and the record minimum temperature of -48.3°C was recorded in January 1954.

2.3 Runoff Characteristics

The general pattern of runoff in the study area is illustrated in Figure 4 which shows the recorded discharge hydrograph for the Beaver River near Cold Lake over the period 1956 - 1980. Typically, the annual peak occurs from snowmelt in the second half of April and is often followed by several smaller rainfall peaks. However, the highest flow on record in the basin occurred in June 1962 as the result of rainfall runoff. The second highest peak flow resulted from snowmelt in April 1974.

The year of lowest runoff was 1968, and the year of highest runoff was 1962.

Mean annual runoff estimates for various parts of the study basin are presented in Table 1. Data for Table 1 were abstracted from a number of sources and were based on flow records of various levels of quality and extent,

so that they should be considered as estimates only. All values have been adjusted, as best as possible, to a common time base. In this table surface runoff is defined as runoff from land surfaces (not including the major lakes in the watershed). Net runoff is the remainder calculated when direct precipitation on lakes is added to surface runoff and evaporation from the lakes is subtracted.

For purposes of comparison, Table 1 also contains data for three basins outside the study area:

- Dillon River at outlet of Dillon Lake, immediately northeast of the Cold Lake basin.
- Beaver River Dorintosh local, which is the inflow between the Beaver River gauge at Cold Lake (526) and a gauge located further downstream in Saskatchewan, east of the study area.
- Atimoswe Creek, located to the south of the study area.

Comparison of runoff estimates in Table 1 reveals a wide variation within the basin. There is a general trend of decreasing runoff from north to south; the greatest annual surface runoff (125 mm) occurs within the Primrose-Cold Lake basin, followed by the upper Sand River watershed (94-104 mm), the upper Marie Creek watershed (70 mm), Jackfish Creek (54 mm), and finally the lower Beaver River (48 mm). This trend is confirmed by the data for the Dillon River (138 mm) to the northeast, higher than any area within the study basin, and Atimoswe Creek (26 mm) to the south, lower than any area within the study basin.

These variations in annual runoff largely parallel the differences in physiography discussed above.

There are no data on which to base the estimation of runoff in the southern part of the basin, other than a limited amount of flow records on Columbine Creek, located in the south-central region. These data indicate an annual runoff of about 45 mm for Columbine Creek, roughly equal to the mean runoff in the lower Beaver River watershed.

2.4 Study Points

Twenty-seven study points were selected by Alberta Environment and are shown in Figures 1, 2 and 3. These study points are as follows.

<u>Location</u>	<u>SSARR Model Designation</u>
<u>Main stem</u>	
Amisk River near Highway 36	102
Beaver River at Goodridge	502
Beaver River above Sand River	503
Beaver River above Jackfish Creek	515
Beaver River above Marie Creek	5171
Beaver River at Beaver Crossing	526
<u>Sand River</u>	
Caribou Lake	204
Sand River above Wolf River	505
Wolf Lake	506
Sand River at Truman	508

<u>Mooselake River</u>	
Moose Lake	210
<u>Manatokan Creek</u>	
Manatokan Lake	212
<u>Jackfish Creek</u>	
Tucker Lake	216
Jackfish Creek at mouth	516
<u>Marie Creek</u>	
Burnt Lake	218
May Lake	219
Marie Lake	220
Ethel Lake	221
Marie Creek at mouth	521
<u>Muriel Creek</u>	
Muriel Lake	223
Muriel Creek at mouth	525
<u>Reita Creek</u>	
Reita Lake	227
Angling Lake	228
Reita Creek at mouth	529
<u>Cold River</u>	
Medley River	132
Primrose Lake	231
Cold Lake	233

3. HYDROMETRIC AND METEOROLOGICAL DATA BASE

An index map of hydrometric and meteorological stations in the study area was provided by Alberta Environment and is reproduced in Figure 5. This map shows the location and period of record for streamflow gauges operated by Water Survey of Canada, streamflow and lake elevation gauges operated by Alberta Environment, and several additional streamflow and lake level gauges operated by Esso Resources Canada Ltd.

The period of record for hydrometric stations is shown in Figure 6. As shown in this figure, there are only two hydrometric stations in the study area that have records for the entire 1956 to 1980 period: the Beaver River at Cold Lake Reserve (discharges) and Cold Lake (elevations). All other stations have records that are either short term, seasonal, or discontinuous.

Forty short-term intermittent stations shown in Figure 5 that do not coincide with designated study points are not included in Figure 6.

The hydrometric stations have been grouped into three categories depending on the length and continuity of record. These categories are:

- first order stations. Stations of this type have continuous records for the whole study period and are the Water Survey of Canada (WSC) gauges on Cold Lake and on the Beaver River at Cold Lake Reserve. Cold Lake levels were recorded on a weekly or bi-weekly basis, but are considered continuous because they do not change greatly in

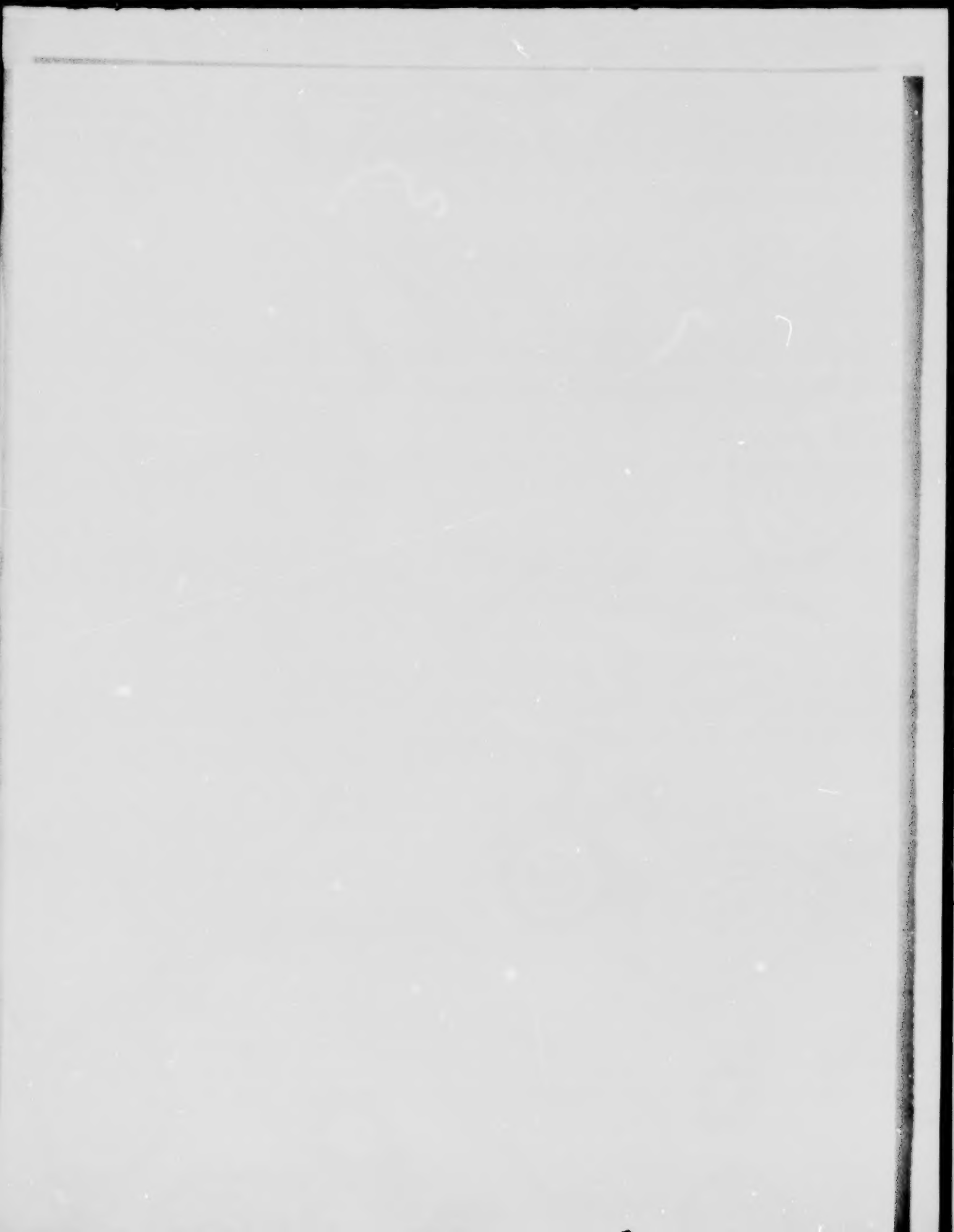
that short a time by virtue of the large amount of lake storage.

- second order stations. These are stations that have a significant length of continuous or seasonal records, and include the WSC Sand, Wolf, Amisk, Beaver, Goodridge, and Jackfish streamflow gauges. These stations all have approximately 10 years of record that is continuous during the summer season. Only the Wolf gauge has winter season records.
- third order stations. These stations have short term, intermittent, or occasional records and are primarily the lake level stations operated by Alberta Environment and short-term stations operated by Stanley Associates Engineering Ltd. for Esso Resources Canada. Data for third-order stations are too sparse to be of direct use in determining lake levels or discharges for the 25-year period; because they are so sparse, their usefulness in the present study is primarily as a means of verifying the computed results.

The locations of the first, second and third-order stations relative to the watershed schematic are shown in Figure 7.

A large number of meteorological stations are indexed on Figure 5, but only a small number lie within the study area. Most of these stations report air temperature and precipitation, and several report other meteorological parameters such as wind, sunshine and pan evaporation.

Meteorological data are not used in this study other than in estimating lake evaporation, and consequently the data will be given little discussion. Evaporation estimates based on the Cold Lake airport data were used in this study as will be discussed later.



4. STUDY METHODOLOGY

A brief introduction to the study methodology is given below. Further details are provided in subsequent sub-sections.

As noted above, the basic study objective was to calculate weekly streamflows for 27 selected study sites for the period 1956 to 1980.

Of the 27 sites, two have streamflow or lake level records for the whole study period, Beaver River at Cold Lake (Beaver Crossing) and Cold Lake. These correspond to the two first-order hydrometric stations. Daily flows have been recorded on the Beaver River since late 1955 and weekly or bi-weekly lake elevations have been recorded on Cold Lake since 1954.

Of the remaining 25 sites, 5 are located at second-order streamflow sites that have roughly ten years of data; the Amisk River at Highway 36 (study point 102), Beaver River near Goodridge (502), Sand River (508), Wolf River (506), and Jackfish Creek (516). All have seasonal record only (spring to fall) except the Wolf River site which has a continuous record. For these sites, the available record was extended to 25 years by correlation with the Beaver River streamflow gauge at Cold Lake Reserve. This analysis will be discussed in some detail later, and involves the following steps:

1. interpolation between late fall and early spring for stations that have no winter record,
2. correlation of recorded daily streamflow with the Beaver River at Cold Lake Reserve,

and 3. calculation of streamflows for the earlier period using the regression equations developed in the correlation analysis.

At the remaining 20 sites, hydrometric data are at best sketchy and at worst non-existent. Discharges and lake levels therefore had to be determined by other means.

A computer modelling approach was adopted for the 20 remaining sites. The basis of this approach is a computer model to simulate the streamflow runoff and routing process within the basin. It involves essentially two tasks: calculation of local runoff hydrographs into lakes and streams, and analysis of routing and water balance to calculate stream discharges and lake water levels at the specified points.

A daily time step was adopted for the calculations instead of the weekly time step proposed in the terms of reference. This decision was made because:

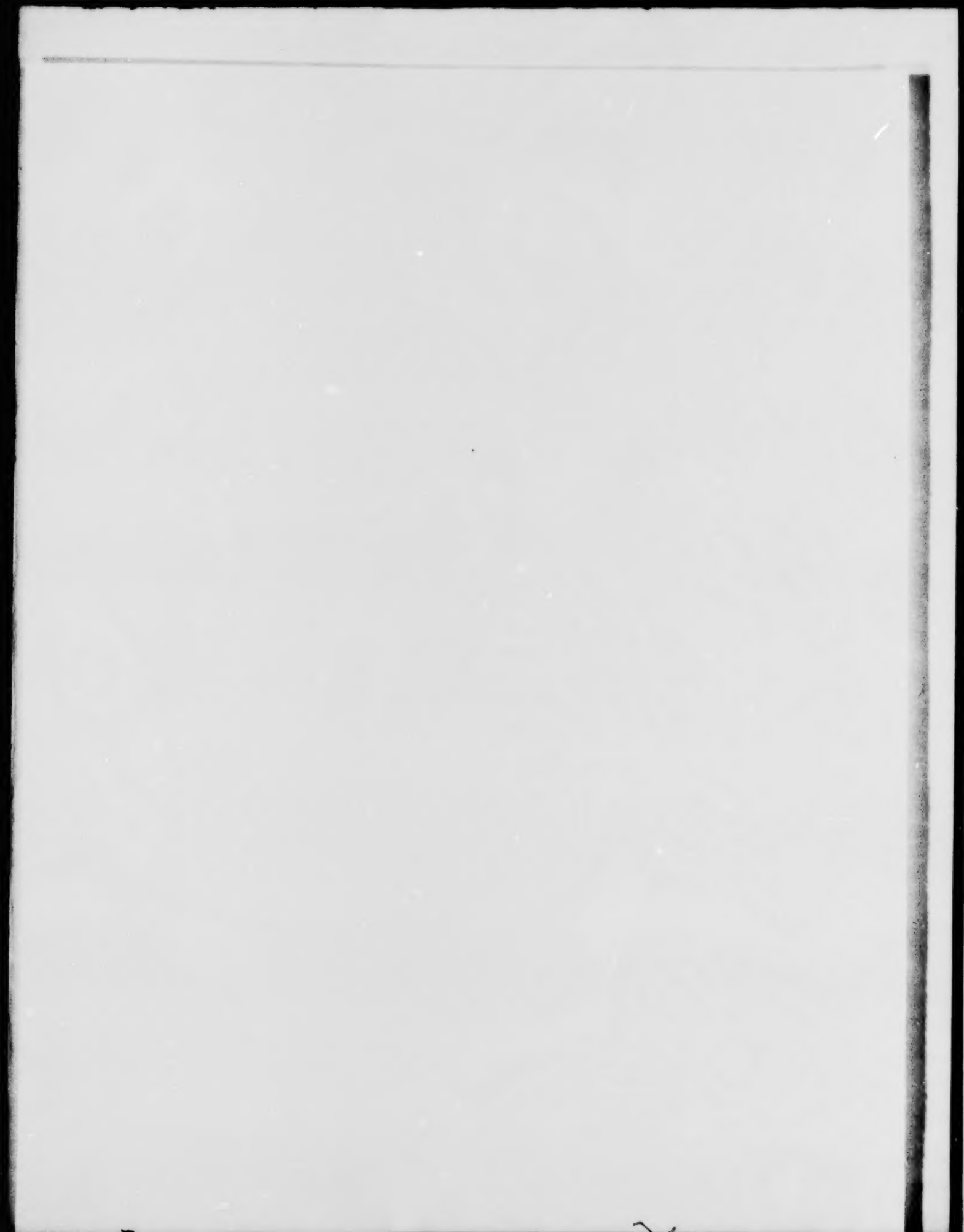
- restrictions in the computer model allow a maximum length of time step of about four days;
- the input data for the correlation analysis and for the model are recorded on a daily basis and thus would have to be converted if a longer time step were adopted;
- the output data would be directly compatible with time steps of seven days or longer and could be converted if necessary at that stage.

Thus the selection of a daily time step was primarily for computational reasons. This should not be

taken to suggest that the results are necessarily meaningful on a daily basis.

The computer model will be discussed in some detail in a later section.

To provide additional data required in the routing analysis, a brief survey program was also conducted.



5. CORRELATION AND EXTENSION OF STREAMFLOW DATA

The objective of the correlation or regression analysis was to extend the data record of the second-order stations to a common 25-year time base, by correlation of the five second-order station flows with those of the Beaver River at Cold Lake.

The first step in this analysis involved the estimation of winter base flows for the four seasonal gauges (Sand River, Amisk River, Beaver River at Goodridge, and Jackfish Creek.) Winter base flows for the four stations were estimated by exponential interpolation using an equation of the form;

$$Q = Q_0 e^{-k(t-t_0)}$$

where Q = computed discharge on day t
 Q_0 = measured discharge on day t_0
 and k = recession coefficient

Values of k were computed directly from the last fall flow measured in each year (typically in early November) and the first flow measured at the start of the subsequent year (typically late February or early March). Values of k for each winter for each station were computed by solving the above equation using the two recorded flows. The computed values of k were then substituted along with the recorded Q_0 to calculate daily discharges.

Where the computed k for a given winter deviated markedly from the norm for a station, it was assumed that the recorded early-winter flows were being affected by unusually high fall runoff. In these cases, daily discharges were calculated with the above equation using a

"normal" or average value of k and the recorded late-winter flow, and the recorded fall flows were extrapolated to meet the recession line. These adjusted winters are as follows:

Sand River	1970/71
Amisk River	1973/74
Jackfish Creek	1973/74

Approximately 40 equations were used (4 stations X 10 years), each of the general form above and differing only in the value of the coefficient k .

The second step in this analysis was a regression made with a standard linear regression program. Linear regression is a general statistical technique through which one can analyse the relationship between a dependent or criterion variable and an independent or predictor variable. This relationship is expressed as a predictive equation of the form;

$$Y = a + bX$$

where Y is the dependent variable (for example the daily discharge on the Sand River),
 X is the independent variable (the daily discharge for the Beaver River at Cold Lake in the present analysis),
 and a and b are coefficients that are calculated by a least-squares curve-fitting technique in the regression analysis.

In the present study, a regression analysis was made of daily flows at each of the 5 second-order gauge sites with the daily flows on the Beaver River at Cold Lake

(study site 526). All recorded flows at the second-order sites were used in the analysis. In addition, estimated winter flows were used for the four seasonal gauges.

To account for lagging or delay between stations, the independent and dependent data were lagged by periods of up to several days and the lag that gave best results was adopted. This provision allows for delays in the river system due to lake routing and stream travel time.

The resulting regression equations express the relationship between daily flows at the desired location with daily flows in the Beaver River at Cold Lake. Adopted regression equations (in Imperial units of cfs) and the correlation coefficients are given below:

<u>Equation</u>	<u>Correlation Coefficient</u>
$Q_{\text{Sand}} = 0.4944 Q_{\text{Beaver}}$	0.952
$Q_{\text{Wolf}} = 0.05779 Q_{\text{Beaver}}(t-5)$	0.936
$Q_{\text{Amisk}} = -12.0 + 0.1705 Q_{\text{Beaver}}$	0.923
$Q_{\text{Goodridge}} = -21.6 + 0.3005 Q_{\text{Beaver}}(t+2)$	0.915
$Q_{\text{Jackfish}} = 0.02844 Q_{\text{Beaver}}$	0.922

In the above equations, the subscript (t-5) denotes a value measured on the Beaver at Cold Lake 5 days earlier and (t+2) denoted a value measured at Cold Lake 2 days later than the date for which a discharge is being computed.

There are two simple statistical measures of the strength of the relationship between the two variables and thus of the accuracy of the results, the Pearson product-moment correlation coefficient (or simply correlation coefficient) and the index of determination.

The correlation coefficient can have a value between 0.0 and 1.0; a value of 0.0 indicates that no relationship exists and a value of 1.0 indicates perfect agreement. The index of determination is calculated as the square of the correlation coefficient and is a measure of the proportion of variance in one variable explained by the other.

In the present analysis the correlation coefficients range from 0.92 to 0.95 and the index of determination ranges from 0.84 to 0.91; i.e. some 84% to 91% of the variation in the dependent variable is explained by the regression equation. These are relatively high values and indicate a fairly strong relationship between gauges.

A visual assessment of the strength of the relationship and hence the accuracy of the regression equation is given by scatter plots and comparative hydrographs, Figures 8 to 12 and 13 to 17 respectively. These figures show that the regression equations provide a good general agreement with recorded flows, although individual flow values may be substantially in error. These errors result primarily from two factors:

1. the variability of rainfall and snowmelt within the watershed; ie. no rainfall or snowmelt event can occur uniformly over a watershed of this size,

2. the hydraulic complexity of the system; ie. lake and channel routing effects are variable over time and are different in different parts of the watershed.

There is no feasible way to eliminate these errors, and hence the results of the regression analysis were adopted for estimating flows in the period prior to streamflow records at the five study sites.

6. FIELD SURVEY AND RESULTS

To provide data required for calculating lake outlet rating curves at several sites, a brief field survey program was undertaken in June 1981. This program included a general reconnaissance of the watershed and outlet surveys at the required sites.

A total of 12 lake outlets were surveyed. These sites were at lake outlets that were to be included in the model and that had little or no data on which to base rating curve calculations.

The outlet survey at each site generally included three cross-sections, a longitudinal slope survey, survey of hydraulic controls (weirs, beaver dams, etc.), survey of lake elevation, and estimation of discharge.

Surveys were done to geodetic elevation at those sites where benchmark elevations were available from the Surveys Branch of Alberta Environment. At six sites, a local datum elevation of 100.0 m was assumed.

Lake outlets that were surveyed were the following:

<u>Lake</u>	<u>Datum</u>	<u>Notes</u>
Caribou	local	- discharge 1.5 cms
Moose	geodetic	- rock weir - low discharge (0.007 cms)
Manatokan	geodetic	- beaver dam at outlet (partially broken)
Tucker	local	- rock-filled timber weir and 2 beaver dams

Burnt	local	- 2 beaver dams at outlet
May	local	- no discernible flow - 2 beaver dams at outlet
Marie	geodetic	- no discernible flow - 3 beaver dams at outlet
Muriel	geodetic	- no flow
Charlotte	local	- road crossing with 3 culverts (one gated)
Reita	geodetic	- no flow
Angling	geodetic	- low discharge (0.03 cms)
Primrose	local	- well-defined outlet channel - discharge 4.8 cms

Several observations resulting from the field survey are as follows:

- many of the lakes in the project area have poorly-defined outlets or have beaver dams at their outlet. As a result, it is not possible to define an accurate outlet rating curve at many of the lakes, and furthermore, the outlet rating characteristics are subject to periodic and unpredictable changes due to construction and destruction of beaver dams;
- lakes in the study area display evidence of only a small range in water level fluctuation;
- except for Primrose and Caribou Lakes, all had very low or no outflow at the time of survey;
- one local resident commented that he had seen no outflow from Muriel Lake in the last 30 years.

The accuracy of this report is suspect, but there is likely some truth to it in that outflows have likely been sporadic and of low magnitude.

Selected photos at each of the lake outlets and on the Beaver River are shown in Photos 1 to 12.

The survey results were used in the calculation of lake outlet rating curves as will be discussed later.



7. THE SSARR ROUTING MODEL

7.1 Objectives

As noted in Section 4, the basic objective of the computer routing model was to calculate streamflows and lake levels at the 20 study points for which there is only a very limited amount of measured hydrometric data. In addition, runoff data and lake outlet rating curves were to be provided to Nanuk Engineering and Development Ltd. for use in the water balance model.

7.2 Model Overview

The computer program used for the routing model is the Streamflow Synthesis and Reservoir Regulation program (SSARR).

The SSARR computer program was developed by the U.S. Army Corps of Engineers, North Pacific Division, for operational use in hydrologic engineering studies and daily streamflow forecasting. It is comprised of two basic components:

- a) a generalized watershed component for synthesizing runoff from snowmelt and rainfall;
- b) a river system component for routing streamflows from upstream to downstream points. The river system component is designed to simulate the routing and travel of runoff flows through a lake and river system. It can be configured to simulate lake storage and routing as well as the routing or travel of water through a river reach.

For the purposes of the present study, only the second component, the river system/reservoir regulation model, was used.

For specific details on the computer program, the reader is referred to the program user's manual, Reference 2.

A basic schematic of the river system model is given in Figure 3. The principal components in the computer model include:

- local area runoff or inflows
- lakes
- channel reaches
- sum points or nodes

The schematic does not show direct lake evaporation/precipitation, nor does it show the dummy or artificial volume balance reservoirs that were used for determination of simulation errors.

The final model data and details of their derivation are described below.

7.3 Model Data

7.3.1 Local Area Runoff

The most fundamental or elementary unit of watershed area in the model is the local drainage area, or simply local area. It is a small portion of the watershed that is considered to yield runoff into lakes or river reaches.

An outline of the local areas included in the model is given in Figure 2, and drainage areas are tabulated in Table 2.

For purposes of the SSARR model, runoff hydrographs for local areas were estimated by comparison or analogy with gauged watersheds having similar runoff characteristics. Streamflows for ungauged local areas were estimated as a percentage of the gauged streamflows based on the size of the local area and an estimate of its annual runoff depth. This procedure will be discussed below with reference to Figure 18.

In Figure 18 local areas that are judged to be of similar hydrologic character or to have similar streamflow patterns on the basis of similarities in location, soils, vegetation and land use are shown as dots of the same colour. The colored lines depict the outlines of sub-basins; these are larger areas for which runoff records are available.

For example, consider sub-basin 516 (it includes local areas 116 and 117). The runoff from the sub-basin is represented directly by the second-order streamflow gauge on Jackfish Creek (sum point 516). On the assumption that local areas 116 and 117 are hydrologically similar and have the same annual depth of runoff, their daily flows were estimated as a percentage of gauged flows for sub-basin 516. These percentages were calculated as the ratio of local drainage area to the total drainage area at the Jackfish gauge¹.

¹ Runoff percentages for local areas 116 and 117 were increased slightly after initial calibration runs to compensate for evaporation losses on Tucker Lake.

The sub-basin boundary and the local areas are shown in the same color in Figure 18 to depict the assumed similarity in hydrologic characteristics.

Runoff flows for local areas 104, 105 and 107 were estimated as a proportion of drainage area of sub-basin 907. This sub-basin consists of the watershed tributary to the Sand River upstream of the Wolf River plus Fisher Creek. The three local areas are physiographically similar and were assumed to be hydrologically similar.

Streamflows for sub-basin 907 have not been measured directly, but can be calculated from recorded flows at the second-order Sand River gauging station (sum point 508) by subtracting runoff flows from local area 108 (Punk and Seibert Creeks) and sum point 506 (Wolf Lake outlet).

Streamflows at sum point 506 are given directly by streamflow records and the correlation analysis discussed earlier. Assuming that Punk and Seibert Creeks have similar runoff characteristics and the same annual depth of runoff as does the watershed upstream of the Wolf Lake outlet², daily flows from local area 108 can be estimated as a proportion by drainage area of daily flows at sum point 506.

² The validity of this assumption cannot be proven in the absence of streamflow records. It appears reasonable because of similarities in watershed physiography, nearness of the watersheds, and the presence of large lakes in all three watersheds.

Since the drainage area of local area 108 is 1.06 times as large as the drainage area of 506, the calculation of streamflows for sub-basin 907 can be expressed as:

$$Q_{907} = Q_{508} - 2.06 Q_{506}$$

Calculation of local area flows in sub-basin 930 (the lower Beaver River sub-basin) is somewhat more complicated than in the other sub-basins because the local areas fall within substantially different physiographic regions and could therefore be expected to have substantial differences in runoff characteristics. This sub-basin includes forested regions tributary to Marie Creek and agricultural land tributary to the Beaver River.

The net daily flows from sub-basin 930 were calculated as the difference between the lower Beaver River gauge (526) and the upstream Beaver, Sand and Jackfish gauges (502, 508, 516). In this calculation a travel time of two days was adopted from the Sand and the upstream Beaver gauge to the lower Beaver gauge, and a travel time of one day was adopted from the Jackfish gauge to the lower Beaver gauge.

The calculation of local area flows in this sub-basin was made as follows:

1. An analog sub-basin was selected. The analog sub-basin is the one which has a seasonal pattern of runoff most similar to the desired local area (usually the closest or most physiographically similar);

2. Daily flows at the analog station were multiplied by a local area yield factor that included the ratios of drainage areas and annual runoff depths, i.e.

$$\text{Factor} = \frac{Q_{\text{local}}}{Q_{\text{analog}}} = \frac{A_{\text{local}}}{A_{\text{analog}}} \times \frac{R_{\text{local}}}{R_{\text{analog}}}$$

where: Q = discharge
 A = drainage area
 R = runoff depth

For each local area, the annual runoff was estimated or assumed by comparison with the runoff data in Table 1.

An identical procedure was used to estimate local area flows downstream of the Beaver River gauge (526) and in the Medley River (local 132).

Analog stations, estimated annual yields, and computed yield factors for each local area are given in Table 3. Analog stations used for the various local areas are depicted graphically in Figure 18.

Inflows into Cold and Primrose Lakes (locals 131 and 133) were calculated from Cold Lake outflows by means of a reverse lake routing procedure. This procedure is similar to a standard lake routing analysis except that it calculates inflows from outflow data rather than the reverse. These local area flows were computed as net inflows, i.e. inflow minus lake evaporation and precipitation. This calculation included the following steps:

1. Route Cold Lake outflows backward through an artificial aggregate lake (area equal to Cold plus Primrose, with Cold Lake outlet rating curve).
2. Subtract local 132 (Medley) estimated as 13% of station 907.
3. Add net evaporation from the aggregate lake.
4. Proportion the result of step 3 on the basis of drainage area between local 131 and 133.
5. Subtract net evaporation from Primrose and Cold Lakes (as a percentage by lake area of the aggregate lake - step 3) from local 131 and 133 respectively.

Daily local inflows into Wolf Lake were also computed for use in the water balance model (not required in the SSARR model). The inflows were calculated from Wolf Lake outflow data (node 506), using the reverse routing procedure, and were calculated as net inflow to include Wolf Lake evaporation and precipitation.

7.3.2 Lake Precipitation and Evaporation

The water balance through each lake requires the addition of precipitation onto the lake surface and subtraction of evaporation from the lake surface (except for Cold, Primrose and Wolf Lakes for which net inflows were provided to include evaporation and precipitation).

For purposes of the SSARR model, daily values of lake precipitation/evaporation were computed with the following procedure:

1. estimate monthly evaporation using a modified form of Meyer's method (discussed further below) and assume evaporation rate is constant over the month;
2. subtract daily evaporation from daily precipitation to calculate net precipitation/evaporation;
3. convert net precipitation/evaporation to an equivalent flow rate in cubic feet per second into an artificial or reference lake of 200 square miles (518 km^2). This reference lake was designated model point 800;
4. compute individual lake precipitation/evaporation as a percentage of station 800 based directly on lake area (Table 4)¹.

The calculation of monthly evaporation rates made use of Meyer's method as outlined in Reference 3 combined with Morton's method as outlined in Reference 8. Meyer's method uses empirical relationships between monthly evaporation and air temperature, water vapour pressure, and wind speed, whereas Morton's method uses a more detailed energy budget approach. In the final model runs

¹ The lake size of 200 mi^2 adopted for station 800 is larger than any lake in the watershed and was adopted to permit all lake precipitation/evaporation percentages to be less than 100%.

the monthly values computed with Meyer's method were adjusted to total the annual values predicted with Morton's method².

7.3.3 Lake Routing Characteristics

For simulation of the lake routing process, the computer model uses an elevation-storage-discharge table for each lake. This table represents the relationship between lake elevation, outflow rate, and storage volume, and is used in simulating the lake water balance.

The storage volume for each lake was estimated from the nominal lake water area (Table 4), assuming a circular lake with shoreline slopes of 10 horizontal to 1 vertical. This procedure is only approximate, but is thought to result in only small errors in most cases if the range in water level fluctuation is relatively small.

Lake storages were defined to have a zero value at the elevation of the lake outlet sill. Elevations above the sill were assigned positive values of storage and elevations below were assigned negative values.

² Monthly values computed with Morton's method were used in early model runs but were found to over-estimate annual evaporation. Experience with the two methods of calculating lake evaporation seems to indicate that Morton's method gives more reliable estimates of annual totals but less reliable estimates of monthly variation. Hence the hybrid approach described above was adopted.

Outlet discharge rating curves were determined from the following (in order of decreasing priority):

- Water Survey of Canada rating curves
- Alberta Environment rating curves
- Stanley Associates Engineering Ltd. rating curves
- hydraulic computations based on surveys conducted by Northwest Hydraulic Consultants Ltd., Alberta Environment, and Stanley Associates Engineering Ltd.

Two factors affect the reliability of the outlet rating curves and should be kept in mind in interpreting the study results. The first is non-stationarity of the rating curves, i.e. the outlet rating characteristics of a lake can change from time to time due to changes at the lake outlet such as erosion of the outlet channel, construction or destruction of beaver dams (e.g. Ethel Lake), and construction of man-made controls (e.g. rock weir at Moose Lake outlet and constructed channel at Reita Lake outlet). In all cases except Ethel Lake the rating curves were based on conditions that existed in early 1981. On Ethel Lake a lake level control weir was constructed in 1980 and the rating curve used in the model was based on conditions that existed between August 1978 and March 1980.

The second factor that affects the quality of the computed rating curves is the limited amount of field data at several sites which was sufficient for only approximate calculations.

Because of these two factors, the rating curves used in this study should be considered a first estimate

only, subject to change with time and subject to field confirmation with discharge measurements.

A summary of the source data used in developing outlet rating curves is presented in Table 5, and the outlet rating tables are included in the model data listing in Appendix A.

7.3.4 Channel Reaches

The river model also simulates streamflow routing through channel reaches. It simulates storage and lagging effects in river reaches with Wilson's storage routing equation (Reference 2).

The routing method requires two coefficients (KTS and n) to describe the routing characteristics of a channel reach. In the present study an approximate method of estimating KTS and n was developed from an analysis of Water Survey of Canada gauge rating curves as follows:

1. Flow velocity was plotted against discharge for each gauge site (Figure 19) to yield estimates of the coefficient n (the exponent in the equations fitted to the data in Figure 19). The curves have a common value for n equal to 0.5 which was adopted for all channel reaches in the study.
2. Values of k_1 (the constant in the equations in Figure 19) were plotted against the drainage area of each gauge (Figure 20). The fitted curve in Figure 20 was adopted to calculate the coefficients k_1 and KTS (defined by $KTS = k_1 L$ where L is the length of reach) for all study reaches.

Each reach in the model was divided into a number of lengths or phases of 8 to 11 km in length (5 to 7 miles), in accordance with Corps of Engineers recommendations.

Values of KTS, n and number of phases adopted in the model are provided in the model data, Appendix A.

7.3.5 Sum Points

Sum points in the model (also called transfer points) represent nodes or confluences in the river system. They provide a location for input or output of discharge and lake level data into the model.

7.3.6 Dummy Reservoirs

A number of artificial or "dummy" reservoirs are included in the model for the purpose of calculating the amount of simulation error and providing an overall check on volumes of runoff. These dummy reservoirs are defined in such a manner that every foot of storage or reservoir elevation represents one inch of basin runoff in the corresponding watershed basin.

The dummy reservoirs are essentially "invisible" to the user of model output in that they are provided for computational purposes only. Consequently they have not been shown on the model schematic. They have been included in the model data listing (Appendix A) and in the calibration output (Appendix B).

7.3.7 Initial Conditions

The operation of the model requires that initial or starting levels of discharge and lake elevation be specified at the start of the simulation period (January 1, 1956).

Without measured data it is not possible to determine initial conditions directly so they must be estimated indirectly. Consequently the initial conditions at the start of run may be substantially in error although in most cases the errors tend to damp out within a few weeks. On larger lakes (e.g. Cold and Primrose Lakes) or in connected lakes (e.g. upper Marie Creek watershed), the initial error may persist for some time, possibly up to a year or more.

In the present study, a first run was made to establish initial conditions for the start of simulation. Lake levels and discharges computed at the end of December 1974 in the first run were taken as a first estimate of January 1956 conditions. These estimates were then adjusted on the basis of gauged streamflows in the Beaver River at Cold Lake for January 1956 to give the initial conditions finally adopted in the model.

7.3.8 Data Listing

A complete copy of the final model data including sample streamflow input data for the 1956 to 1980 period is provided in Appendix A. A complete disc file was provided to Alberta Environment, containing the computer program, the model data, all streamflow input data, and a

complete set of computer procedure commands required for operating the model. This file was provided under separate cover.

7.4 Model Results

Model results were provided under separate cover to Alberta Environment, and included the following:

- discharge and lake stage hydrographs;
- flow/elevation frequency and flow/elevation duration curves;
- tables of daily flows and lake elevations and historical summaries.

A sample of the discharge hydrographs is given in Figure 4 for the Beaver River at the Water Survey of Canada gauge site (526), and three flow frequency curves (minimum, mean and maximum flow frequency curves) and one flow duration curve for the same site (526) are provided in Figure 21. The significance of the flow frequency and duration curves is discussed below.

The minimum annual flow frequency curve (top left of Figure 21) indicates the frequency with which the annual minimum flow was lower than a certain value; it indicates that only one year in 25 (4% of the years) had a day of flow that was lower than about $0.4 \text{ m}^3/\text{sec}$. Conversely, the maximum annual flow curve (upper right in Figure 2) shows the frequency with which the annual peak flow exceeded a given value; for example, that a daily

flow of about $600 \text{ m}^3/\text{sec}$ or more occurred in only one of the 25 years.

The mean annual flow frequency curve (top centre of Figure 21) shows that the annual flow (ie. the average over the whole year) was greater than about $60 \text{ m}^3/\text{sec}$ once in 25 years (4% of the years) and that it was greater than about $8 \text{ m}^3/\text{sec}$ in 24 of the 25 years (96% of the years). The median annual flow (50% frequency) was about $25 \text{ m}^3/\text{sec}$.

The frequency curves can be used as probability curves to predict the occurrence of more severe events, as is done in a flood frequency analysis. In a flood frequency analysis, a probability distribution is assumed and is "fitted" to the plotted data. The distribution for floods is commonly assumed to be a log-normal distribution and the curve can be fitted by eye or by means of a statistical routine. Once this is accomplished, the curve can be extrapolated (with caution) beyond the range of observed data to predict a more severe event.

In the above example, a fitted curve (by eye) is shown on the maximum flow frequency curve. Extrapolating to a 1:100 frequency (1% probability) gives a flood estimate of about $850 \text{ m}^3/\text{sec}$.

The flow duration curve shows the frequency with which daily flows exceeded a certain value when all flows are considered (rather than the annual minima, maxima, or means as in the case of the flow frequency curves). For example, it shows that half of all days in the period (50%) had a daily flow of $25 \text{ m}^3/\text{sec}$ or greater. The highest and lowest daily flows in the whole period of record are also shown on this graph.

It should be noted that the finest resolution achieved in the model output is $0.03 \text{ m}^3/\text{sec}$ (1.0 cfs). Zero flows or flows less than $0.03 \text{ m}^3/\text{sec}$ are plotted as $0.001 \text{ m}^3/\text{sec}$ for the 20 modelled sites (effectively $0.0 \text{ m}^3/\text{sec}$).

An example of a lake elevation plot (stage hydrograph) is given in Figure 22 for Cold Lake (study point 533), and elevation frequency and duration curves are given in Figure 23 for the same site.

8. DISCUSSION OF RESULTS

The following is a discussion of the reliability and quality of the computed flows and water levels at the 27 study sites.

8.1 First-Order Hydrometric Gauge Sites

At two study sites (533 and 526), flows and water levels were based directly on hydrometric records for the entire 25-year period. Consequently the reliability of study results for these two sites is high and is limited only by the accuracy of the discharge and water level measurements (generally accurate within a small percentage).

8.2 Second-Order Streamflow Gauge Sites

Five of the study points are located at second-order hydrometric gauge sites (study points 506, 508, 103, 502 and 506). Discharges at these sites have been recorded since about 1970. For the period prior to 1970, discharges were estimated with the correlation analysis as described in Section 5 of this report.

Discharges calculated with the correlation analysis are subject to uncertainty due to scatter or imperfect fit in the correlation relationships between the second-order stations and the first-order station on the Beaver River at Cold Lake. As demonstrated earlier, there may be substantial variation in individual runoff events even though there appears to be a strong and generally

consistent relationship between the stations. This variation is not surprising, given the variability of rainfall and snowmelt within a basin as large as the Beaver River basin, but it can lead to substantial differences in individual daily flows.

A second source of uncertainty is that correlation relationships were extrapolated outside the range of measured data used in the correlation and were assumed to be constant with time. It was assumed without means of verification that the relationships developed on the basis of streamflows recorded since 1970 are applicable to the earlier period. Because of land use changes in parts of the watershed over time and natural variability in the hydrologic cycle, this assumption may not be strictly valid. In particular, the earlier period included a drier year (1968) and a wetter year (1962) than occurred in the period of record and hence the computed discharges would be less reliable in those periods. In addition, it would appear that winter flows were consistently lower in the period prior to 1970 than in recent years (see Figure 4) and it is not certain whether the correlation relationships are reliable for the lower flows.

There are no hydrometric records in the watershed with which to confirm the computed results directly. Prior to 1970, discharge measurements were made only for the Beaver River at Cold Lake. It is possible to achieve only general confirmation by comparing the recorded discharges with the discharges computed by the model at that location. This comparison is made in Appendix B and demonstrates good agreement for the entire period with no apparent differences between the 1956 - 1970 period and

the 1970 - 1980 period. The total runoff computed over the 25 year period was 2.6% higher than the measured runoff. This comparison gives some confidence to the computed flows.

8.3 Other Sites

At the 20 sites where flows and water levels were computed by modelling, the reliability of the computed results is even more variable and more difficult to quantify. The only measure of accuracy is given by the small amount of recorded data at those sites within recent years.

A visual assessment of reliability at these sites is given in Figures 24 through 34 in which computed water levels and discharges are plotted along with recorded data. No lake level or discharge records are available prior to 1970 and the data since 1970 are adequate for only a general definition of water levels and flows.

The available lake elevation data shows reasonably good agreement with computed levels at all sites. There is a general agreement in seasonal variation (ie. computed levels rise more-or-less as they should) and computed water levels are generally within about 0.1 to 0.2 m of the recorded levels.

During several periods there is poor agreement; in 1973 on Moose Lake, winter 1974 on Manatokan Lake, winter 1977-78 on Marie and Ethel Lakes and the 1978-79 period on Augling Lake. The maximum deviation is 0.4 m on Moose Lake, 0.7 m on Manatokan Lake, 0.3 m on Marie Lake,

0.6 m on Ethel Lake and 0.4 m on Angling Lake. In each of these periods the recorded levels were higher than normal and were higher than the computed levels, suggesting that recorded levels were affected by construction of beaver dams (this is known to be true on Ethel Lake).

Recorded discharge data are even less dense than lake elevation data, and consist only of miscellaneous discharge measurements and annual peaks at the outlet of Manatohan Lake since 1971 and occasional discharge measurements downstream of Marie, Ethel and Primrose Lakes and on the Medley River since 1977.

There is good agreement between observed and computed flows at Primrose Lake and Manatohan Lake outlets and on the Medley River, although the model appears to over-estimate dry period flows on the Medley. There is no consistent comparison at Marie Lake and Ethel Lake outlets (note that the recorded flows for Marie Lake were measured some distance further downstream and hence include some downstream runoff). The model appears to under-estimate in 1978 for these two sites, to overestimate in 1980, and to give good comparison in 1979. However, it is difficult to assess the significance of these differences, given the short length of record and the fact that recorded discharges are affected by beaver damming activity in 1978 and by operation of the Ethel Lake control structure in 1980.

There are no discharge measurements in the Moose, Muriel and Reita basins. Consequently the computed flows and lake levels should be used with caution. These basins have a large area of lake relative to the size of the contributing watershed (Moose, Muriel and Reita Lakes) and

as a result lake evaporation is a significant percentage of total lake inflow in these areas. Consequently computed streamflows and lake elevations are sensitive to small errors in estimating either evaporation or inflow.

One of the most significant factors that affects the computed and recorded levels above is the effect of beaver damming on lake outlet characteristics. It is known that beaver dams presently exist at a large number of the smaller lakes included in the model, and it is likely that beaver dams have existed at other lake outlets in the past. These beaver dams have significant hydrologic impacts because they can cause substantial changes in lake levels and outlet flows, and furthermore they are variable over time as dams are constructed, raised, aged, or destroyed. Consequently, a "random" effect is introduced into the hydrologic cycle; it is an effect that cannot be related to other hydrologic events except in a general fashion.

An example of the effects of beaver damming at a lake outlet is at Ethel Lake for which beaver dam effects over the past seven years have been documented in a report by Stanley Associates Engineering Ltd. (Reference 6) and in a subsequent report by Northwest Hydraulic Consultants Ltd. (Reference 7). High water levels in the winter of 1977-78 (see Figure 27) are known to have resulted from a dam at the outlet; these water levels are about 0.5 m above the normal winter level recorded in other years, in spite of the fact that 1977 was the year of second lowest runoff in the study basin during that period. As pointed out in Reference 7, this results in a temporary storage of runoff water that can be as great as 30% of the total annual runoff at that point, which can affect flows and

water levels well downstream in the system as well as on Ethel Lake itself.

Other than temporal effects introduced by beaver damming activity, the study results appear to be reasonably reliable.

Other effects are apparent on Ethel Lake in 1980, due to the construction of a control weir at the lake outlet. High lake levels (relative to previous winters) occurred in winter of 1980 when the runoff was the lowest of the seven years.

8.4 Evaluation of Results

On the basis of experience gained with the basin in the course of this study, it is possible to make a subjective assessment of the reliability of the results for use in planning studies. This assessment is summarized as follows (see below for definition of ratings:

	<u>Study</u> <u>Point</u>	<u>Flows</u>	<u>Water</u> <u>Levels</u>
<u>Sand River</u>			
Caribou Lake	204	G	G
Sand River above Wolf River	505	G	-
Wolf Lake	506	G-E	G-E
Sand River at Truman	508	G-E	-
<u>Mooselake River</u>			
Moose Lake	210	G	F
<u>Manatokan Creek</u>			
Manatokan Lake	212	G	F

	<u>Study Point</u>	<u>Flows</u>	<u>Water Levels</u>
<u>Jackfish Creek</u>			
Tucker Lake	216	G	F
Jackfish Creek at mouth	516	G-E	-
<u>Marie Creek</u>			
Burnt Lake	218	F	F
May Lake	219	F	F
Marie Lake	220	F	F
Ethel Lake	221	F	F
Marie Creek at mouth	521	F	-
<u>Muriel Creek</u>			
Muriel Lake	223	F	F
Muriel Creek at mouth	525	F	F
<u>Reita Creek</u>			
Reita Lake	227	F	F
Angling Lake	228	F	F
Reita Creek at mouth	529	F	-
<u>Cold River</u>			
Medley River	132	F	-
Primrose Lake	231	G	GW
Cold Lake	233	E	GW

Definitions:

- E - Excellent quality; based directly on recorded flows; useful for all purposes
- G - Good quality; main stem flows based on correlation analysis and modelling; not sensitive to modelling assumptions; useful for planning purposes; dry periods less reliable with no verification data
- F - Fair quality; based on modelling with little verification data; sensitive to modelling assumptions and beaver damming and other temporal effects; good general agreement with substantial variation in specific events; useful for planning purposes with caution

W - Shoreline locations are subject to wind set-up effects.

These subjective evaluations above are based on our experience and familiarity with the basin gained in the course of this study and on the limited amount of hydrometric data available. In the above definitions, planning purposes are considered to include water resources allocation studies at the planning level, as opposed to design studies or regulation of water users.

9. CONCLUSIONS

The Surface Water Supply Study as described above has provided a synthetic 25-year record of streamflows and lake elevations at 27 points within the Beaver River - Cold Lake Study area, and has provided a computer model of the river system that can be readily updated in the future as additional hydrometric data become available.

As discussed earlier, the reliability of the present estimates of water levels can be demonstrated at several locations where lake levels have been recorded periodically within the past ten years. The computed lake levels are generally within 0.1 to 0.2 m of the recorded levels, except for several periods when beaver damming activity that is not simulated in the model is suspected of having affected the recorded levels.

The reliability of the present estimates of streamflow is more difficult to establish with the present data base because the streamflow records presently available are too sparse to yield meaningful comparisons. It is felt that the long-term trends are simulated well, but that short-term trends are influenced strongly by temporal effect that cannot be simulated precisely, such as beaver damming effects and variability of storm events over the watershed.

Until such time that additional streamflow data are available to confirm or refine the streamflow and water level estimates, the model results should be used with caution.

Additional hydrometric data will be required to update the model parameters and to confirm the validity of

the model results more definitely. Model refinements will be possible in two areas:

1. refinement of local area runoff calculations. As additional streamflow data are collected in the watershed, it may be possible to refine the estimates of local area runoff and to confirm them with recorded discharges.
2. refinement of lake outlet rating curves. Several of the outlet curves were based on a minimum of data and consequently they are subject to significant error. Improvements may be possible with a more extensive program of surveys and lake level/discharge measurements. Any future changes in outlet conditions will be detected with more frequent surveys; and can then be reproduced in the model. It should be noted however, that historic effects of beaver damming construction and destruction cannot be reconstructed.

Collection of the required hydrometric data will entail a fairly substantial commitment of effort and funds because the gauges would have to be operated continuously over a period of two to three years or more to achieve a significant improvement. These data would be of direct use in the planning process and would also provide a basis for confirming or refining the model. It is our view that a small number of streamflow and lake elevation gauges located in the most critical or important areas of the watershed would give useful data.

Selection of specific gauge sites is beyond the scope of this report.

The effects of beaver dam construction and destruction on lake levels and stream flows has been noted above. It is not clearly understood how the overall effects of beaver damming influence the hydrologic cycle nor how the construction and destruction cycles are influenced by the hydrologic cycle, except in a general way. There may be grounds for research in this area.

In the early stages of project planning, two stages of modelling were considered. They were:

- a routing model (the present study), to simulate hydraulic routing through lakes and stream channels. Runoff hydrographs (into lakes and stream channels) for this stage have been estimated from gauge records by analogy or correlation;
- a runoff-routing model. This second stage model would be built on the first stage model and would involve synthesizing inflow hydrographs with a watershed runoff model. The watershed runoff model would be based on a detailed simulation of the runoff process, including precipitation, snowmelt, infiltration and soil moisture balance and three-phase routing (surface, runoff, interflow and groundwater flow).

It was intended that a decision to implement the runoff model would be based on the findings of the present study, as it was felt that the runoff process would be better understood at this stage and that the feasibility of runoff modelling could be better assessed.

It is our present opinion, at the conclusion of the routing model operation, that the runoff model would

require an extensive amount of data that is not available within the watershed. The data, required for simulation, calibration and verification, include meteorological data, soils data, and an extensive amount of runoff data. None of this data is more extensive, complete, or better distributed within the watershed than the basic streamflow data that has been used as input to the routing model. Consequently the results of a runoff model, in the absence of an extensive data base, would not be more reliable than those of the routing model used in the present study.

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TABLE 1
RUNOFF ESTIMATES IN THE STUDY BASIN

BASIN	NET ⁽¹⁾ RUNOFF (mm)	SURFACE RUNOFF (mm)	SOURCE
<u>BEAVER RIVER BASIN</u>			
Sand R. above Wolf R.	104	104	AE Reference 4 ⁽²⁾
Wolf R. above Wolf L. outlet	71	94	AE Reference 4
Marie Cr. above Ethel L.	44	71	SAEL revised (Reference 5)
Marie Cr.-Marie L. to Ethel L.	22	70	SAEL revised (Reference 5)
Jackfish Cr. above WSC gauge	45	54	NHCL from WSC data
Amisk R. above WSC gauge	58	nd	NHCL from WSC data
Beaver R.-Amisk gauge to Goodridge gauge	46	nd	NHCL from WSC data
Beaver R.-Goodridge gauge to Cold L. gauge	36	48	AE Reference 4
Columbine Cr.	43	43	AE Hydrology Branch
<u>COLD RIVER BASIN</u>			
Primrose L.	85	125	NHCL from SAEL data
Cold L.-Primrose L. to Cold L. outlet	91	125	NHCL from SAEL data
<u>ADJACENT BASINS</u>			
Dillon R. above WSC gauge	138	nd	NHCL from WSC data
Beaver R.-Cold L. gauge to Dorintosh gauge	51	nd	NHCL from WSC data
Atimoswe Cr.	17	26 ¹	AE Hydrology Branch

¹ Surface runoff is defined as runoff from land surfaces into lakes. Net runoff is surface runoff minus net lake evaporation.

² AE - Alberta Environment Hydrology Branch
 SAEL - Stanley Associates Engineering Ltd.
 NHCL - Northwest Hydraulic Consultants Ltd.
 WSC - Water Survey of Canada
 nd - not determined

TABLE 2
LOCAL DRAINAGE AREAS

LOCAL AREA	DRAINAGE AREA km ²	mi ²
101	2311	854
102	2500	965
103	549	212
104	1386	535
105	1728	667
106	694	268
107	308	119
108	772	298
109	119	46
110	754	291
111	104	40
112	409	158
115	388	150
116	311	120
117	176	68
118	142	55
119	62	24
120	171	66
121	122	47
1211	181	70
122	254	98
123	303	117
124	181	70
125	233	90
127	54	21
128	148	57
129	47	18
130	62	24
131	3061	1182
132	300	166
133	2238	864
134	730	282

TABLE 3
LOCAL AREA RUNOFF ESTIMATION

<u>LOCAL AREA</u>	<u>ESTIMATED ANNUAL RUNOFF</u> (mm)	<u>YIELD FACTOR AND ANALOG STATION</u>
103	45	111% (516)
104	104	40.5% (907)
105	104	50.5% (907)
107	104	9.0% (907)
108	71	106% (508)
109	45	24% (516)
110	50	169% (516)
111	48	3.3% (930)
112	54	99% (516)
115	48	12% (930)
116	48	67% (516)
117	45	36% (516)
118	71	2.8% (907)
119	71	1.2% (907)
120	71	3.4% (907)
121	31	17% (516)
1211	48	5.7% (930)
122	48	8.0% (930)
123	50	68% (516)
124	50	41% (516)
125	48	7.4% (930)
127	50	12% (516)
128	50	33% (516)
129	48	1.5% (930)
130	48	2.0% (930)
132	104	13% (907)
134	51	25% (930)

TABLE 4
LAKE AREAS

<u>LAKE</u>	<u>MODEL NUMBER</u>	<u>SURFACE AREA</u> (km ²)
Caribou	204	4.2
Wolf	206	30.8
Moose	210	40.4
Manatokan	212	4.5
Tucker	216	6.5
Burnt	218	4.4
May	219	3.2
Marie	220	36.2
Ethel	221	4.8
Muriel	223	68.2
Charlotte	224	10.9
Reita	227	12.5
Angling	228	5.9
Primrose	231	445
Cold	233	349

TABLE 5
SOURCE DATA FOR LAKE OUTLET RATING CURVES

<u>LAKE</u>	<u>DATA SOURCE</u>	<u>NOTES</u>
Caribou	NHCL survey	
Wolf	WSC rating curve	converted to Geodetic with AE lake records
Moose	NHCL survey	weir control
Manatokan	NHCL survey and AE measurements	beaver dam
Tucker	NHCL survey	weir control and beaver dam
Burnt	NHCL survey	beaver dams
May	NHCL survey and AE measurements	beaver dams
Marie	NHCL survey and AE rating curve	beaver dams
Ethel	SAEL rating curve extrapolated	curve for conditions subsequent to August 1978
Muriel	NHCL and AE surveys	control at Landry Lake outlet
Charlotte	NHCL survey	culvert control
Reita	NHCL survey	constructed channel
Angling	NHCL survey	
Primrose	NHCL survey	
Cold	SAEL rating curve	

Note: NHCL - Northwest Hydraulic Consultants Ltd.
 AE - Alberta Environment
 WSC - Water Survey of Canada
 SAEL - Stanley Associates Engineering Ltd.



Photo 1. Sand River at the outlet of Caribou Lake,
looking upstream.



Photo 2. Rock weir at the outlet of Moose Lake
(looking upstream).



Photo 3. Beaver dam at Manatokan Lake outlet
(looking upstream).

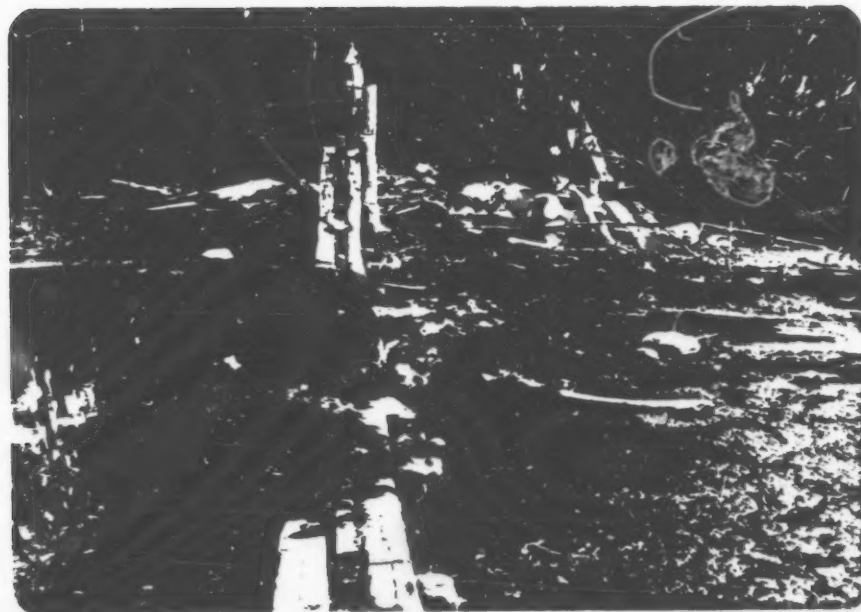


Photo 4. Weir at outlet of Tucker Lake.



Photo 5. Beaver dam at outlet of Burnt Lake.



Photo 6. May Lake outlet channel.



Photo 7. Beaver dam at Marie Lake outlet.

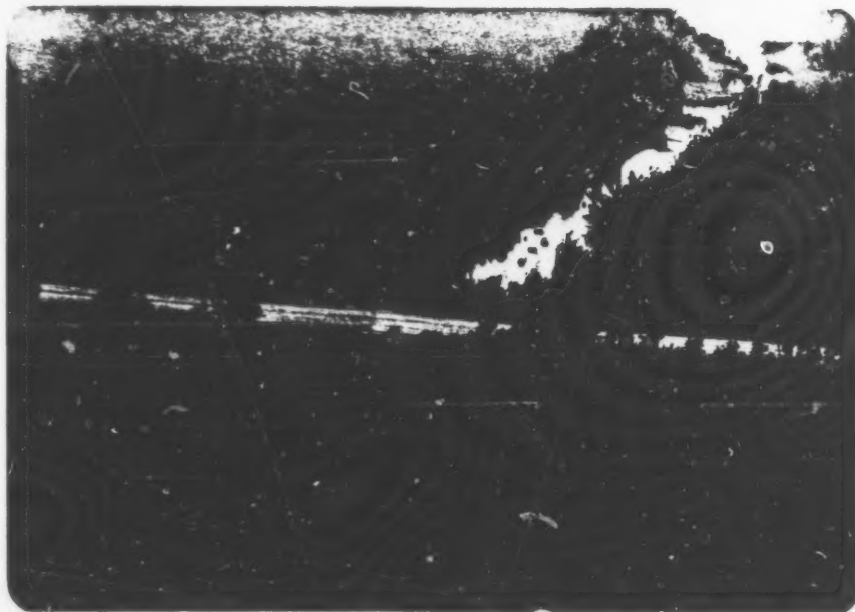


Photo 8. Landry Lake outlet (outlet control for Muriel Lake).



Photo 9. Culvert outlet control on Charlotte Lake.



Photo 10. Channelized outlet of Reita Lake.



Photo 11. Angling Lake outlet channel (Reita Creek).



Photo 12. Primrose Lake outlet channel looking downstream.
(Martineau River)

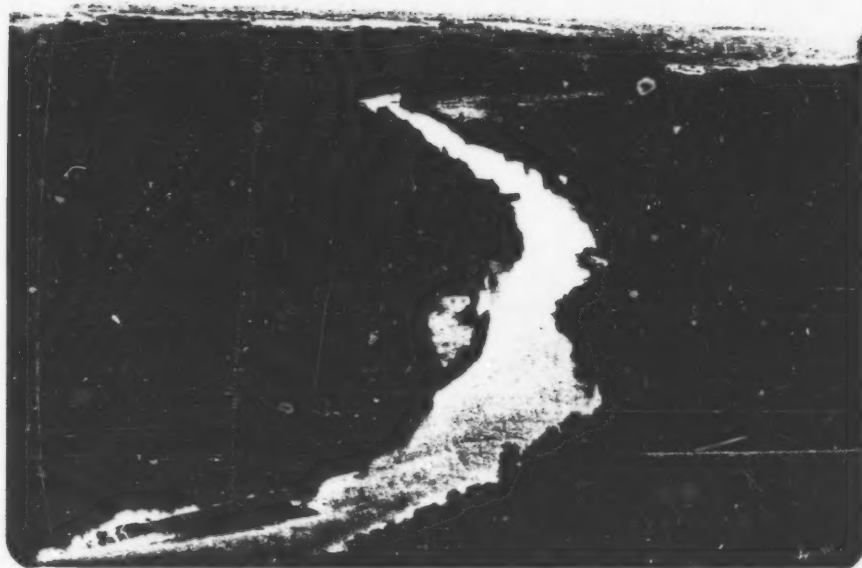
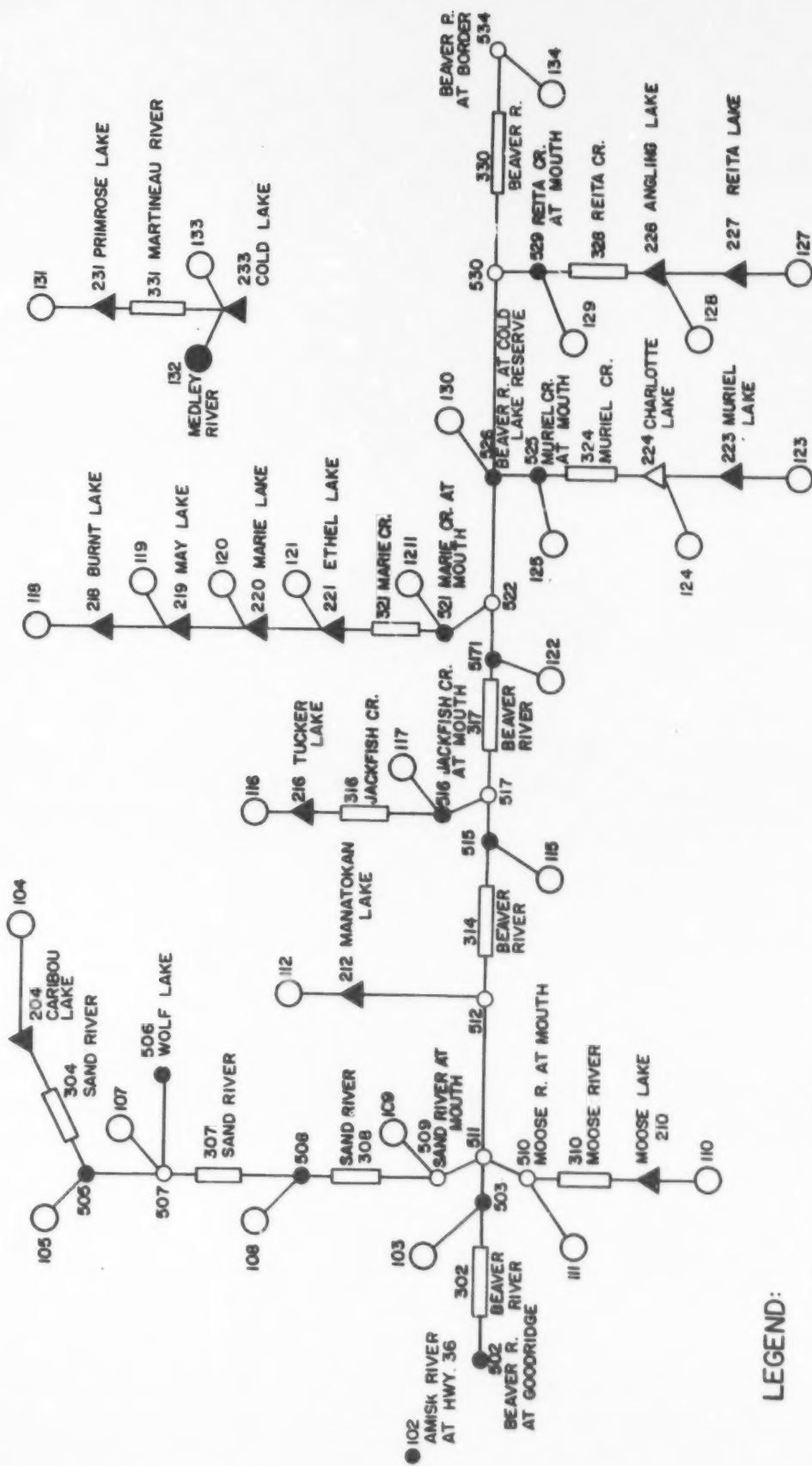


Photo 13. Beaver River downstream of Sand River.





LEGEND:

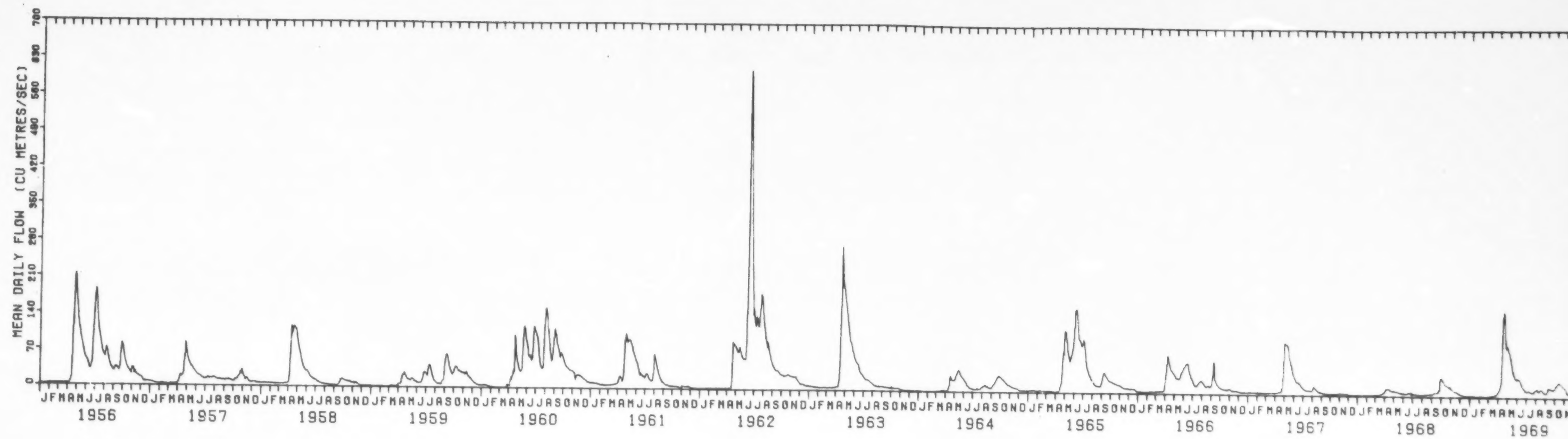
- SUM POINT (CONFLUENCE OR NODE) - ○ 509
- LOCAL INFLOW - - - - - ○ 112
- LAKE - - - - - △ 212
- CHANNEL ROUTING REACH - - - - - □ 308

NOTE: DESIGNATED STUDY SITES ARE DARKENED.

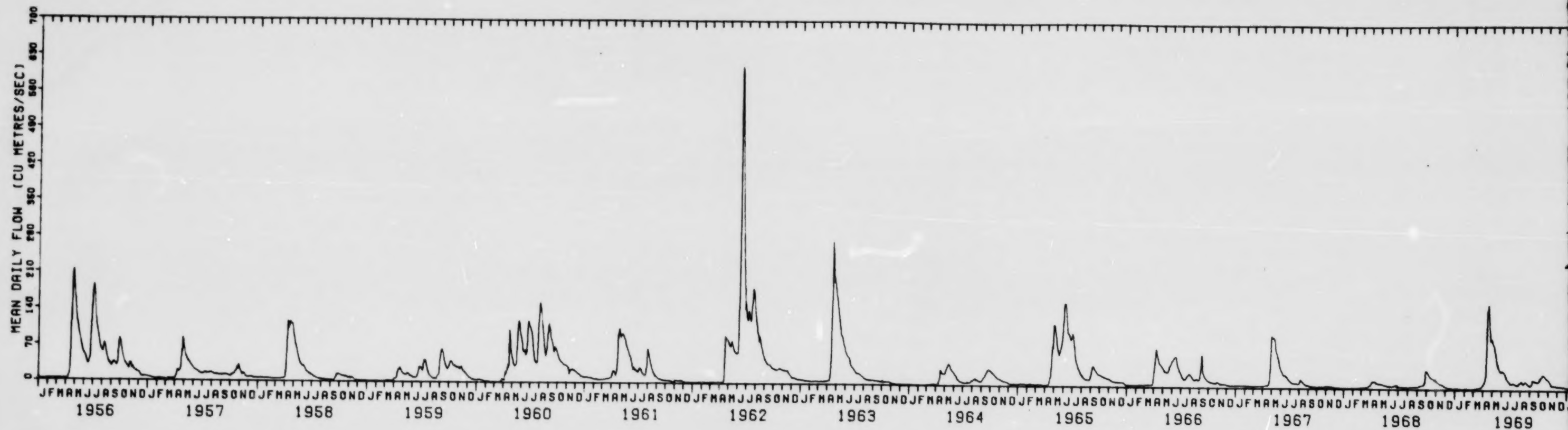
FIGURE 3

WATERSHED SCHEMATIC

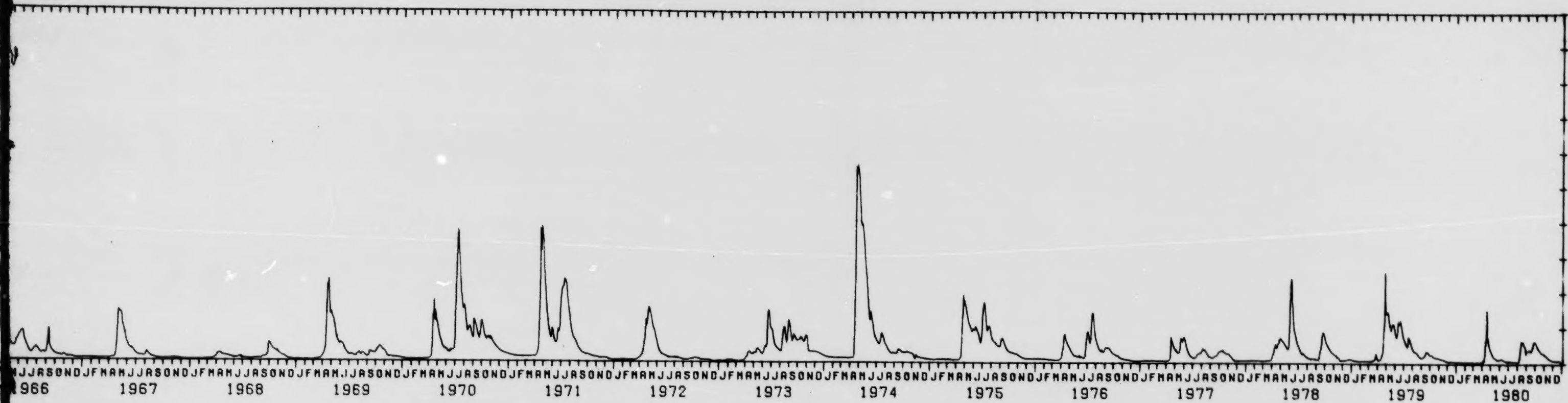




526 BEAVER R. AT W.S.C. COLD

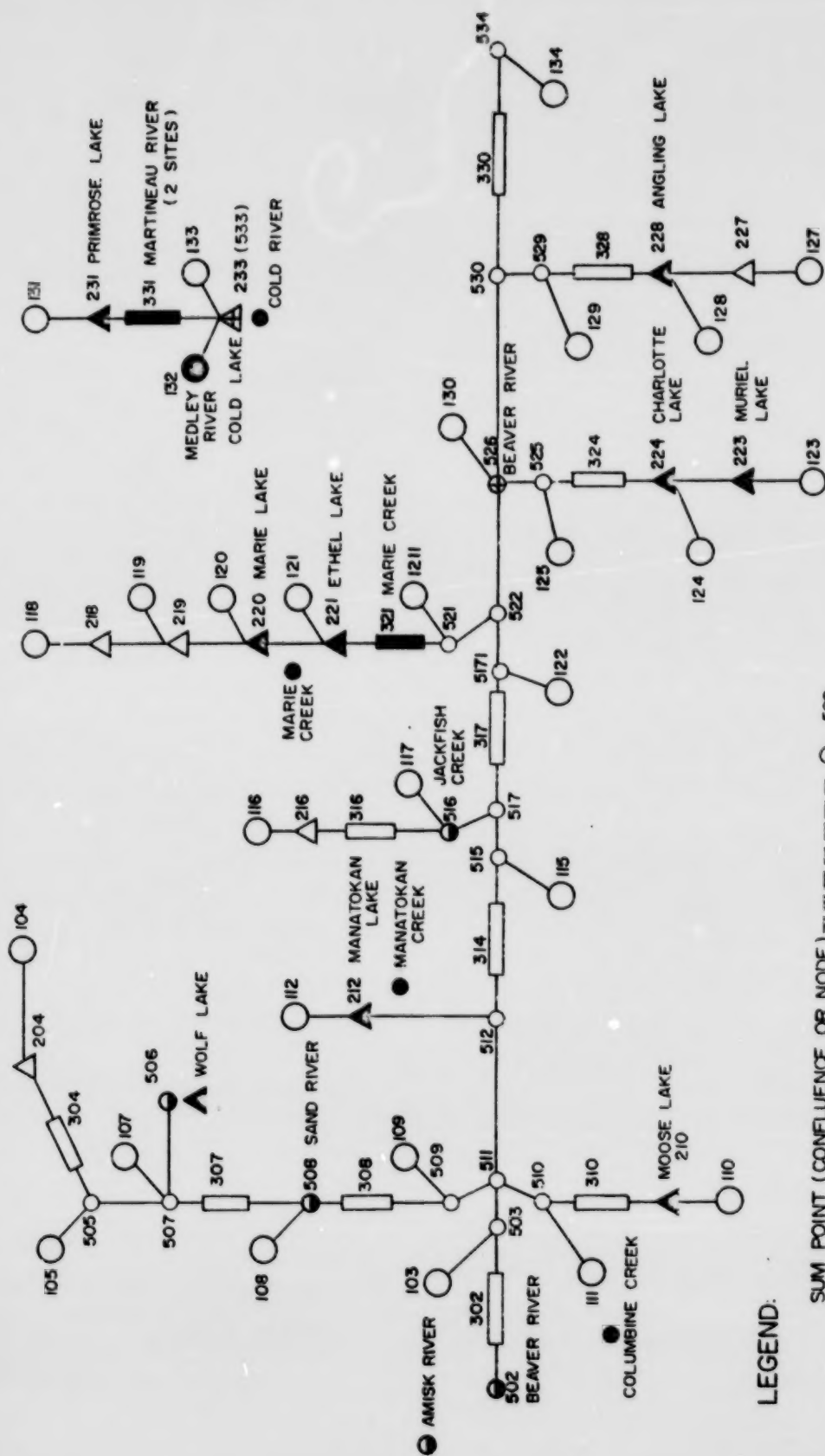


526 BEAVER R. AT W.S.C. COLD



W.S.C. COLD LAKE GAUGE 06AD006

FIGURE 4
DISCHARGE HYDROGRAPH
BEAVER RIVER AT W.S.C.
COLD LAKE GAUGE
(STUDY POINT 326)



LEGEND:

- SUM POINT (CONFLUENCE OR NODE) --- ○ 509
- LOCAL INFLOW --- ○ 112
- LAKE --- △ 212
- CHANNEL ROUTING REACH --- □ 308
- FIRST-ORDER STATIONS (CONTINUOUS 1956-1980) --- ⊕
- SECOND-ORDER STATIONS (CONTINUOUS OR SEASONAL, SINCE 1970) --- ●
- THIRD-ORDER STATIONS (SHORT TERM, INTERMITTENT) --- ●

FIGURE 7

LOCATION OF HYDROMETRIC STATIONS

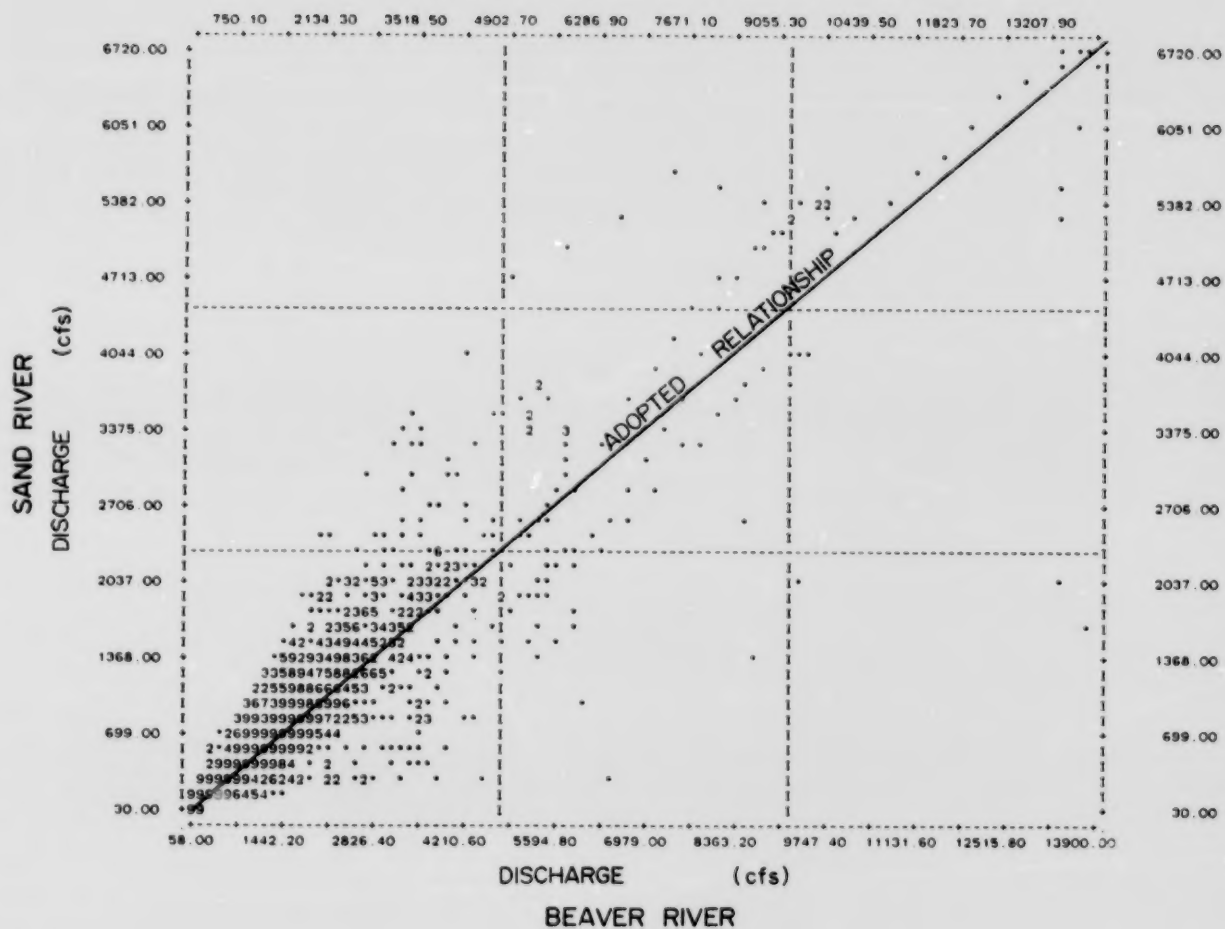
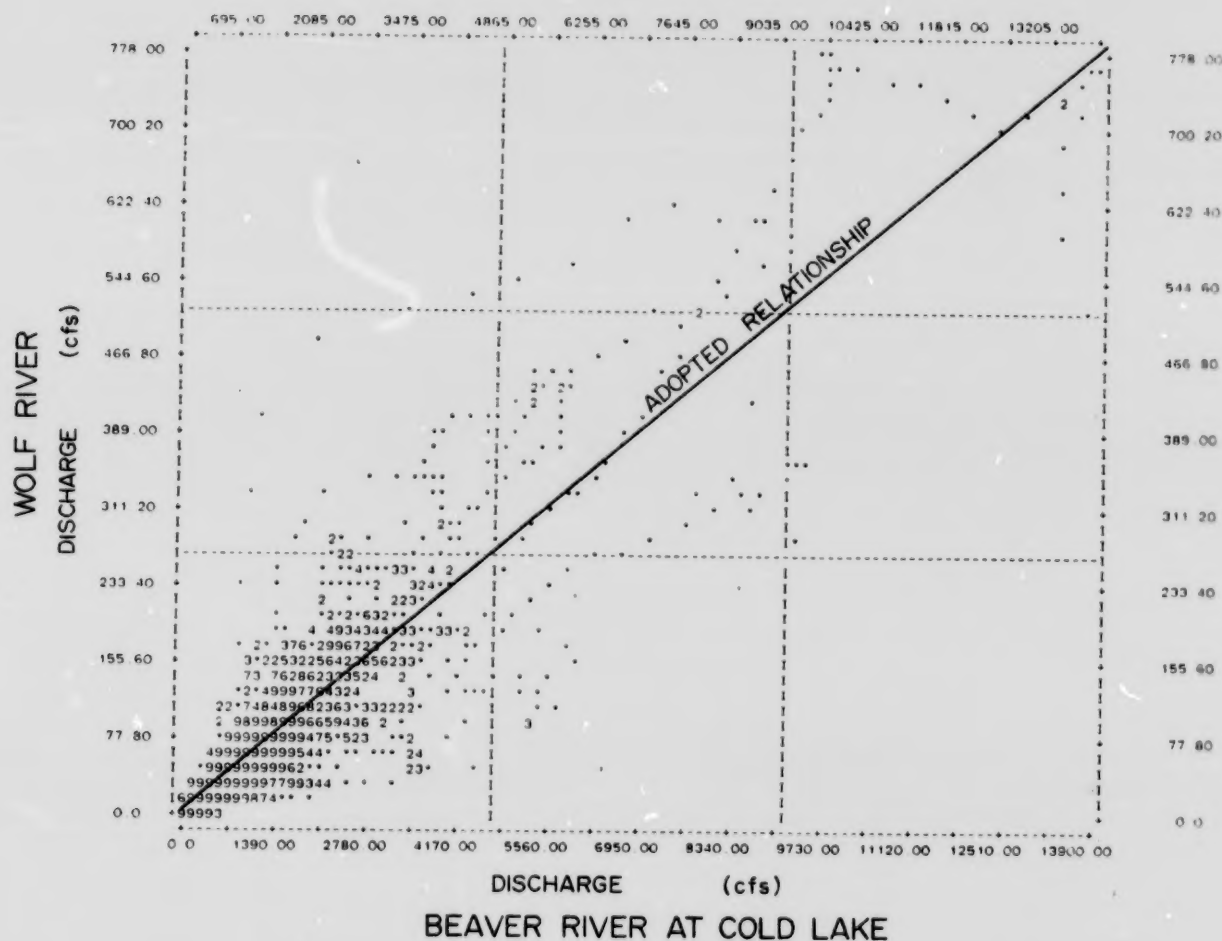


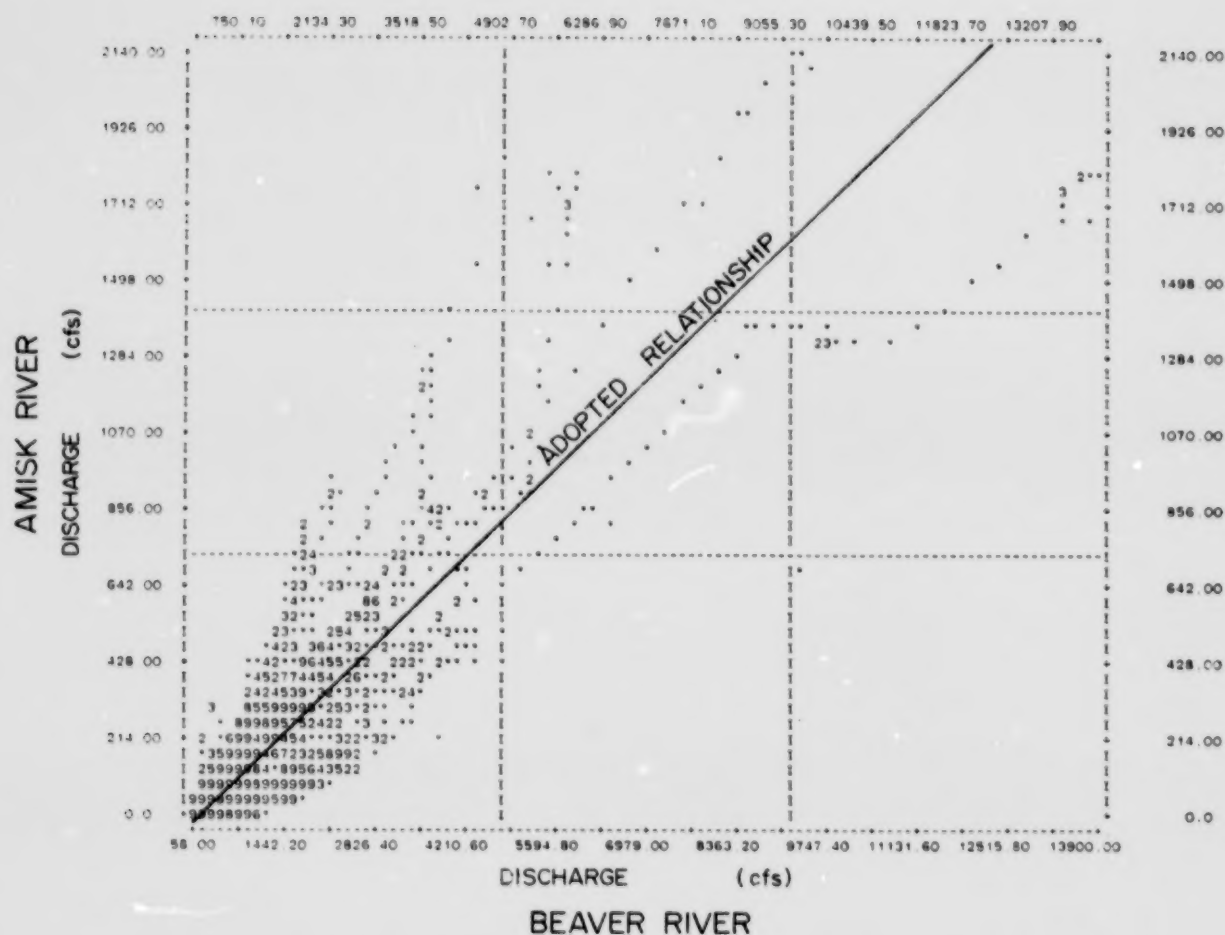
FIGURE 8
CORRELATION OF DAILY DISCHARGES
SAND RIVER vs. BEAVER RIVER

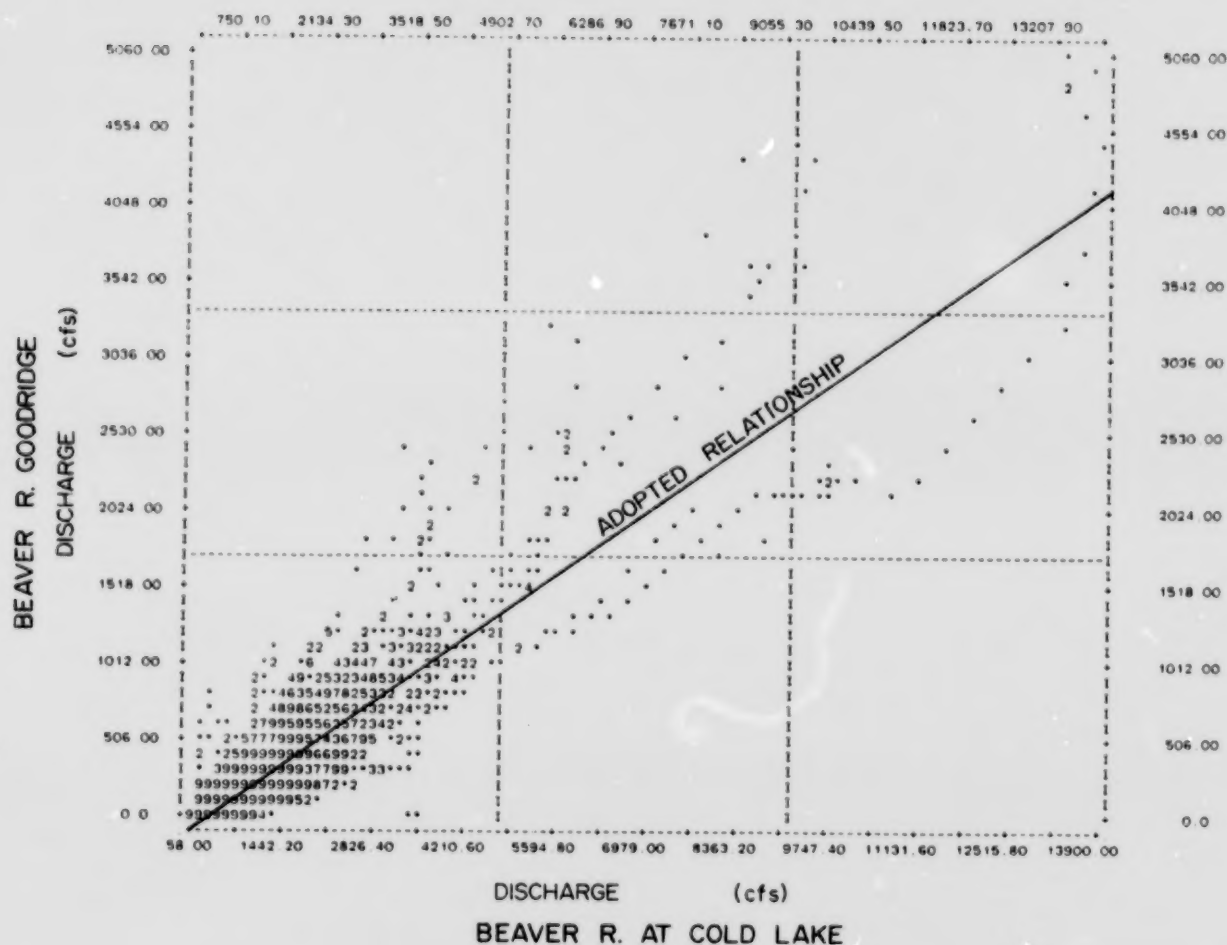


NOTE:
BEAVER RIVER DISCHARGES ARE LAGGED 5 DAYS.

FIGURE 9

CORRELATION OF DAILY DISCHARGES
WOLF RIVER vs. BEAVER RIVER





NOTE: BEAVER RIVER AT GOODRIDGE DISCHARGES ARE LAGGED 2 DAYS.

FIGURE 11

CORRELATION OF DAILY DISCHARGES
BEAVER R. (GOODRIDGE) vs. BEAVER R. (COLD LAKE)

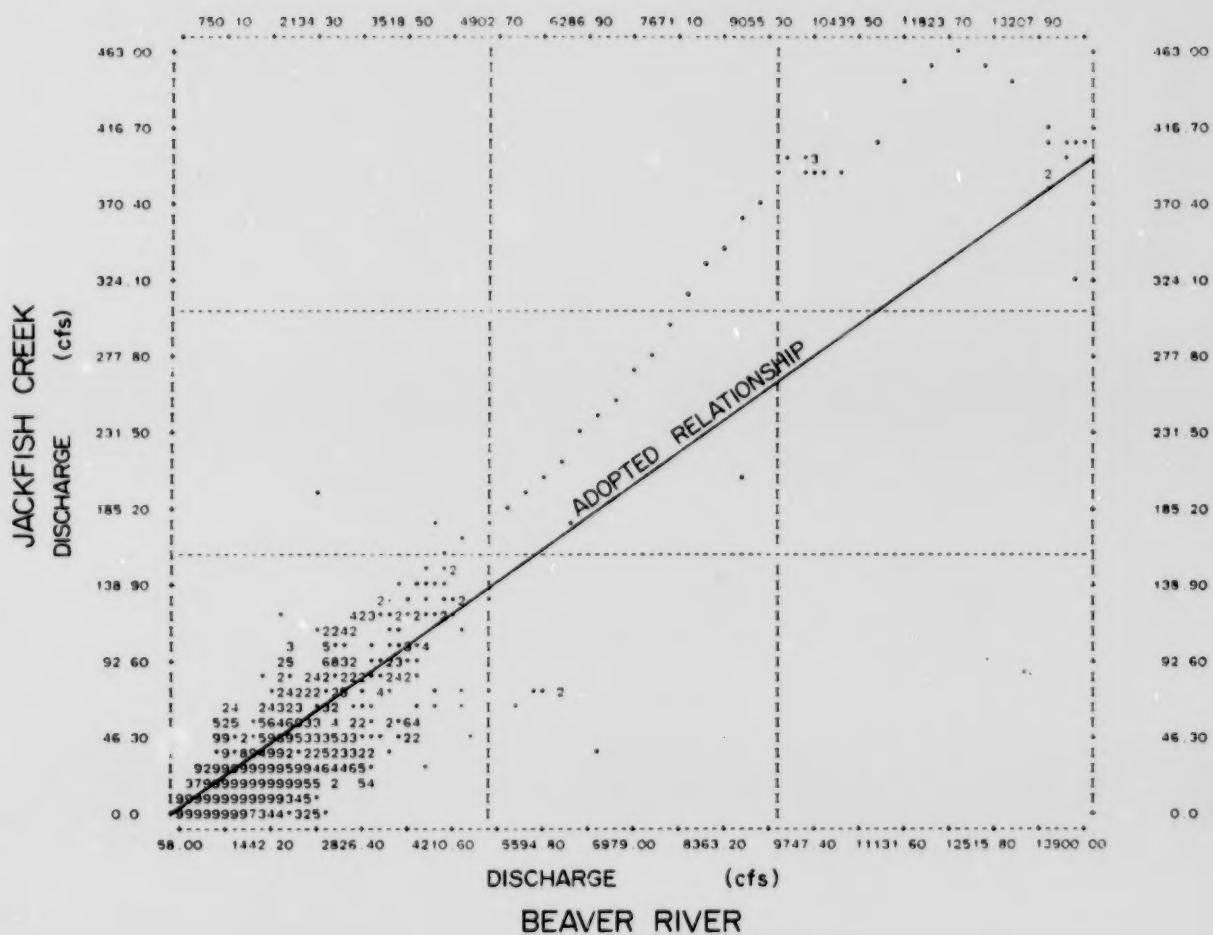
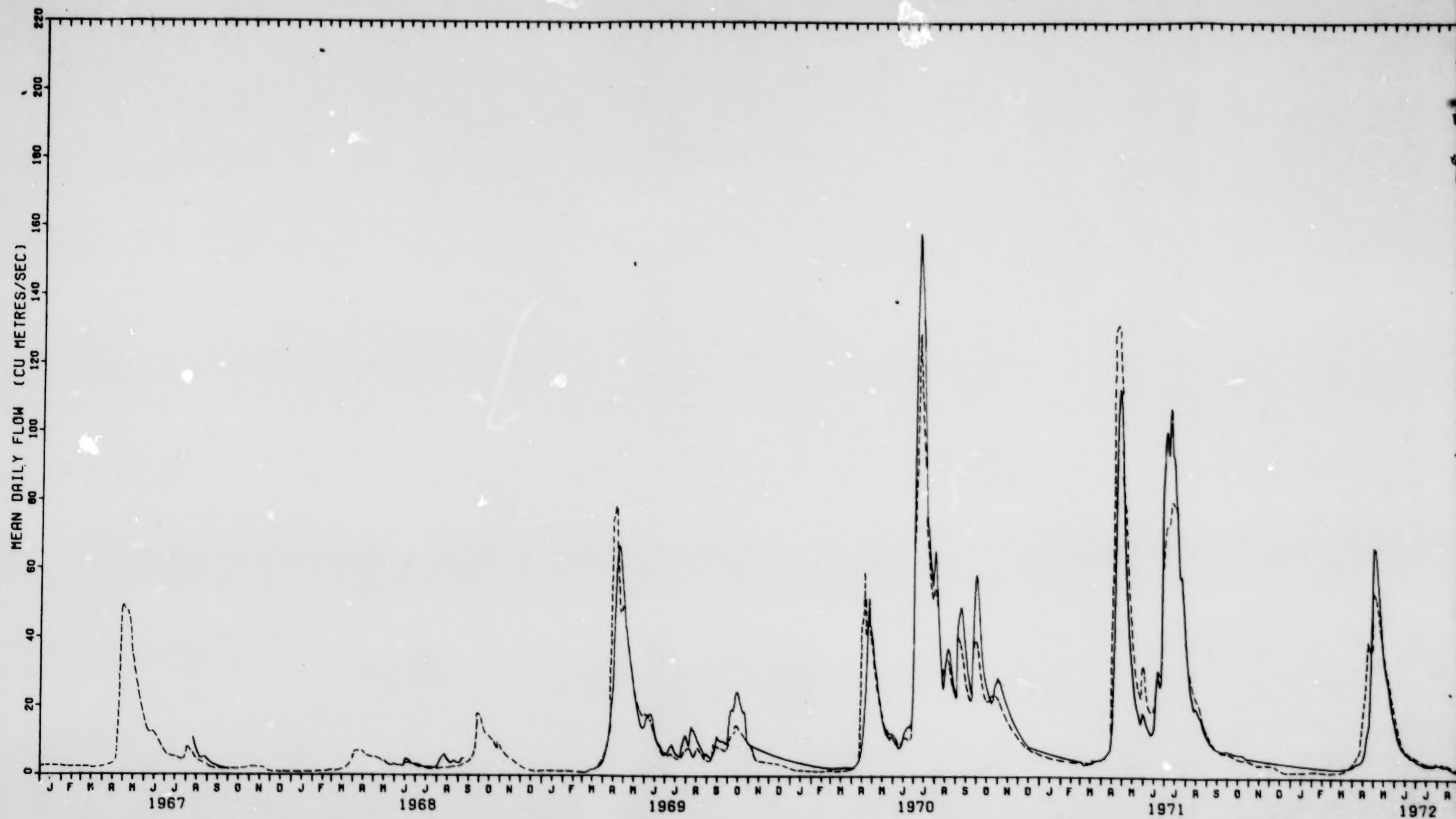
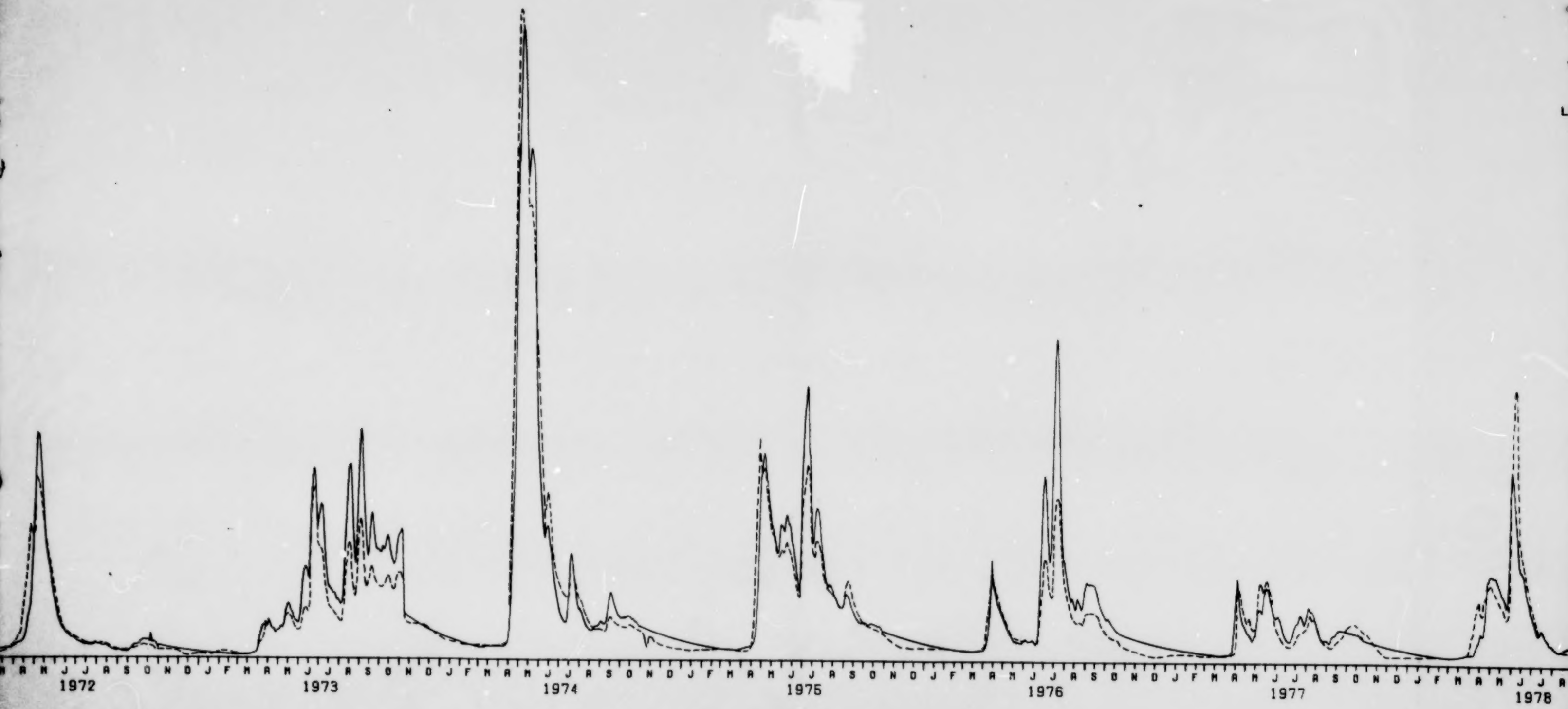


FIGURE 12

CORRELATION OF DAILY DISCHARGES
JACKFISH CREEK vs. BEAVER RIVER





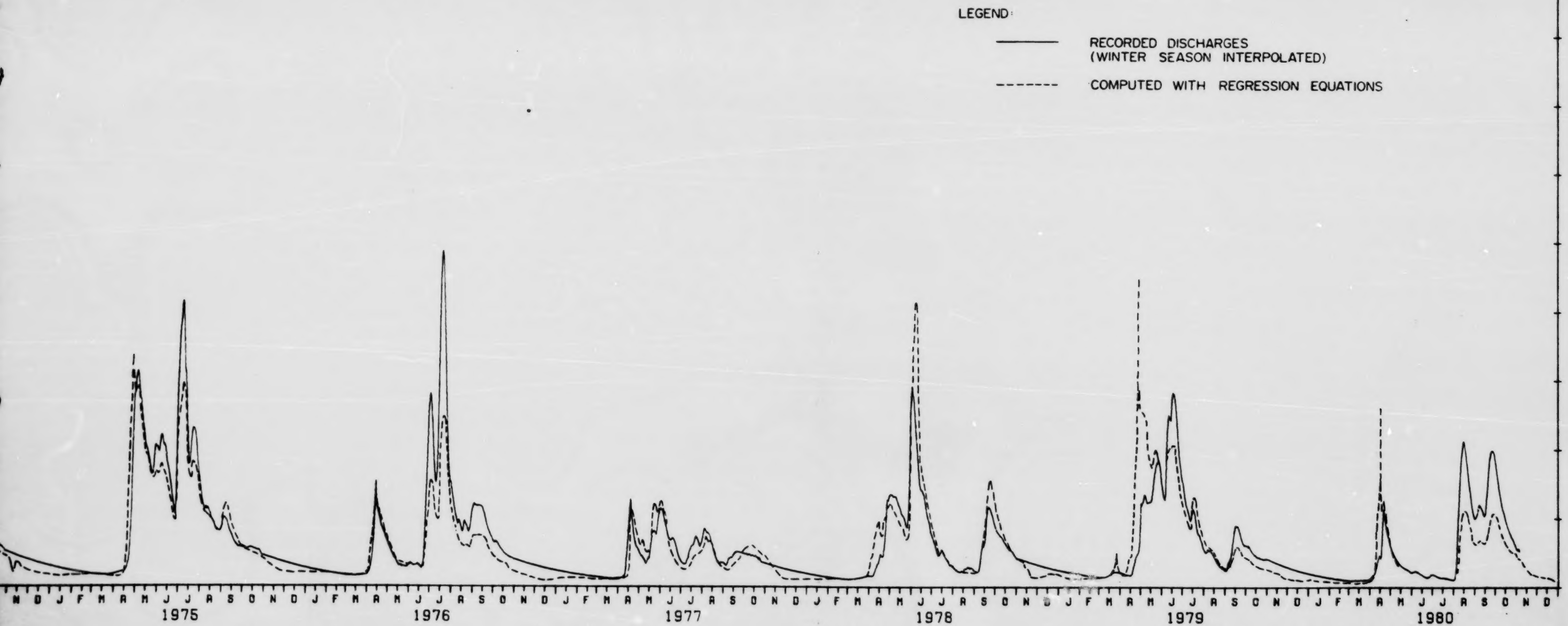
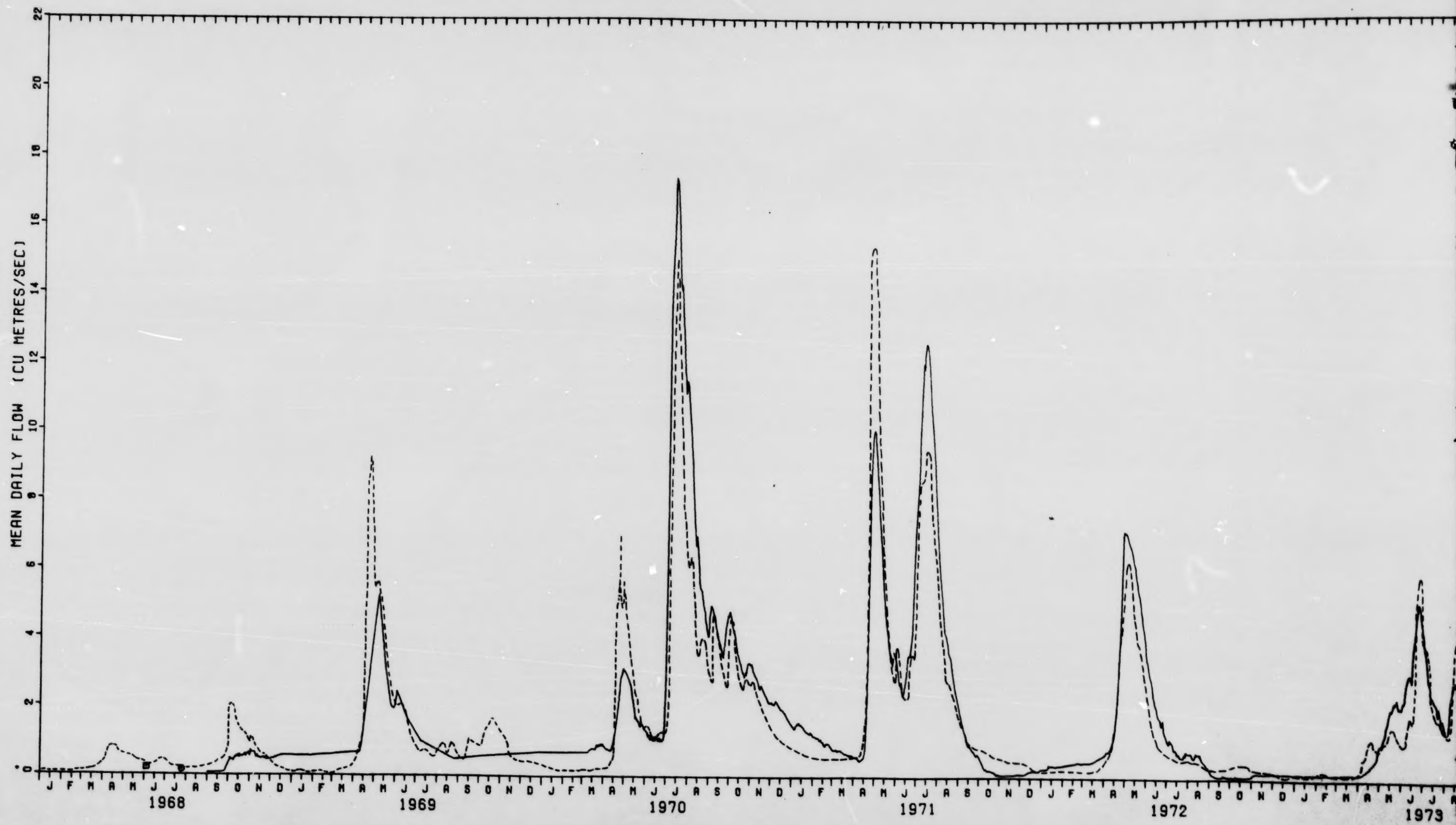
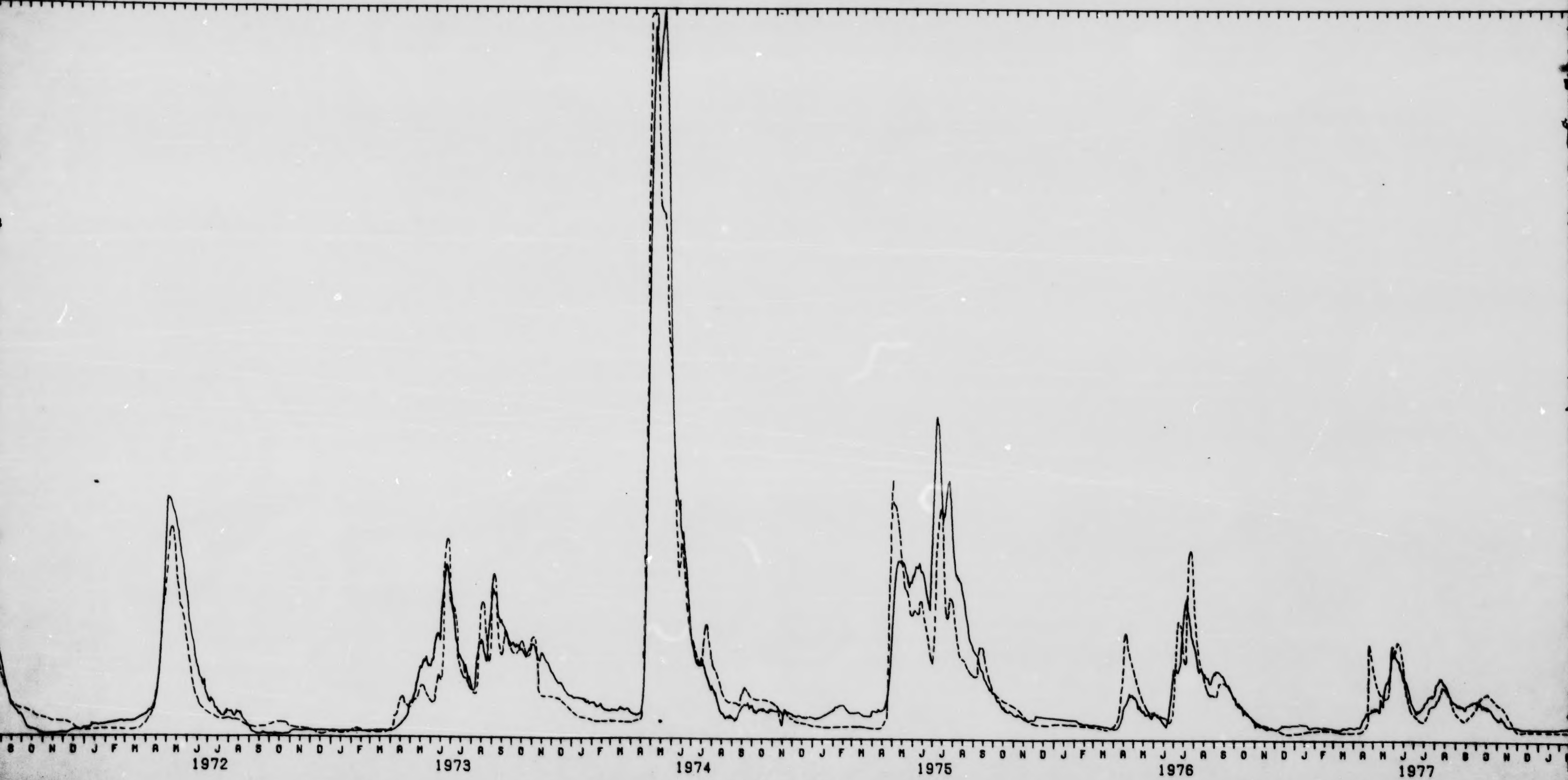


FIGURE 13
COMPARISON OF RECORDED DISCHARGES
WITH RESULTS OF REGRESSION ANALYSIS
SAND RIVER





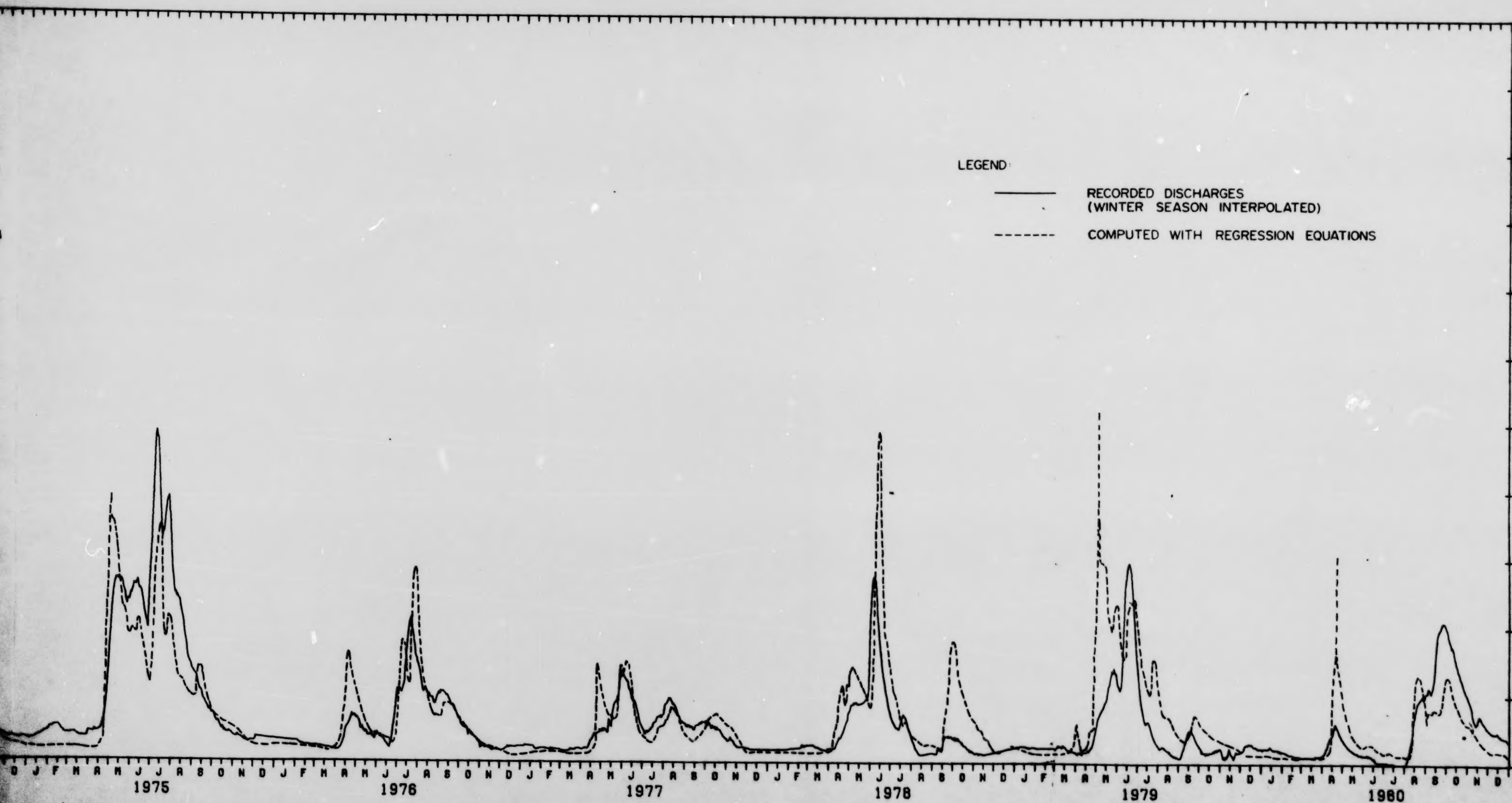
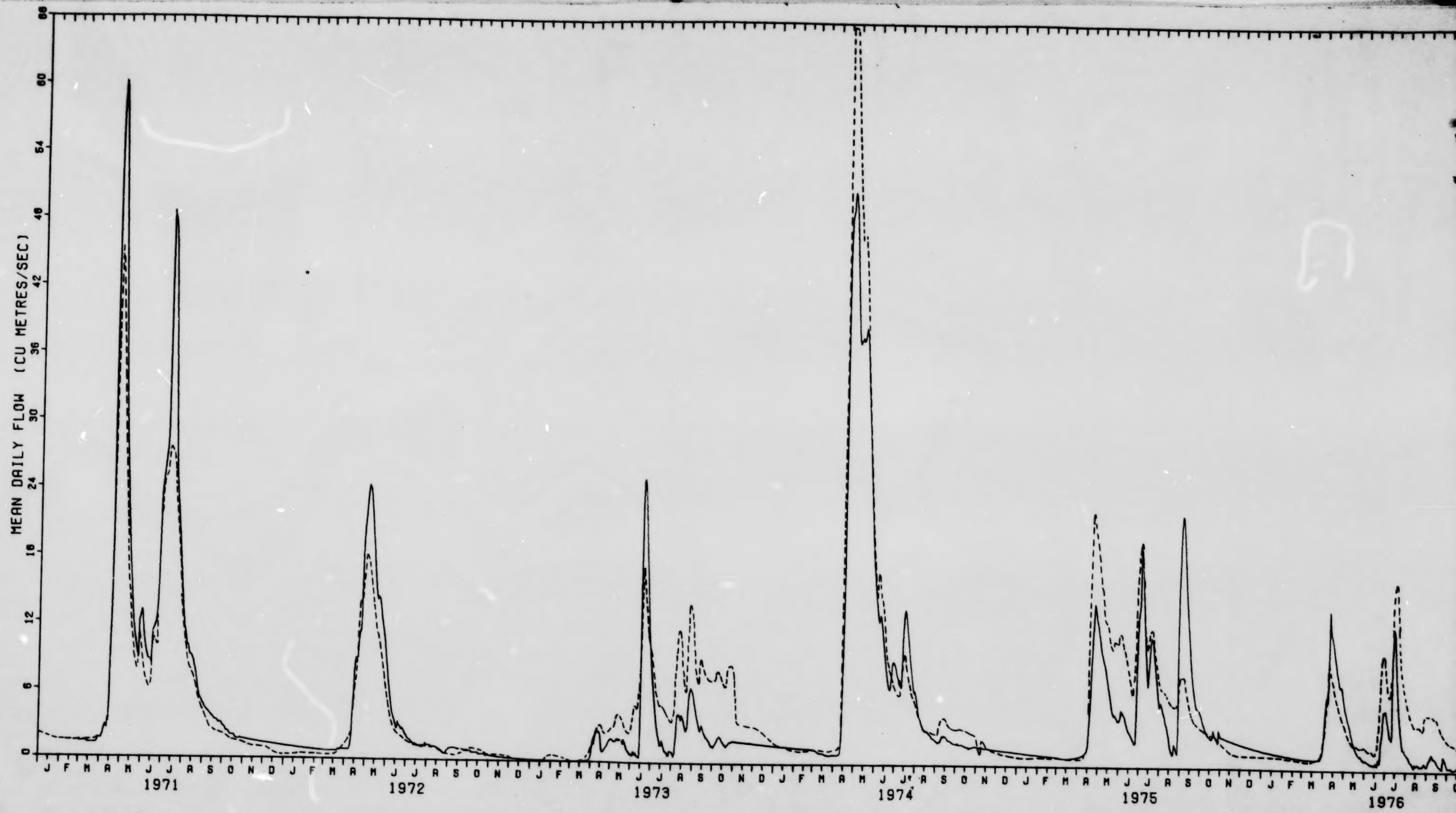


FIGURE 14
COMPARISON OF RECORDED DISCHARGES
WITH RESULTS OF REGRESSION ANALYSIS
WOLF RIVER



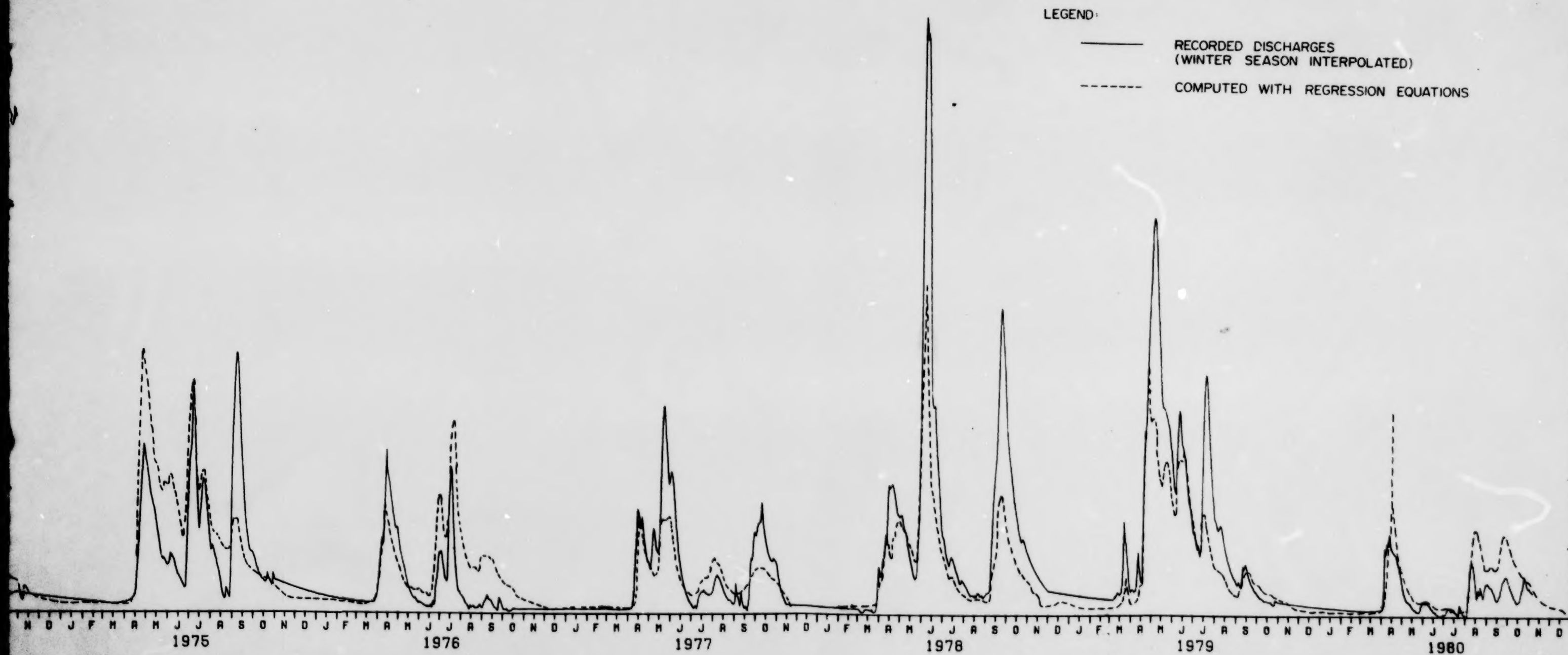
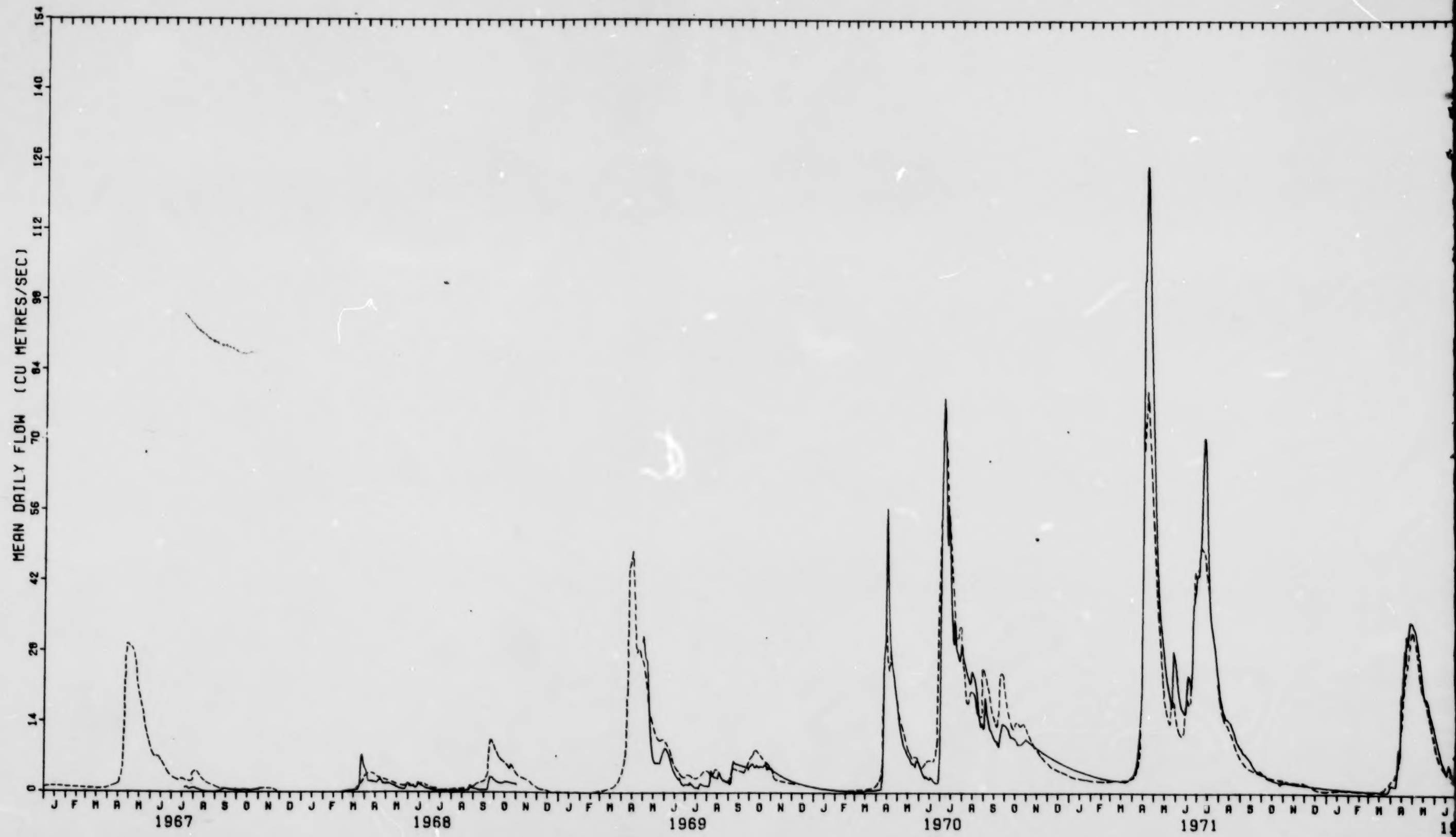
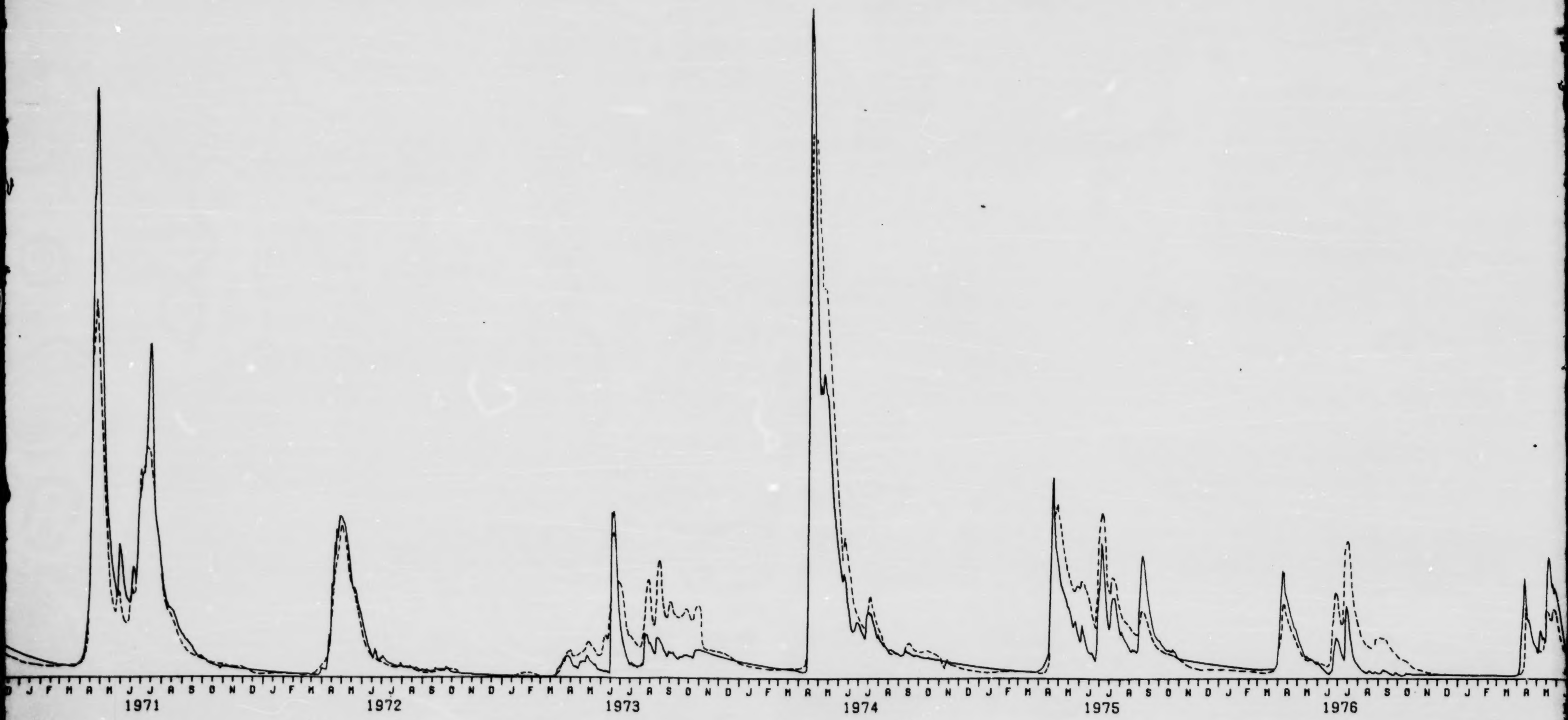


FIGURE 15
COMPARISON OF RECORDED DISCHARGES
WITH RESULTS OF REGRESSION ANALYSIS
AMISK RIVER





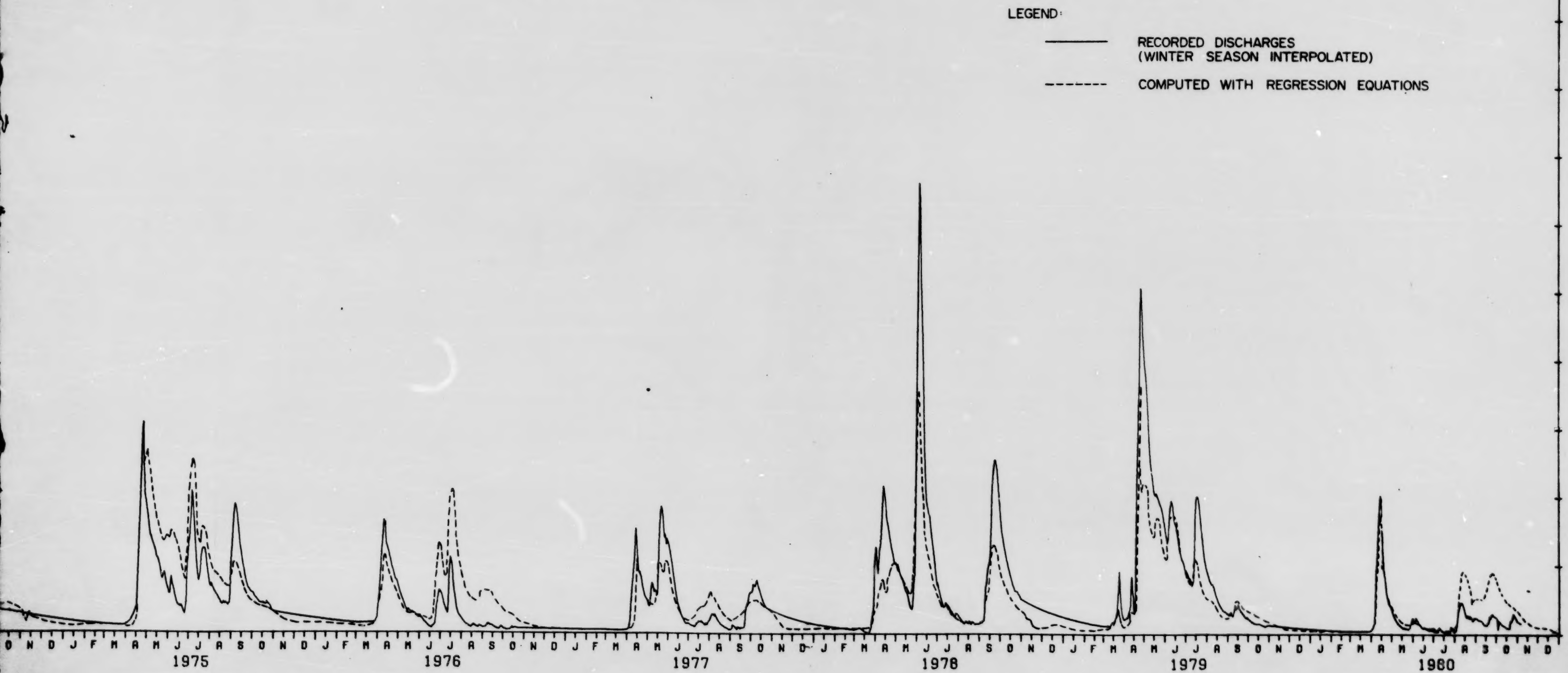
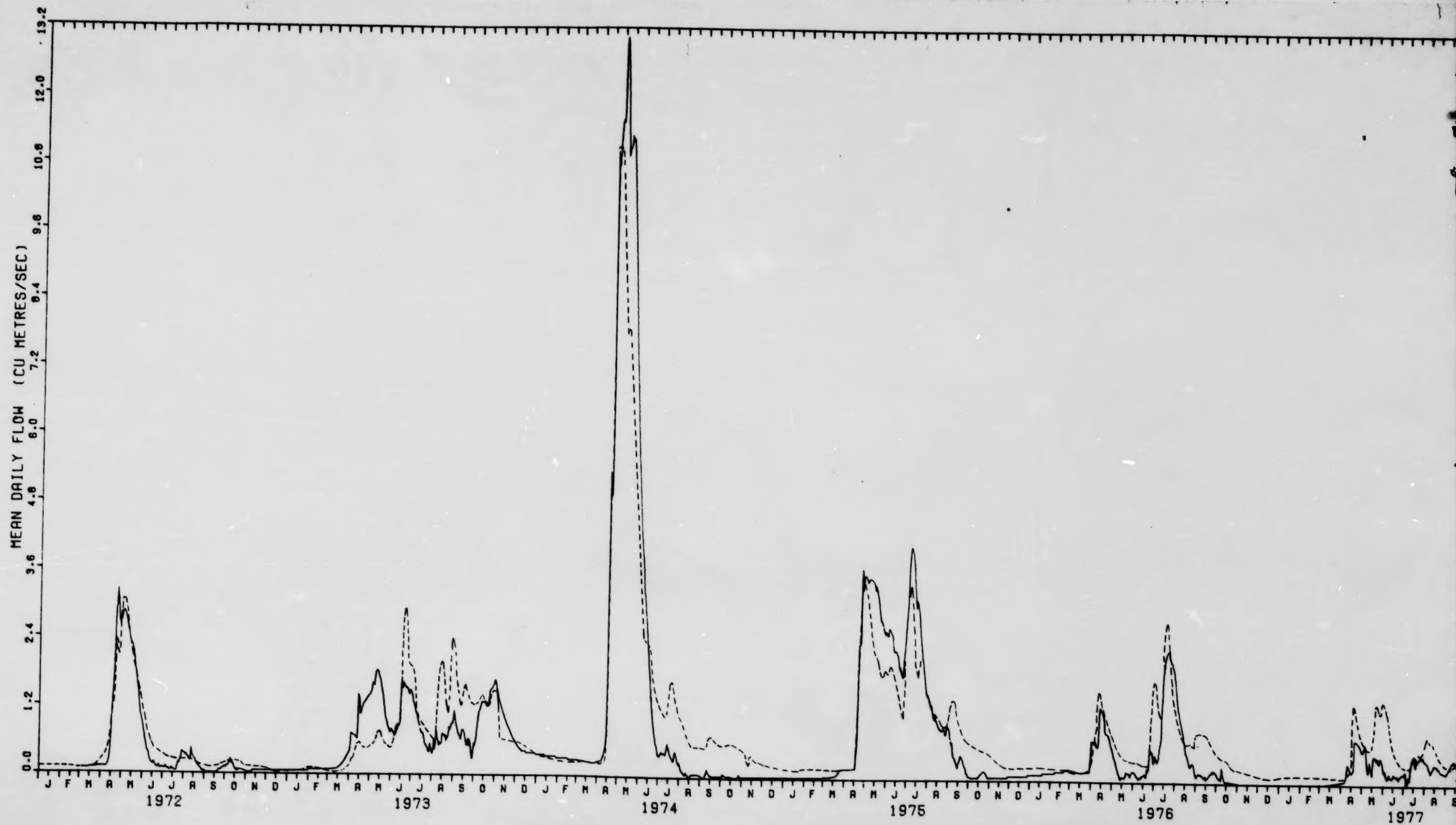


FIGURE 16
COMPARISON OF RECORDED DISCHARGES
WITH RESULTS OF REGRESSION ANALYSIS
BEAVER RIVER AT GOODRIDGE



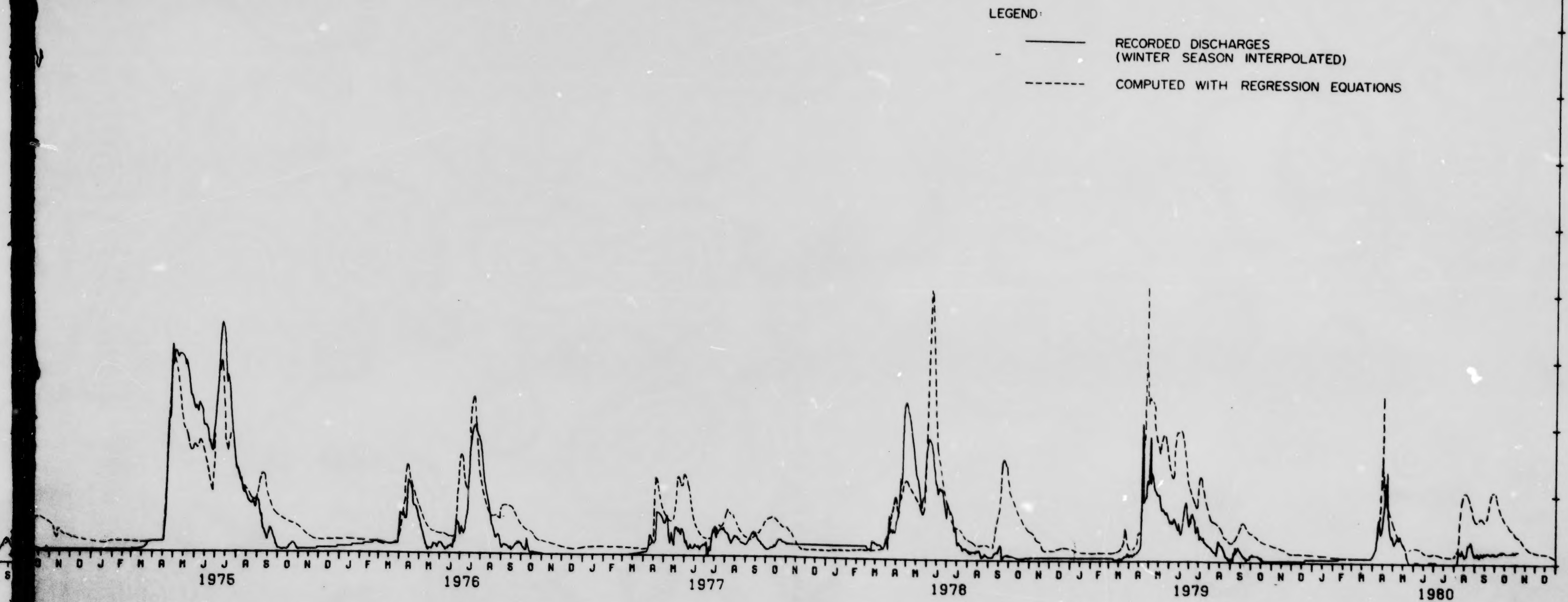
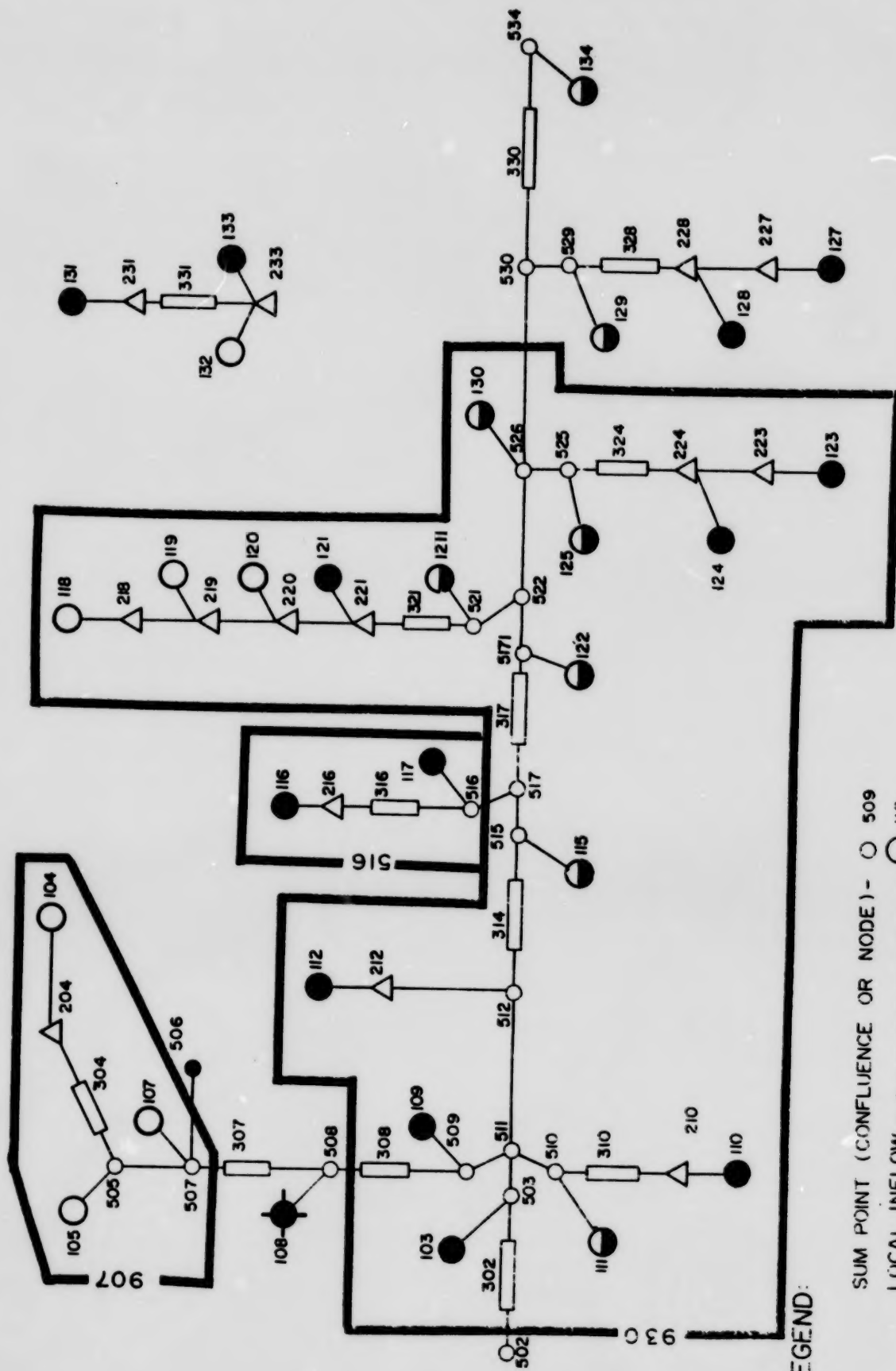


FIGURE 17
COMPARISON OF RECORDED DISCHARGES
WITH RESULTS OF REGRESSION ANALYSIS
JACKFISH CREEK

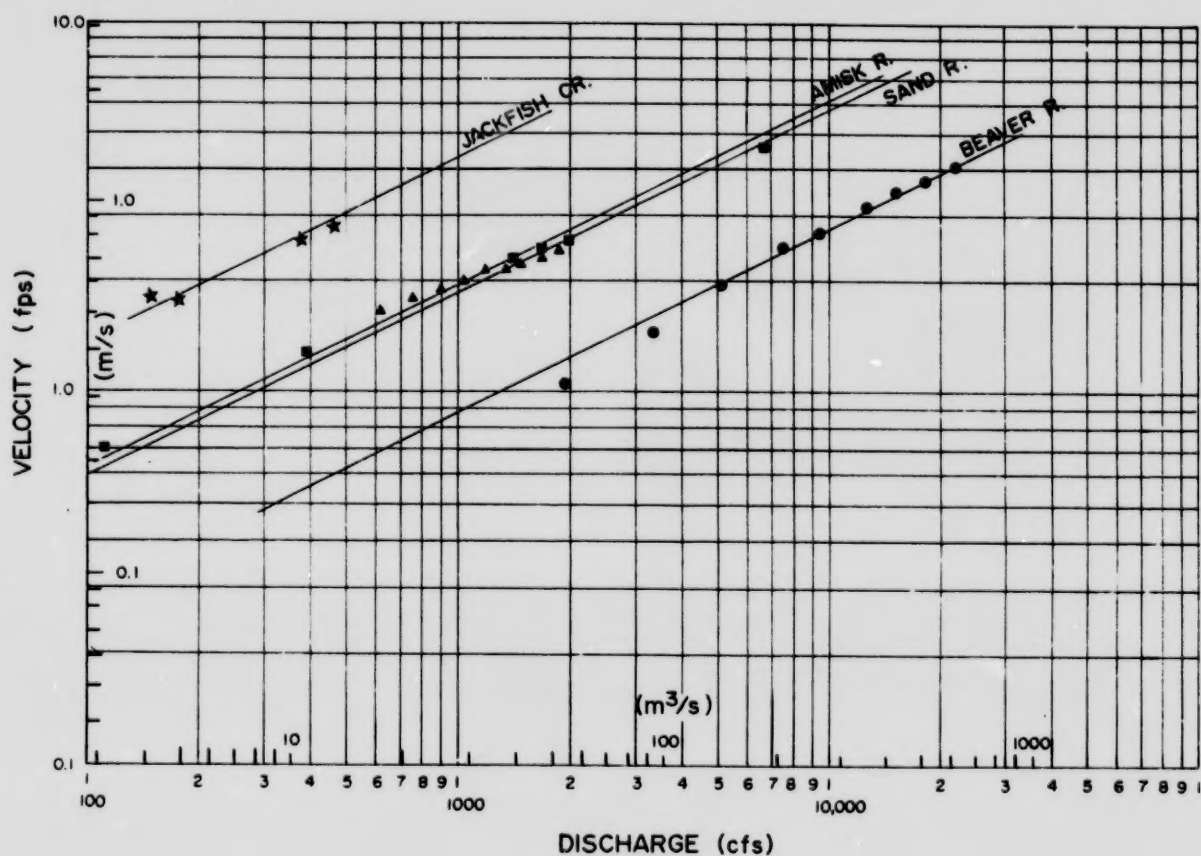


LEGEND:

- SUM POINT (CONFLUENCE OR NODE) - ○ 509
- LOCAL INFLOW - ○ 112
- LAKE - △ 212
- CHANNEL ROUTING REACH - --- 308
- SUB-BASIN BOUNDARY - - - - -

NOTE COLOURED CIRCLES DESIGNATE LOCAL DRAINAGE AREAS OF COMPARABLE RUNOFF CHARACTERISTICS

FIGURE 18
CLASSIFICATION OF
LOCAL DRAINAGE AREAS



LEGEND:

- ★ JACKFISH CREEK
 $V = 0.136 Q^{0.50}$
- ▲ AMISK RIVER
 $V = 0.062 Q^{0.50}$
- SAND RIVER
 $V = 0.059 Q^{0.50}$
- BEAVER RIVER & COLD LAKE
 $V = 0.028 Q^{0.50}$

NOTE:

EQUATIONS ARE GIVEN IN
IMPERIAL UNITS.

FIGURE 19

RELATIONSHIP OF MEAN
FLOW VELOCITY TO DISCHARGE

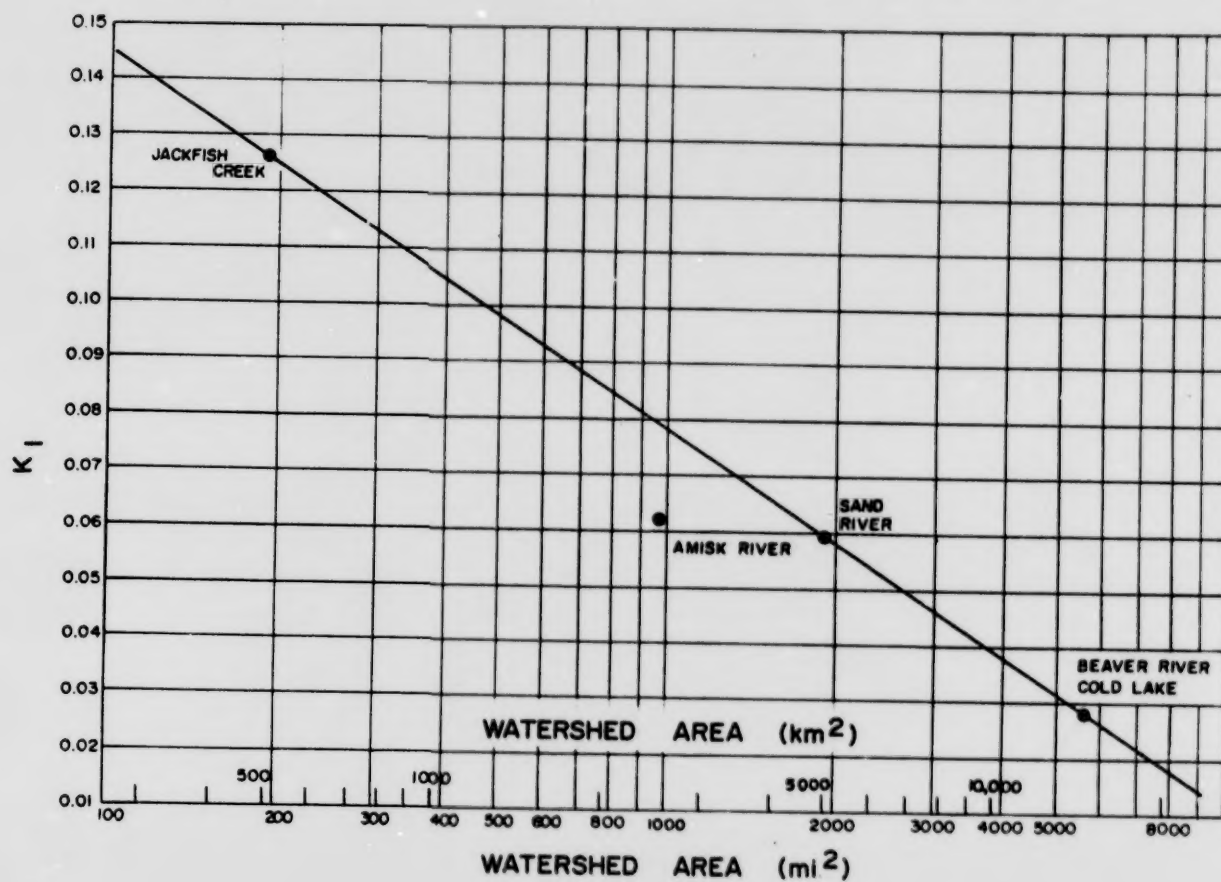


FIGURE 20

K_1 vs. WATERSHED AREA

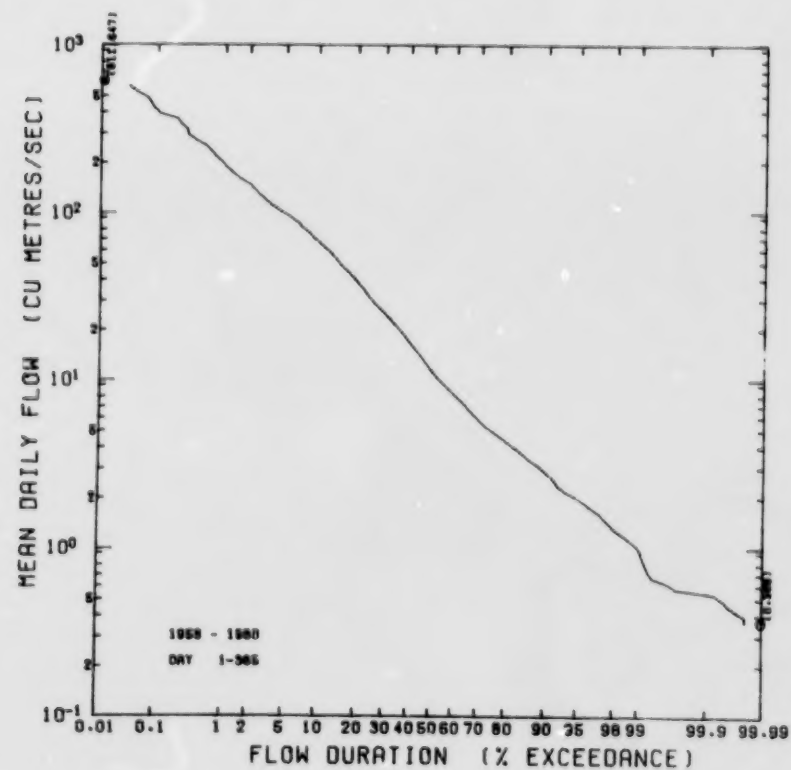
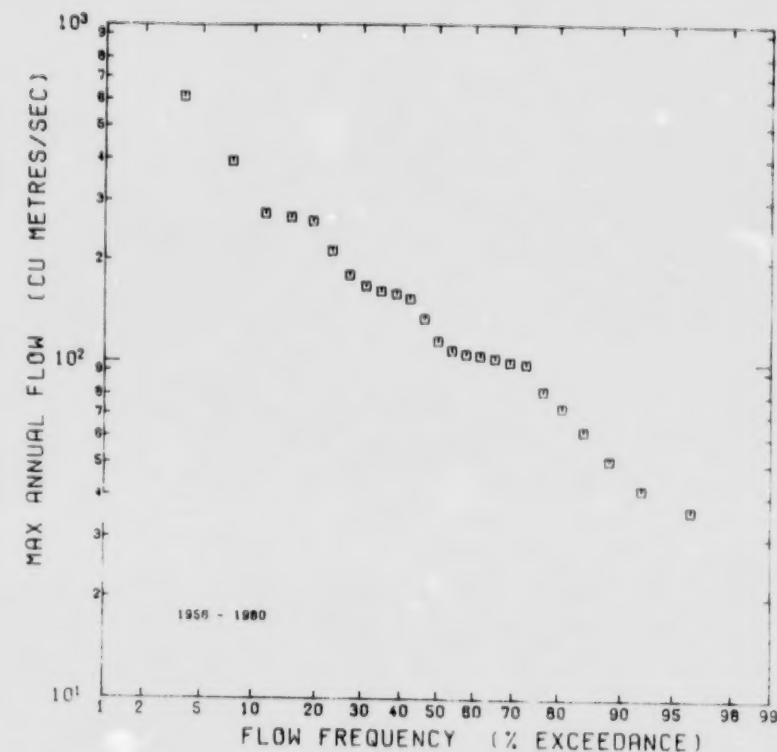
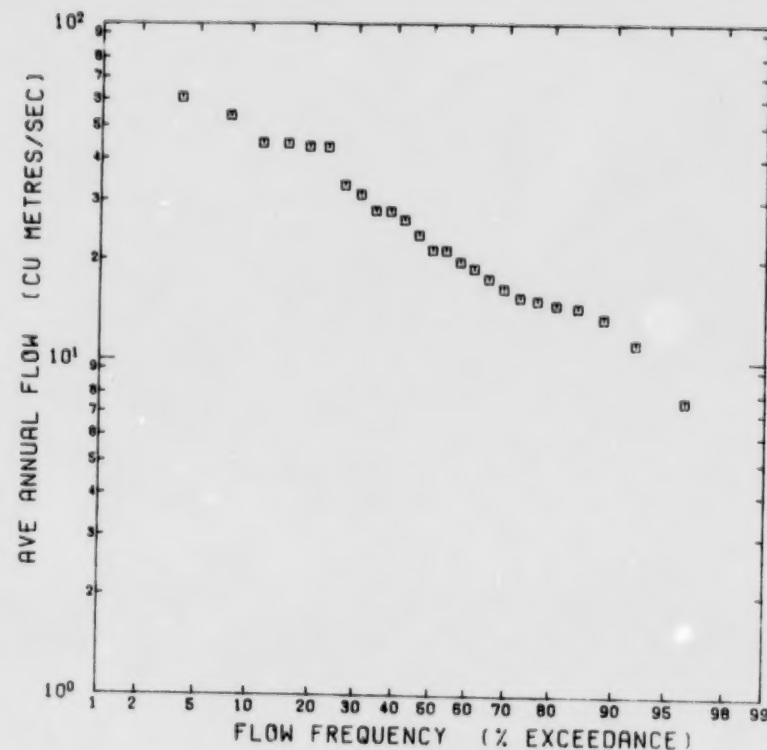
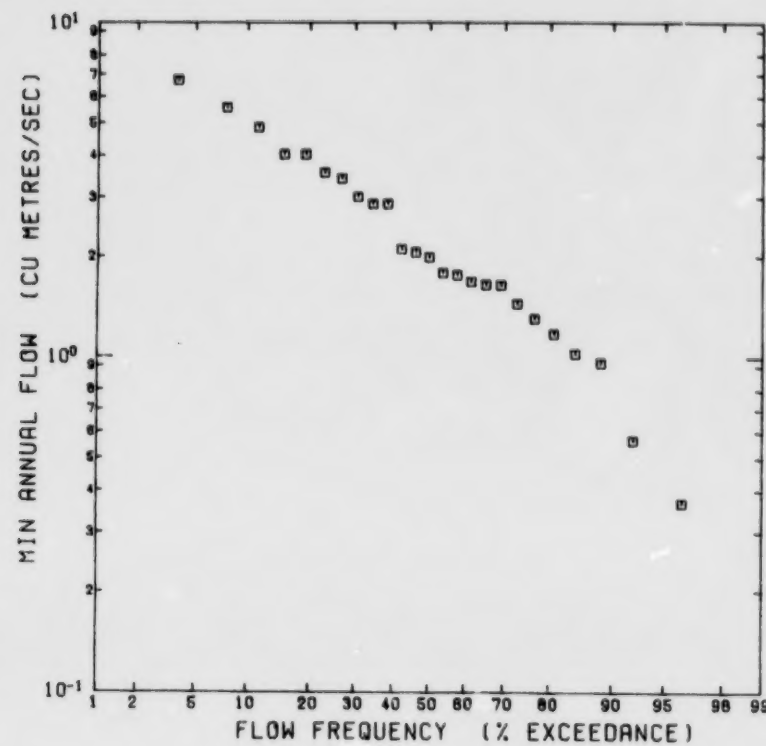
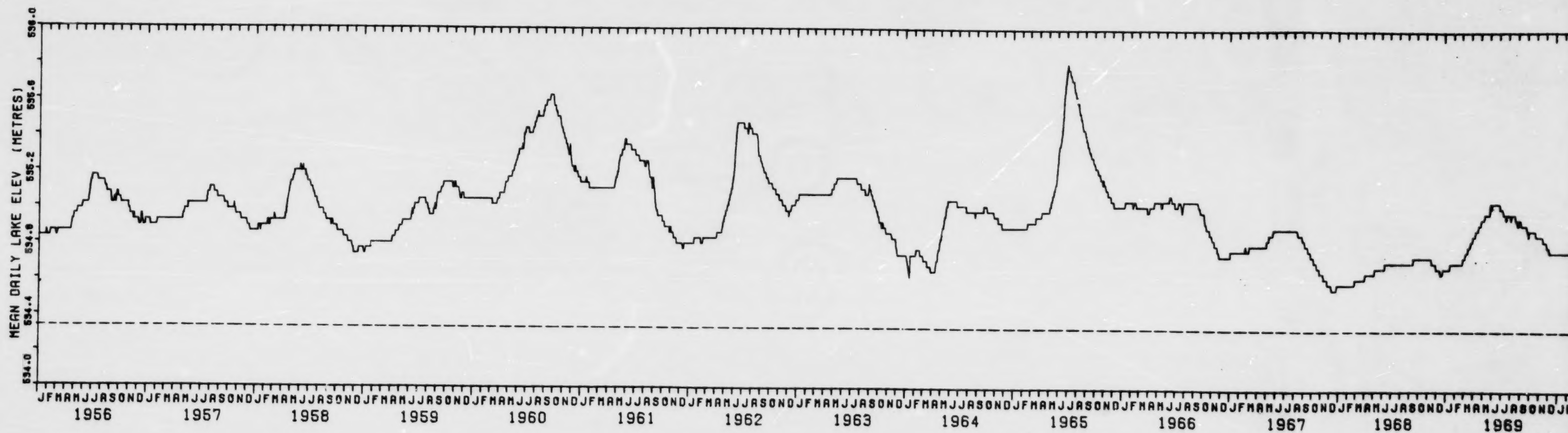


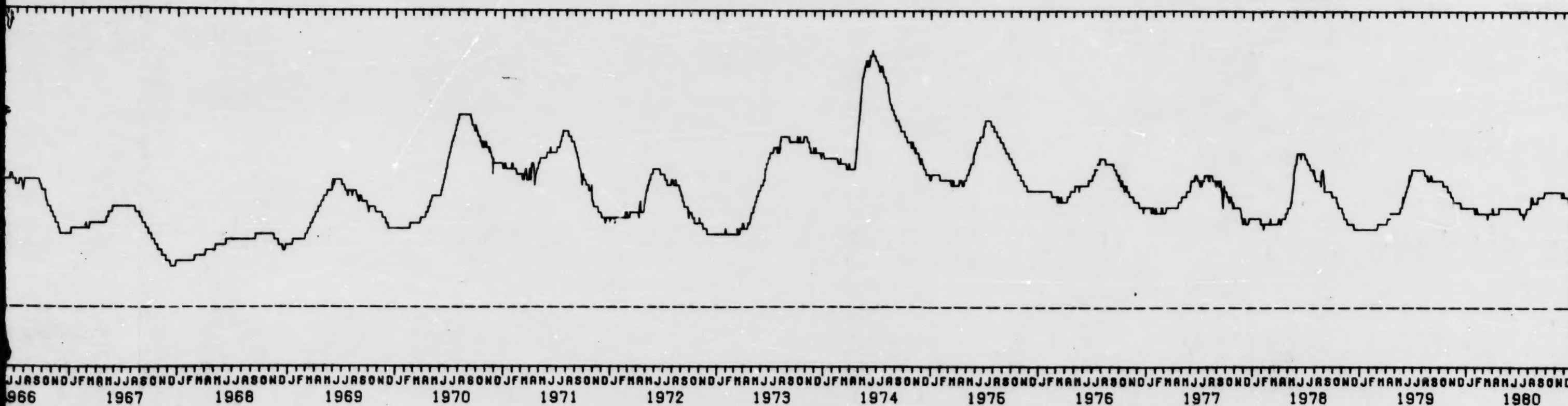
FIGURE 21

FLOW FREQUENCY
AND FLOW DURATION CURVES

BEAVER RIVER AT W.S.C.
COLD LAKE GAUGE 06AD006
(STUDY POINT 526)



533 COLD LAKE WATER LEVELS



ER LEVELS

FIGURE 22
STAGE HYDROGRAPH
COLD LAKE
(STUDY POINT 533)

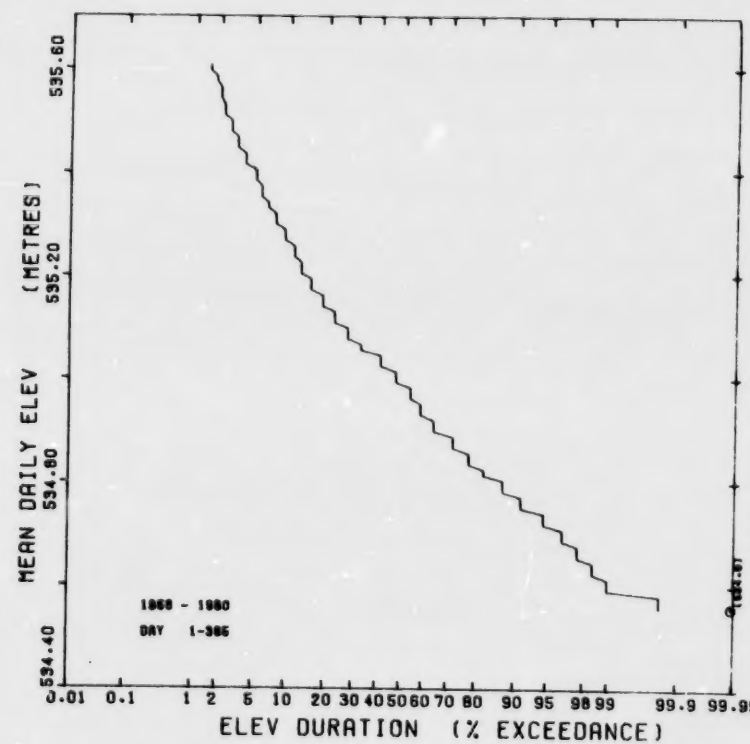
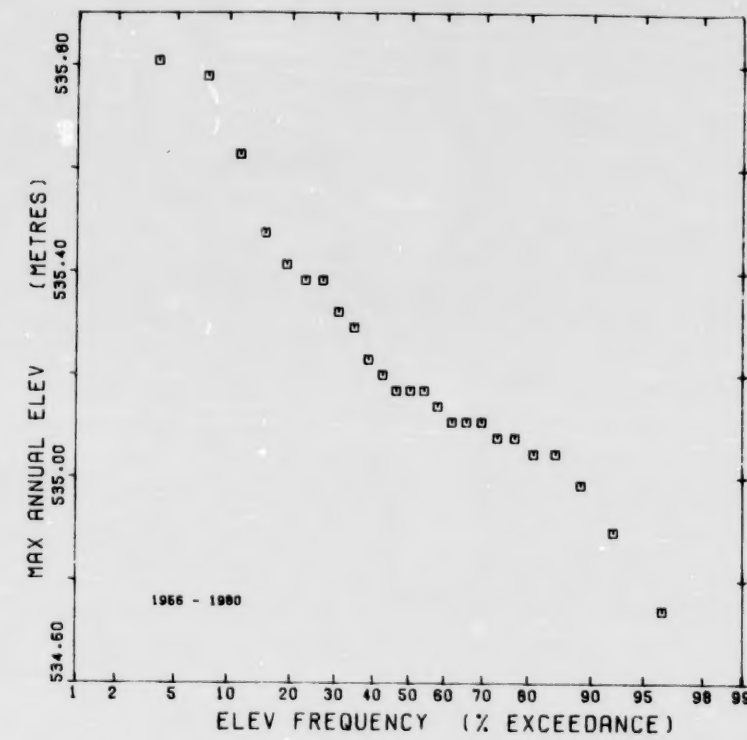
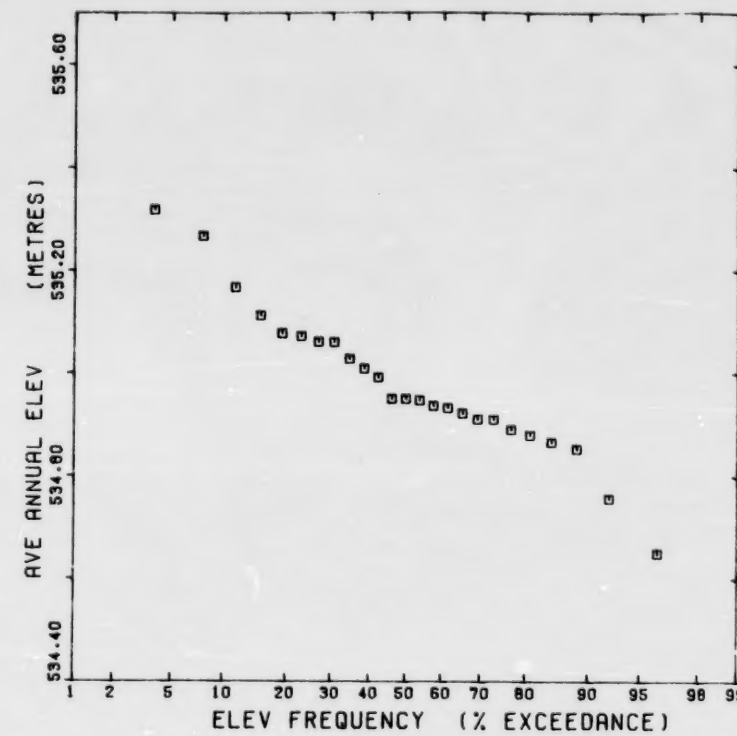
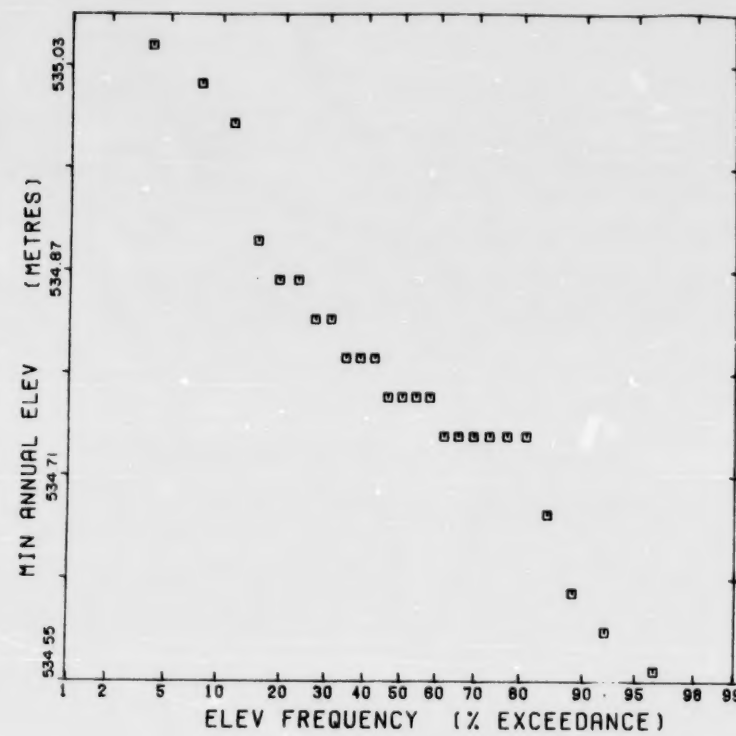
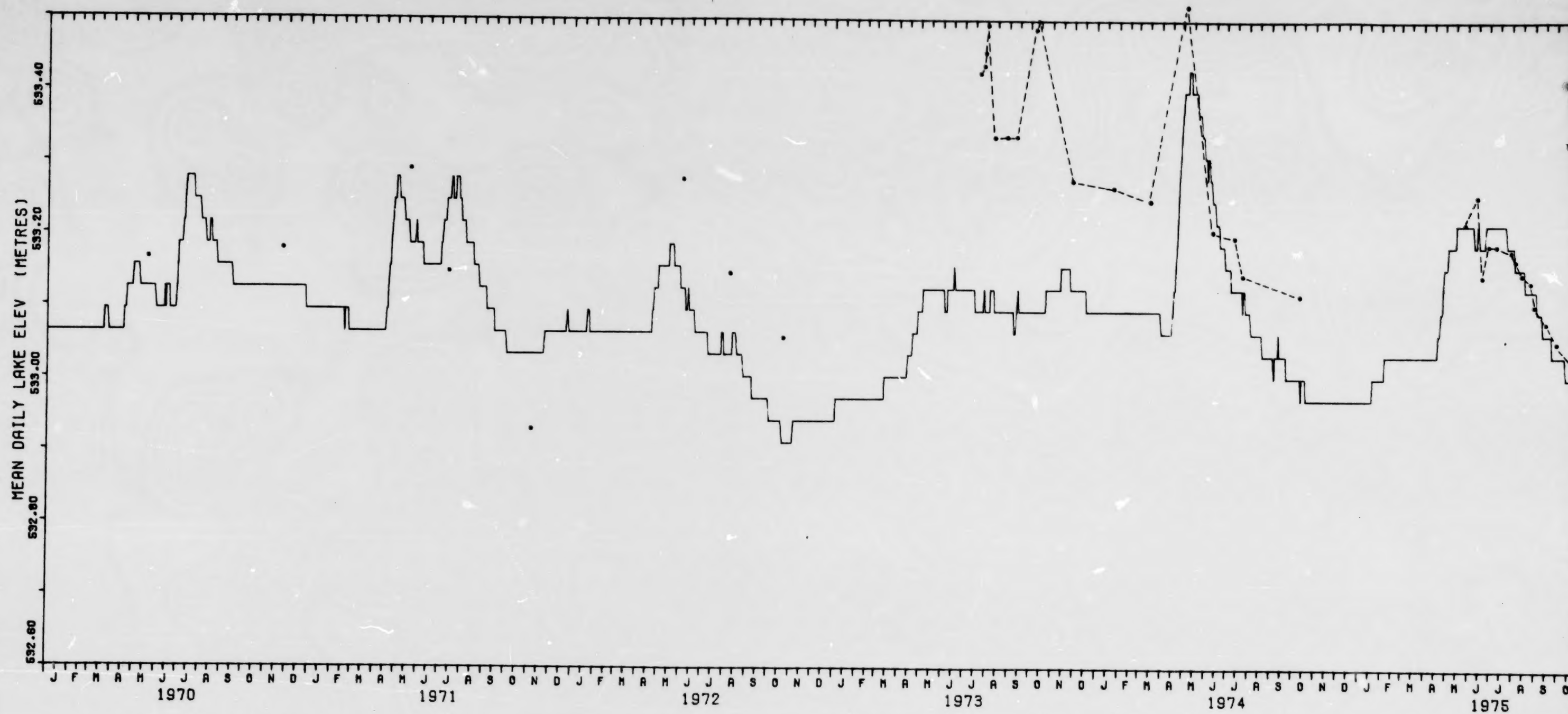


FIGURE 23
ELEVATION FREQUENCY
AND DURATION CURVES

COLD LAKE
(STUDY POINT 533)



210 MOOSE LAKE

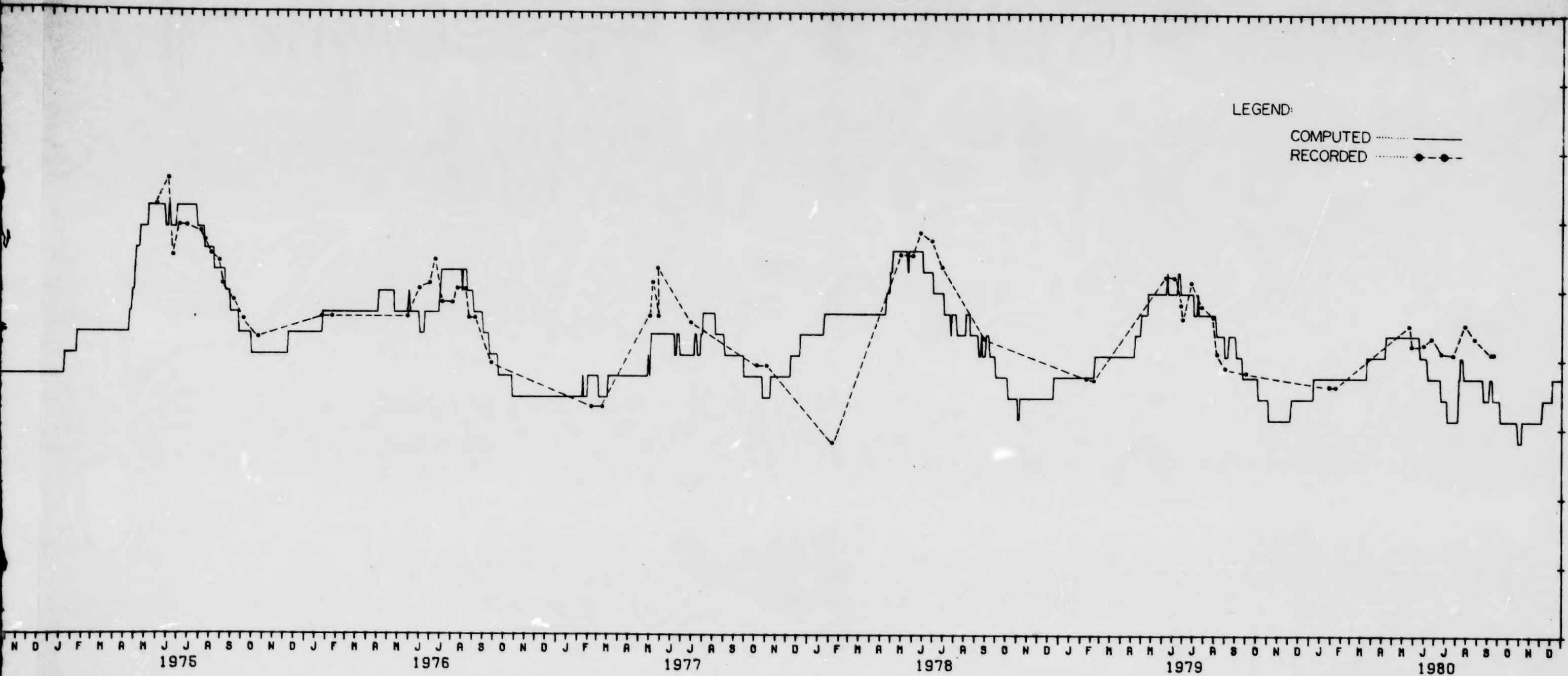
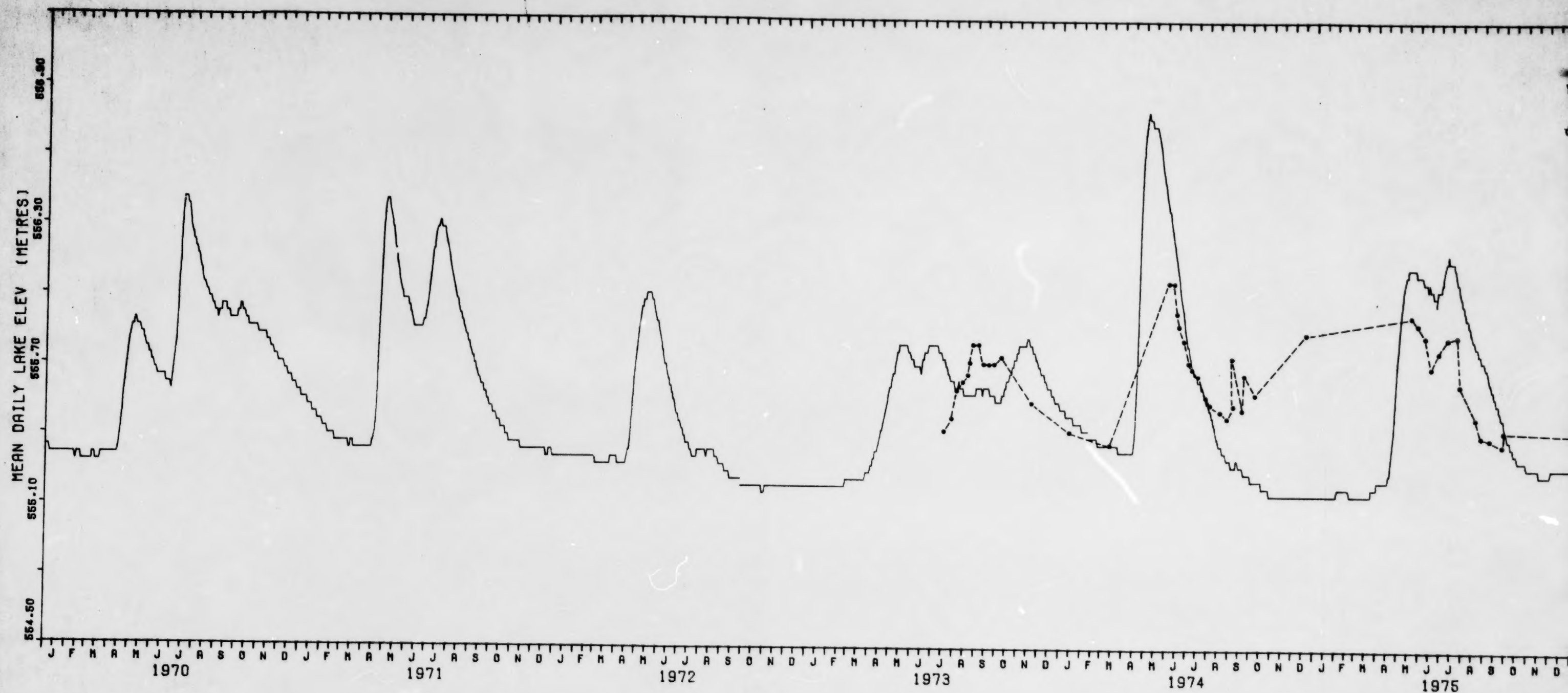


FIGURE 24
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
210 - MOOSE LAKE
WATER LEVELS



212 MANATOKAN LAKE

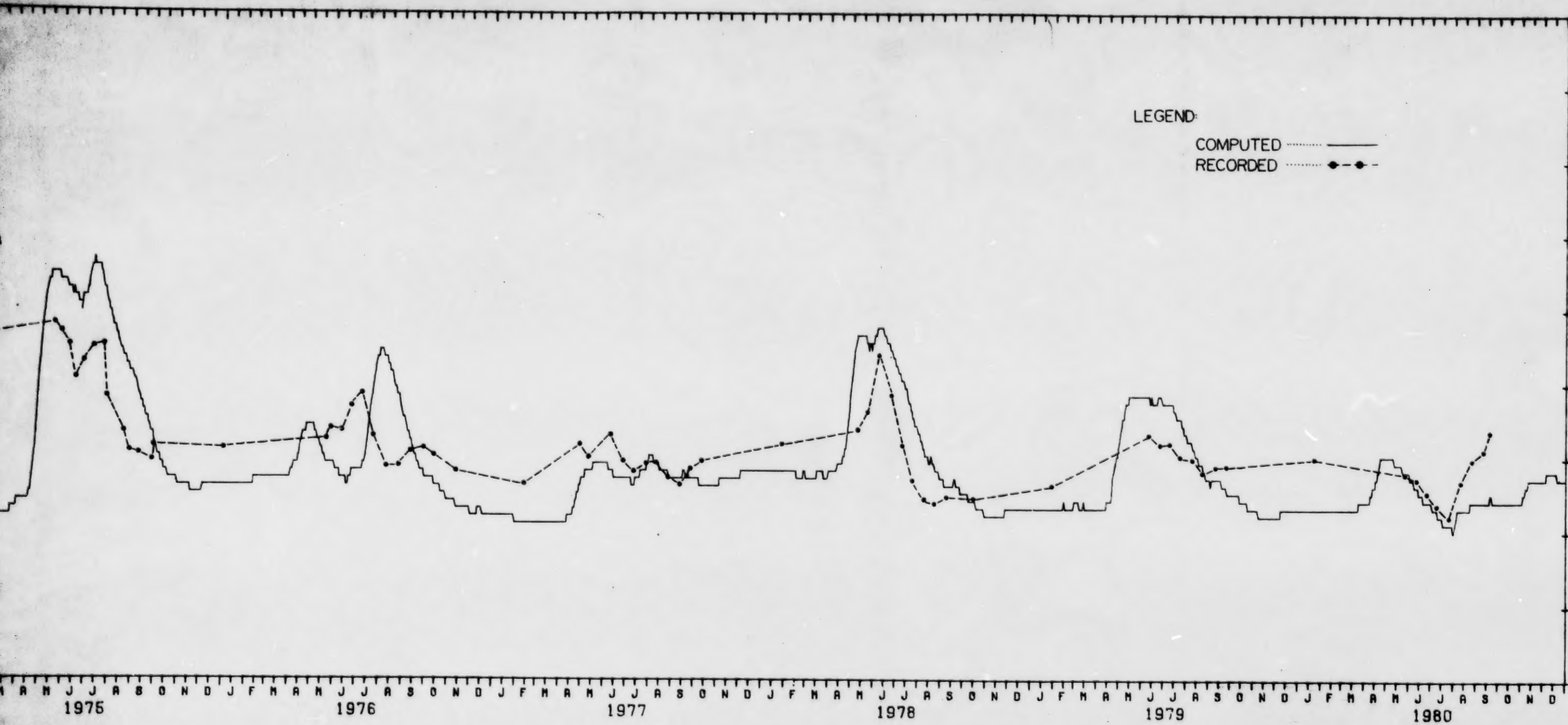
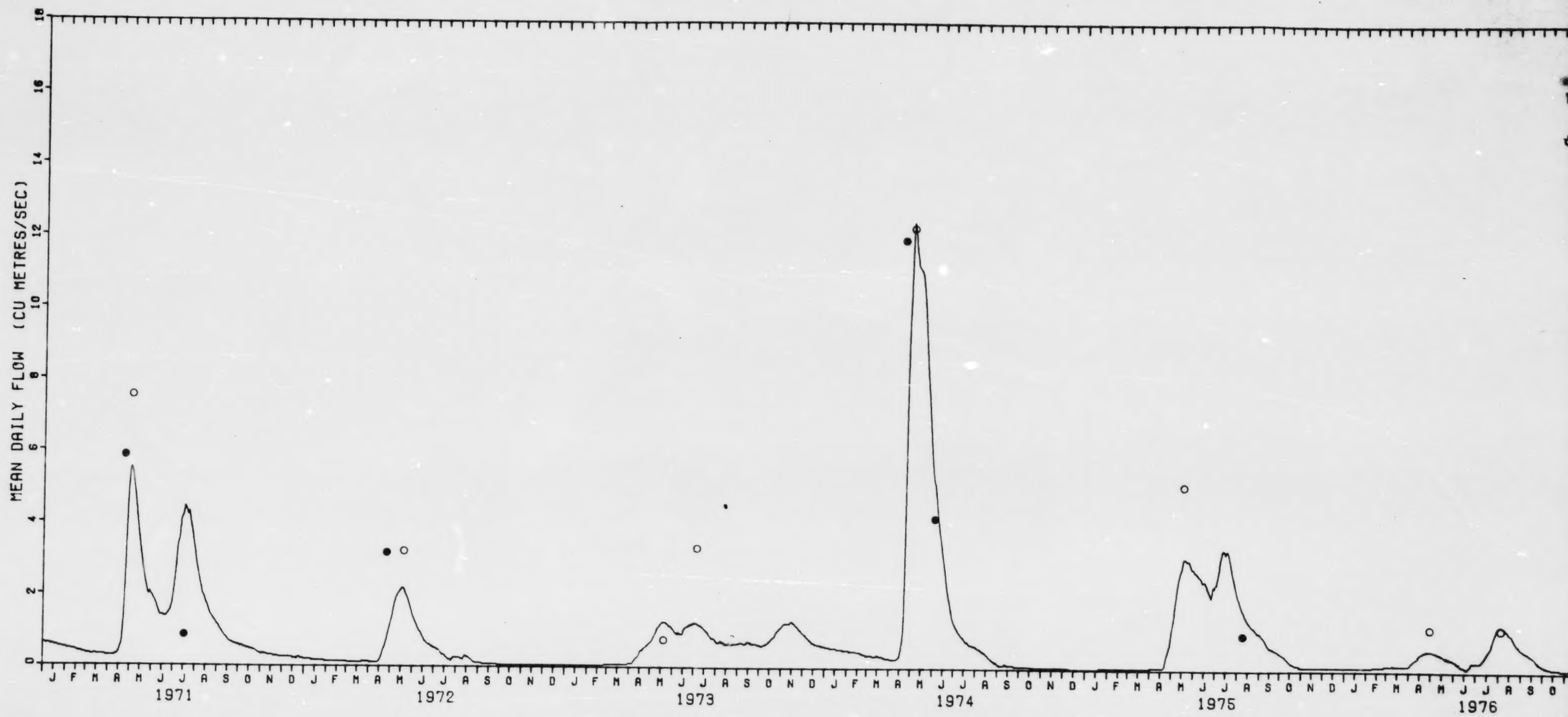


FIGURE 25
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
212- MANATOKAN LAKE
WATER LEVELS



212 MANATOKAN LAKE

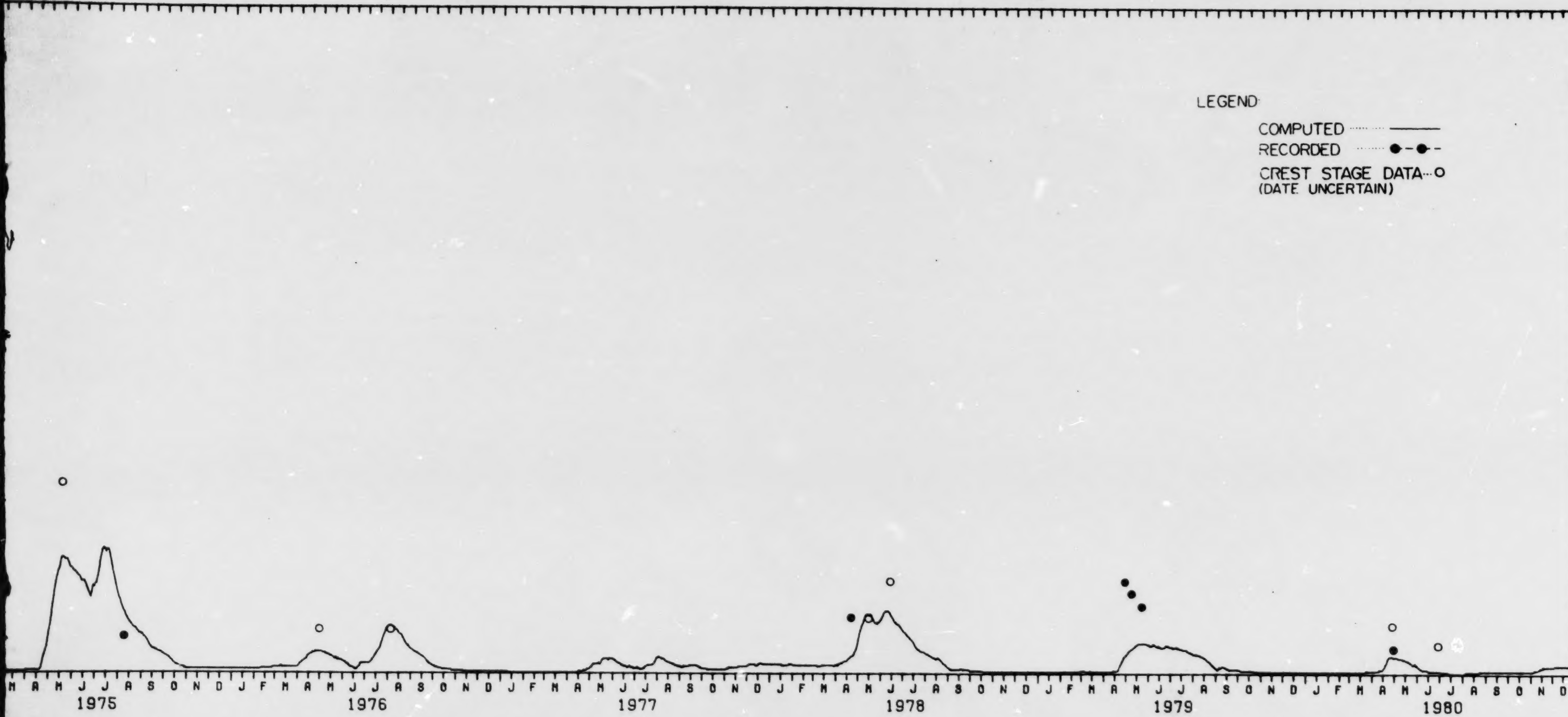
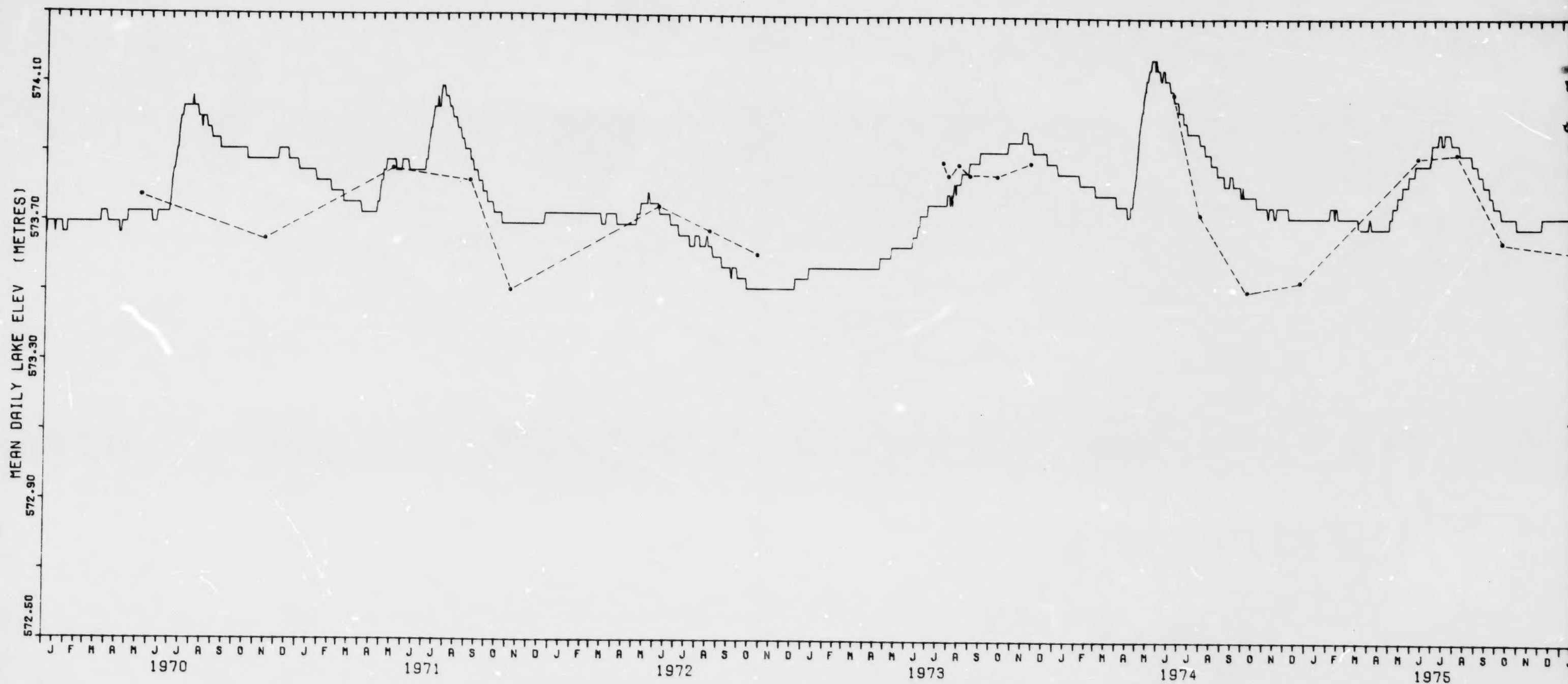


FIGURE 26
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
MANANATOKAN LAKE
OUTFLOW



220 MARIE LAKE

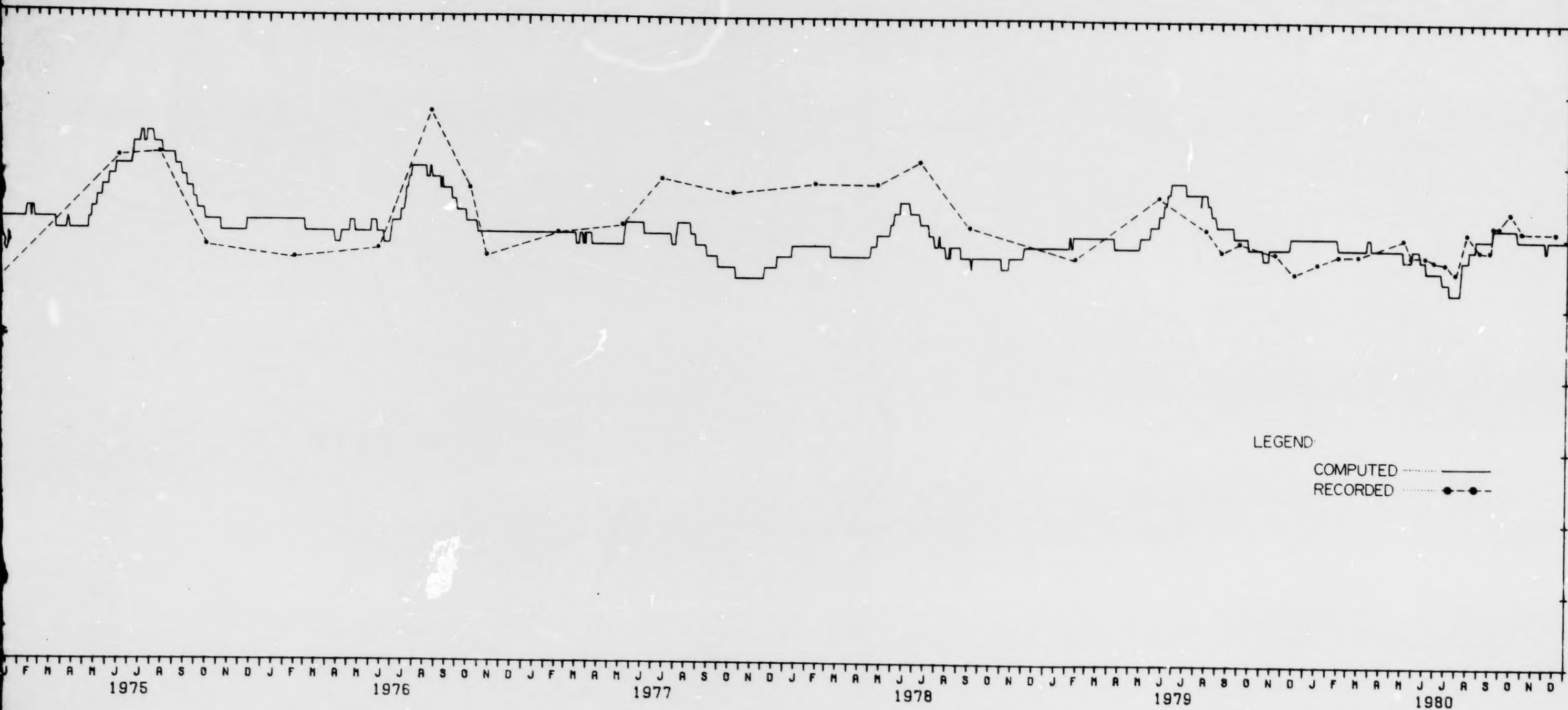
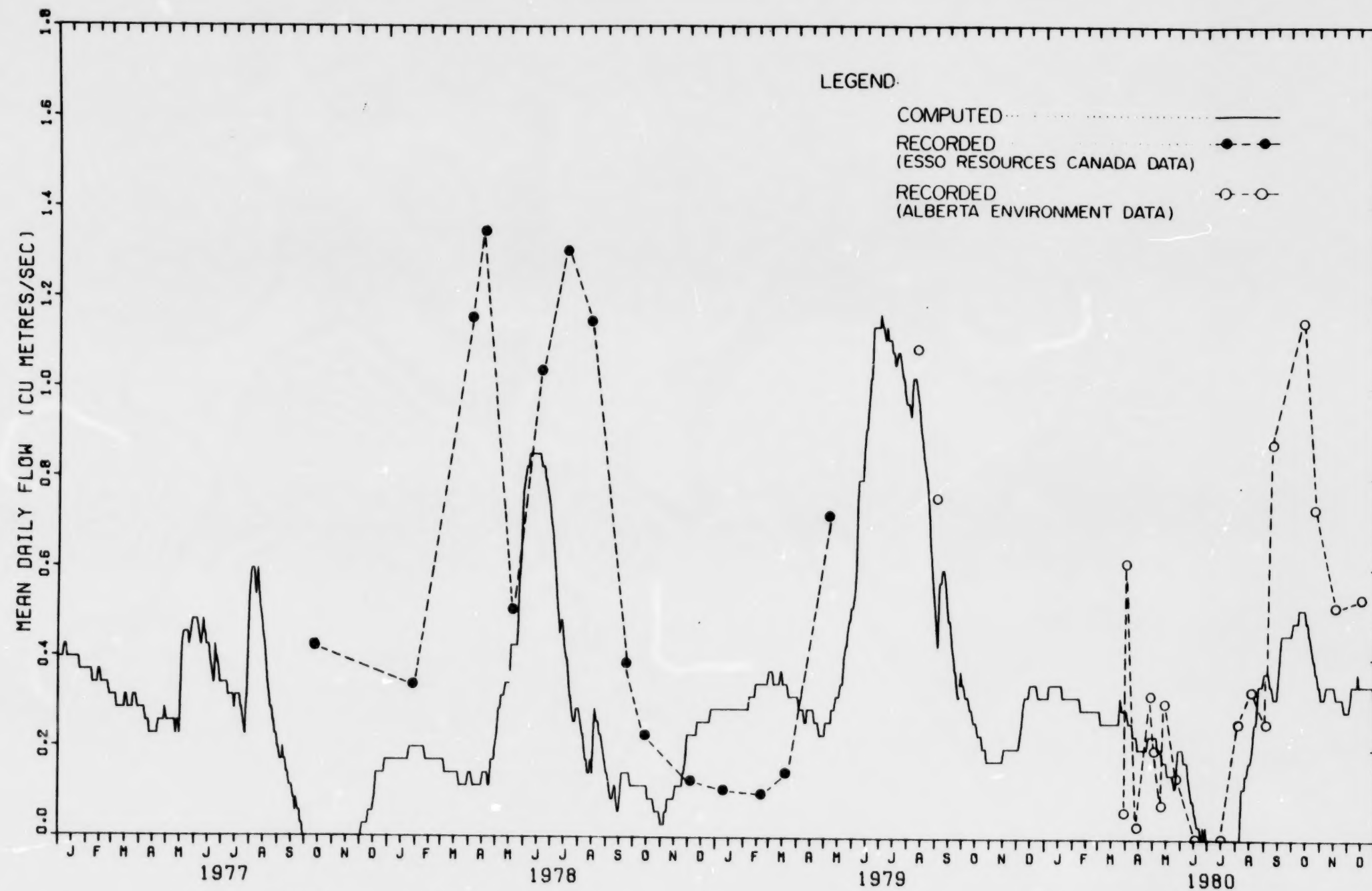


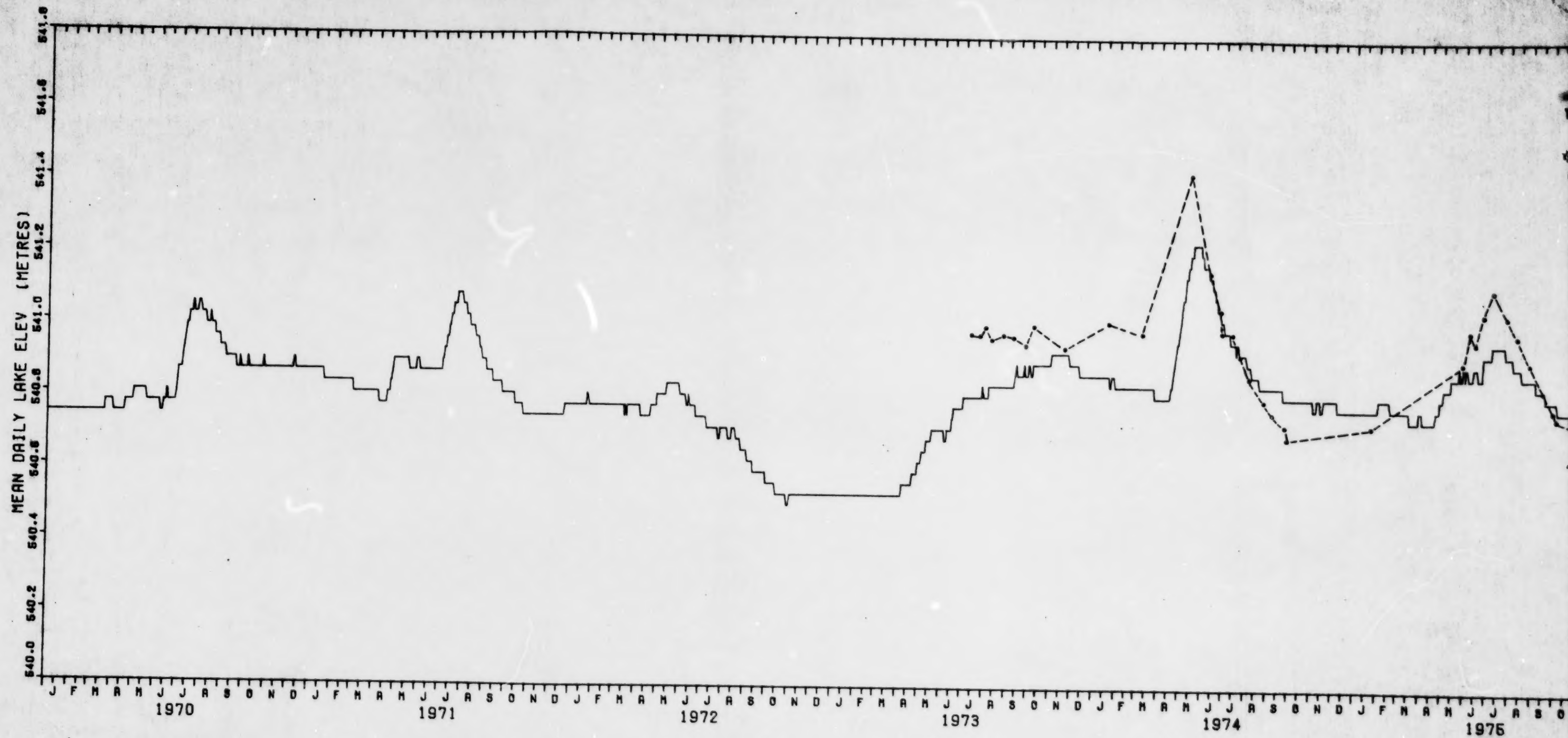
FIGURE 27
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
220 - MARIE LAKE
WATER LEVELS



220 MARIE LAKE

FIGURE 28
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA

220 - MARIE LAKE
OUTFLOW



221 ETHEL LAKE

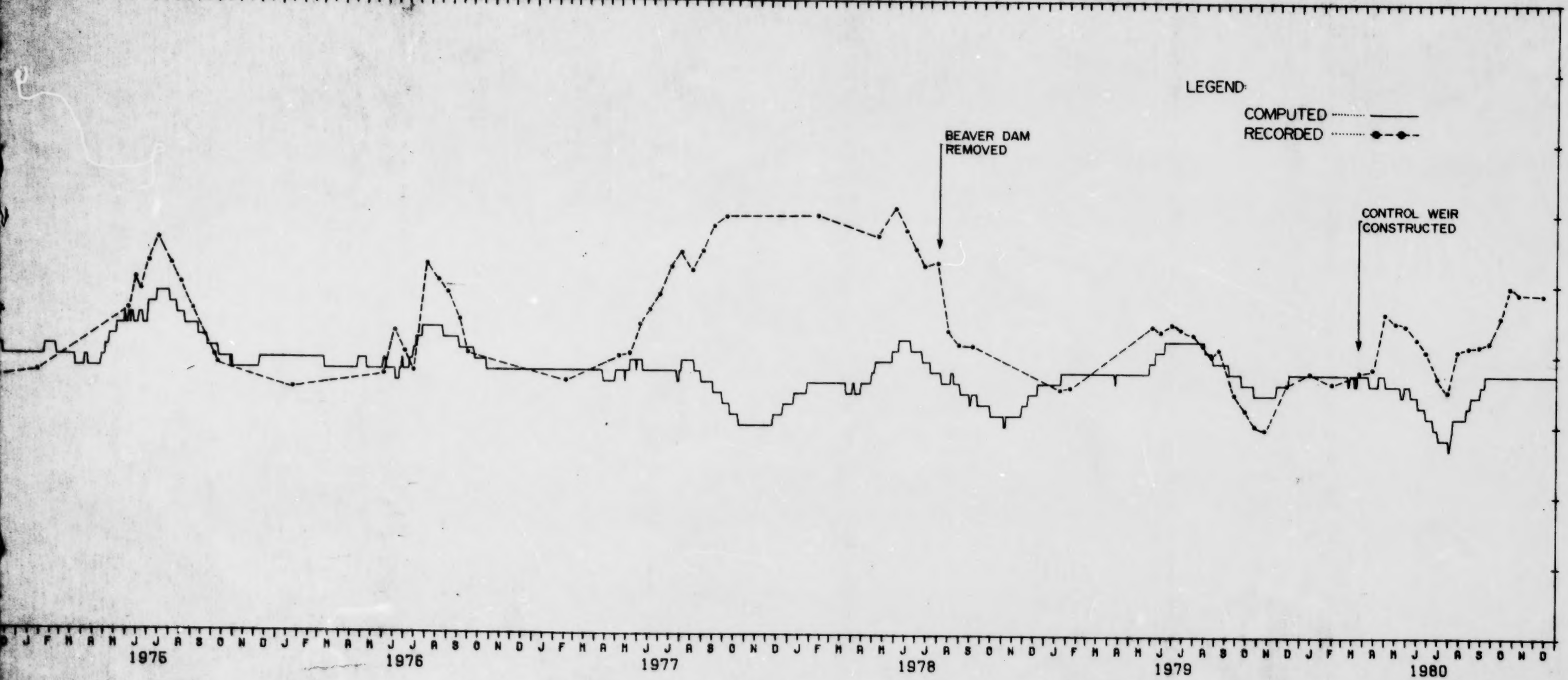
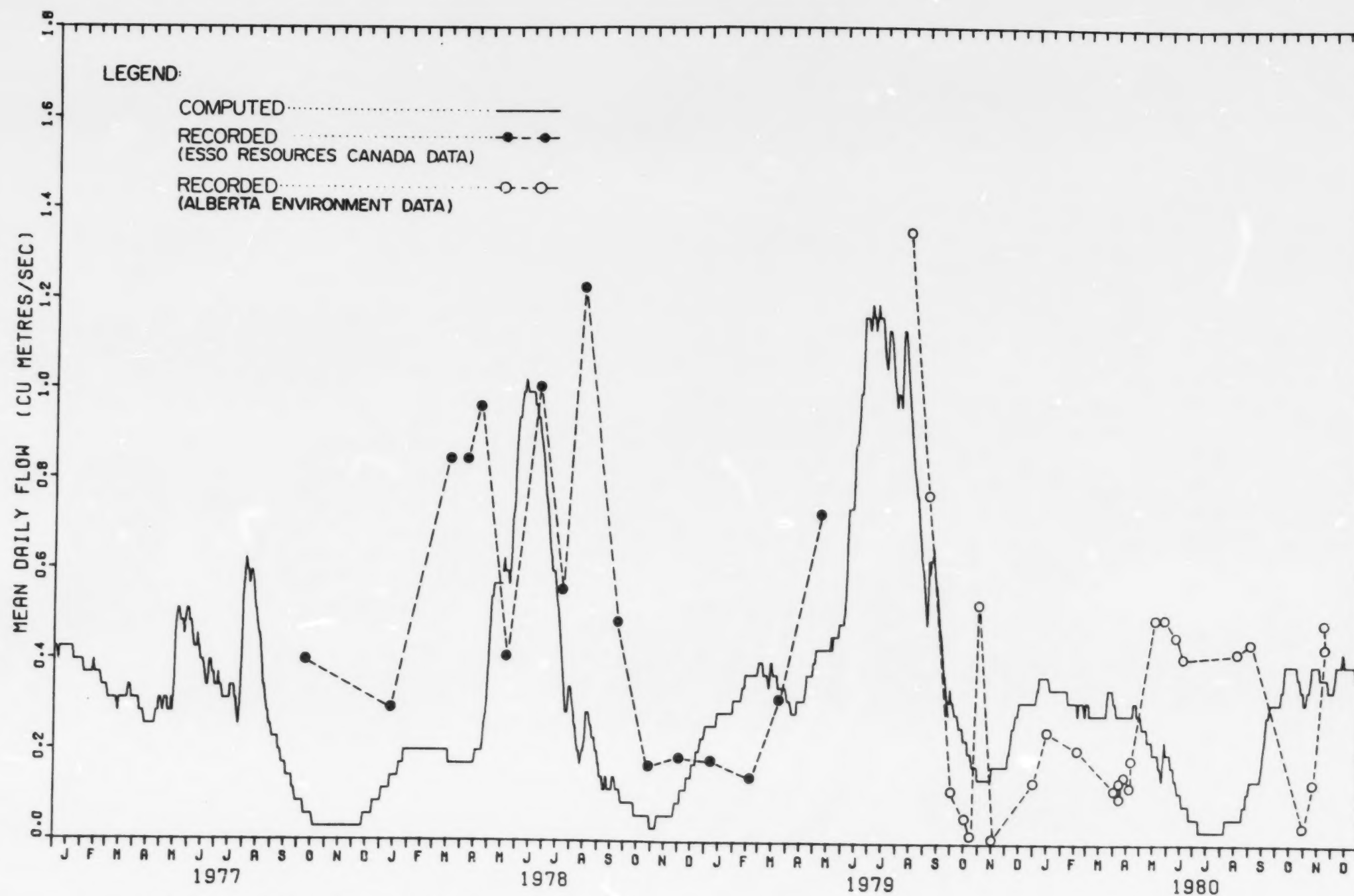


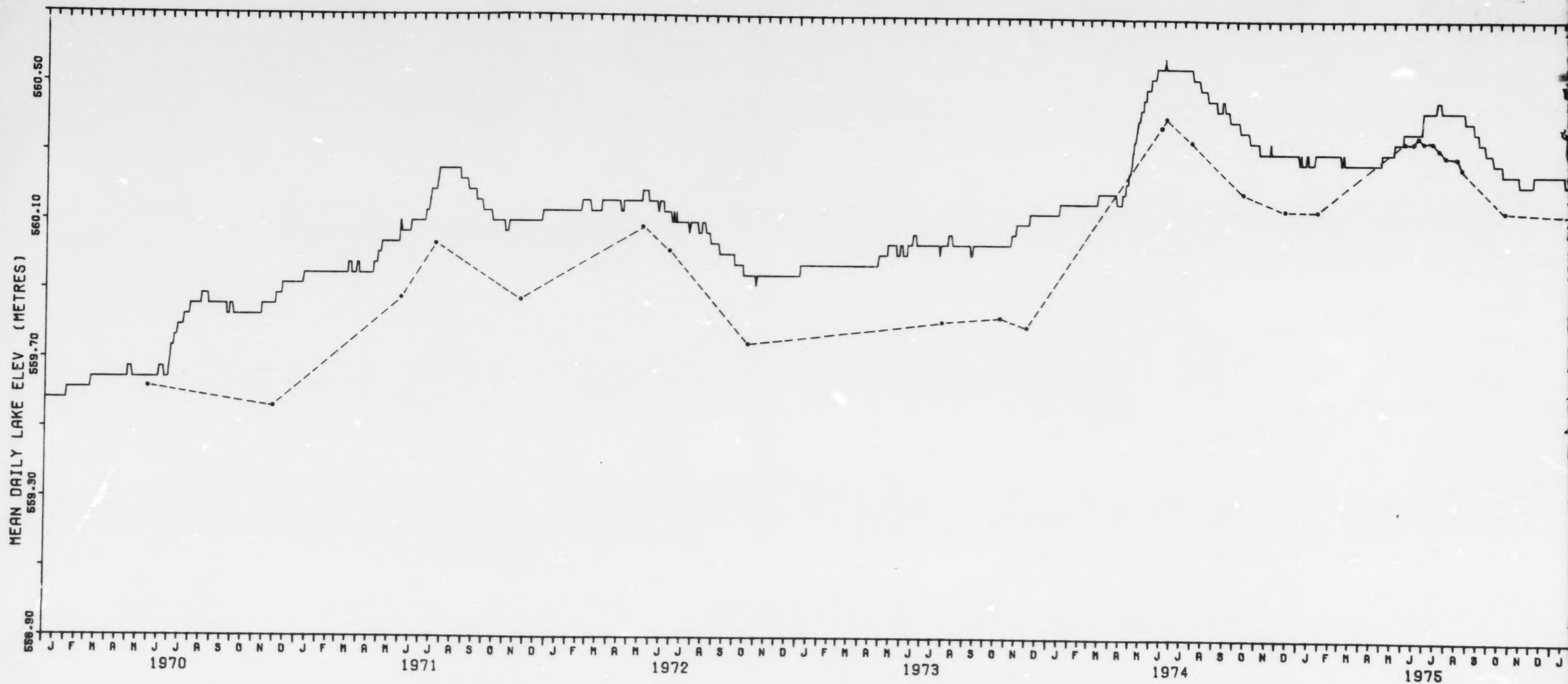
FIGURE 29
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
221 - ETHEL LAKE
WATER LEVELS



221 ETHEL LAKE

FIGURE 30
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA

221 - ETHEL LAKE
OUTFLOW



223 MURIEL LAKE

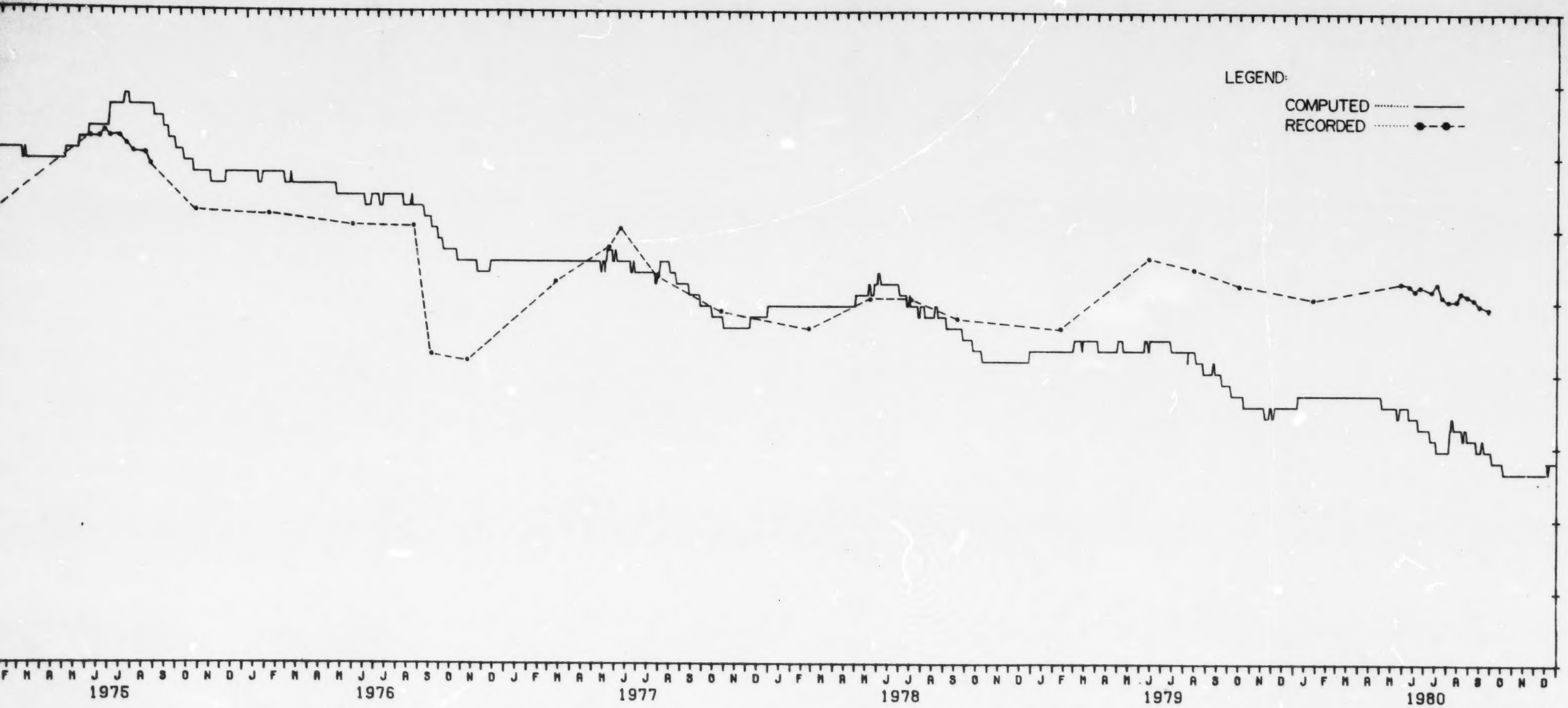
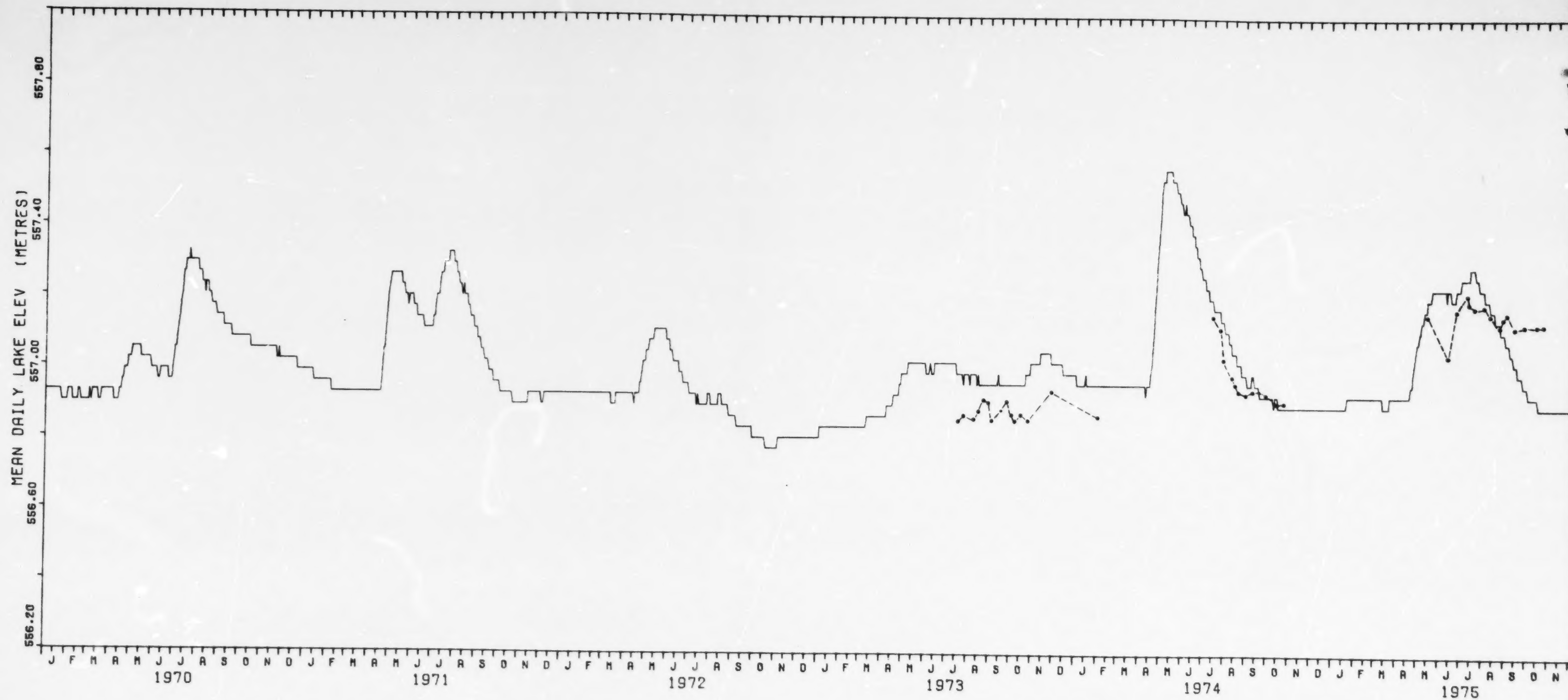


FIGURE 31
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
223 - MURIEL LAKE
WATER LEVELS



228 ANGLING LAKE

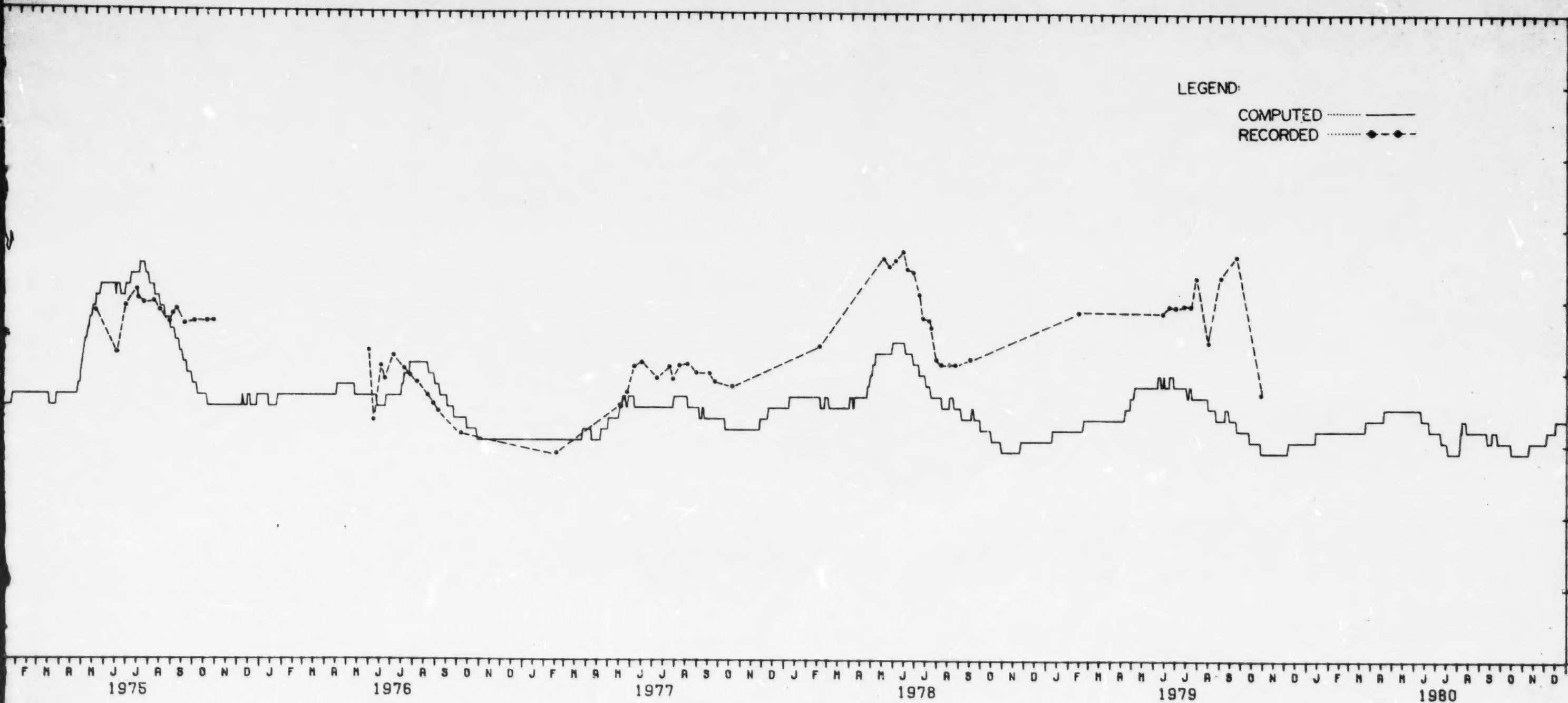


FIGURE 32
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA
228 - ANGLING LAKE
WATER LEVELS

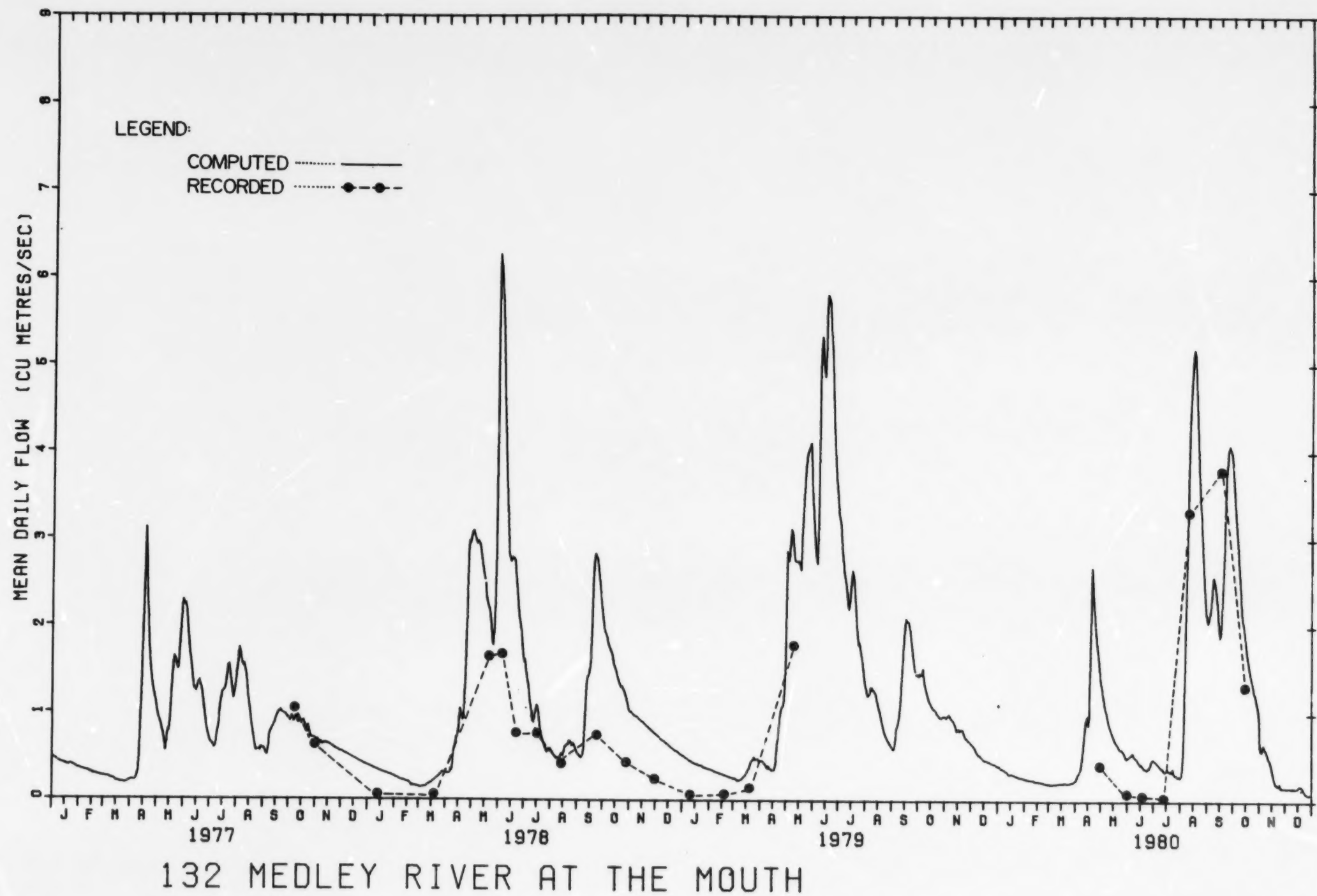
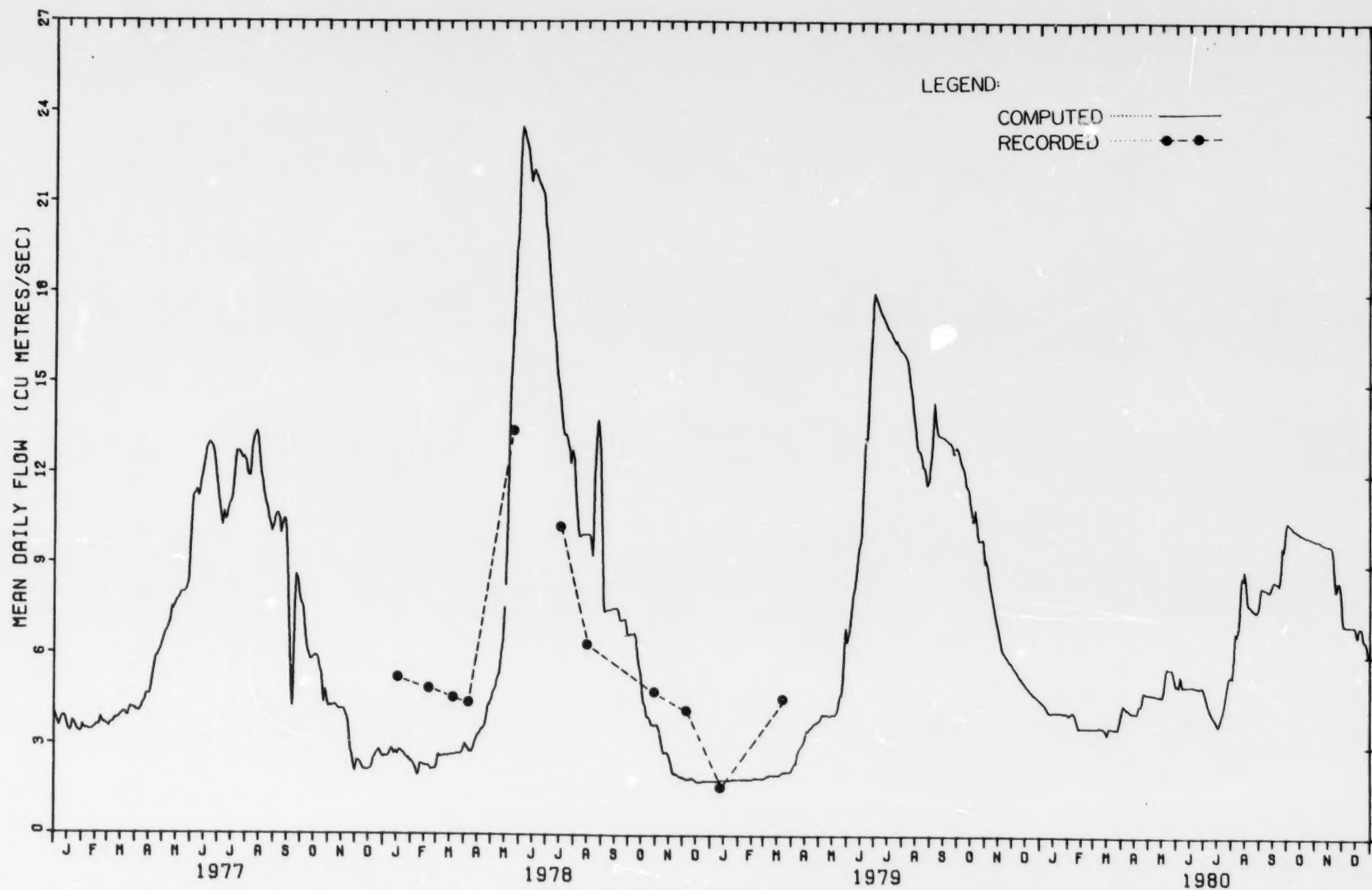


FIGURE 33
COMPARISON OF MODEL RESULTS
WITH RECORDED DATA

132 - MEDLEY RIVER
OUTFLOW



PRIMROSE LAKE OUTFLOW

FIGURE 34
 COMPARISON OF MODEL RESULTS
 WITH RECORDED DATA

231 - PRIMROSE LAKE
 OUTFLOW

APPENDIX A

SAMPLE MODEL DATA LISTING

This Appendix contains a complete listing of the SSARR model data deck used for the 1956 to 1959 simulation run. Model data are listed in the following pages and are described briefly below. For details of the data format the reader is referred to the SSARR User Manual (Reference 2 in the main report). Imperial units are used throughout.

The data consists of the following:

1. Job control cards. These consist of data to identify the model run, such as the date and run name.
2. Streamflow data stations - a list of stations and station labels for locations where streamflow and evaporation data are input (streamflow gauges and evaporation station 800).
3. Sum points and their locations. These are junctions, nodes, or locations other than lakes or river reaches at which output is required.
4. Local area runoff data. For each local area, two data lines occur; the first identifies the local area, and the second specifies the analog station and the yield factor applied to the analog flows to estimate the local area flows.
5. Lake evaporation/precipitation data. For each lake, evaporation/precipitation is specified as a percentage of station 800.

6. Dummy reservoir data. Four lines of data for each dummy reservoir specify reservoir storage characteristics such that a 1-foot change in reservoir level corresponds to 1 inch of runoff in the contributing watershed. These reservoirs are used to compute the accumulated simulation error (in inches over the basin) at locations where streamflows are recorded.
7. Lake elevation-discharge-storage tables. These tables specify lake outflow and storage volumes for the possible range in water levels. Storage volumes are defined to have a value of 0.0 acre feet at the elevation of the lake outlet sill.
8. River channel routing characteristics. These cards specify the number of reaches and values of the routing parameters n and KTS per reach.
9. Configuration data. These data form a "map" of the watershed; they specify the location of all stations with respect to other stations in the watershed.
10. Time control card - specifies the start and end dates of the simulation.
11. Initial conditions - specify lake elevations and stream discharges at the start of the run.
12. Dummy reservoir regulation cards. These cards specify initial and final conditions required in the operation of the dummy reservoirs.

13. Streamflow data for base stations. These data are provided for the four-year period 1956 - 1959 (4 years is the maximum length of simulation possible in one SSARR model run).
14. Plot control cards - control graphical output.

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1 *** AE-COLD SEARR MODEL V1 *** REVISED APRIL 1982 ***
2 *** BEAVER RIVER - COLD LAKE *** 1958 - 1959
3 *** 5101270482 0 1 P1 AE-COLD - BEAVER RIVER MODEL SIMULATION
4 *** STREAMFLOW DATA STATIONS ***
5 CC01 802 0 BEAVER AT GOODRIDGE 08A0001
6 CC01 808 0 WSC FLOWS WOLF R. 08A0002
7 CC01 808 0 SAND RIVER 08A0001
8 CC01 816 0 JACKFISH CR. 08A0001
9 CC01 828 0 BEAVER R. AT COLD LAKE 08A0008
10 CC01 833 0 COLD L. OUTFLOWS GEN FROM RECORDED LAKE LEVELS
11 CC01 131 0 PRIMROSE L. LOCAL + EVAP (CALCULATED)
12 CC01 133 0 CALCULATED SAND RIVER LOCAL U/S OF WOLF
13 CC01 807 0 CALCULATED LOCAL BEAVER D/S GOODRIDGE
14 CC01 830 0 NET EVAP. BASED ON COLD LAKE
15 CC01 800 0
16 *** ADDITIONAL SUM POINTS ***
17 CC01 803 2 SUM BEAVER RIVER U/S OF SAND
18 CC01 806 2 SUM SAND R. U/S WOLF
19 CC01 807 2 SUM SAND R. D/S WOLF
20 CC01 808 2 SUM SAND RIVER U/S OF BEAVER
21 CC01 810 0 MOOSE RIVER AT MOUTH
22 CC01 811 0 BEAVER RIVER D/S SAND RIVER
23 CC01 812 2 BEAVER RIVER D/S MANATOKAN
24 CC01 816 2 BEAVER RIVER U/S JACKFISH
25 CC01 817 0 BEAVER RIVER D/S JACKFISH
26 CC01 8171 2 BEAVER RIVER U/S MARIE CR.
27 CC01 821 2 MARIE CR. AT MOUTH
28 CC01 822 0 BEAVER RIVER D/S MARIE
29 CC01 828 2 MURIEL CR. AT MOUTH
30 CC01 828 2 SUM REITA CR. AT MOUTH
31 CC01 830 2 SUM BEAVER RIVER D/S REITA
32 CC01 834 2 SUM BEAVER RIVER AT BORDER
33 *** LOCAL AREA RUNOFF ***
34 CC01 104 0 CARIBOU INFLOW
35 CC02 807 808
36 CC01 108 0 SAND R. LOCAL U/S WOLF
37 CC02 108 807 808
38 CC01 107 0 FISHER CR.
39 CC02 107 807 80
40 CC01 108 0 SAND R. LOCAL D/S WOLF
41 CC02 108 808 1080
42 CC01 118 0 TUCKER L. LOCAL
43 CC02 118 818 870
44 CC01 117 0 JACKFISH CR. LOCAL
45 CC02 117 818 380
46 CC01 103 0 BEAVER R. LOCAL U/S OF SAND & MOOSE CONF
47 CC02 103 818 1110
48 CC01 109 0 SAND R. LOCAL D/S OF GAUGE B1
49 CC02 109 818 240
50 CC01 110 0 MOOSE L. LOCAL
51 CC02 110 818 1890
52 CC01 111 0 MOOSE R. LOCAL
53 CC02 111 830 23
54 CC01 112 0 MANATOKAN L. LOCAL
55 CC02 112 818 890
56 CC01 118 0 BEAVER R. LOCAL U/S OF JACKFISH
57 CC02 118 830 120
58 CC01 122 0 BEAVER R. LOCAL U/S MARIE CR.
59 CC02 122 830 80
60 CC01 118 0 BURNT L. LOCAL

61 CC02 118 807 28
62 CC01 118 0 MAY L. LOCAL
63 CC02 118 807 12
64 CC01 120 0 MARIE L. LOCAL
65 CC02 120 807 34
66 CC01 121 0 ETHEL L. LOCAL
67 CC02 121 818 170
68 CC01 1211 0 MARIE CR. U/S MOUTH LOCAL
69 CC02 1211 830 87
70 CC01 123 0 MURIEL L. LOCAL
71 CC02 123 818 880
72 CC01 124 0 CHARLOTTE L. LOCAL
73 CC02 124 818 410
74 CC01 128 0 MURIEL CR. MOUTH LOCAL
75 CC02 128 830 74
76 CC01 130 0 BEAVER R. @ COLD L. LOCAL
77 CC02 130 838 20
78 CC01 127 0 REITA L. LOCAL
79 CC02 127 818 120
80 CC01 128 0 ANGLING L. LOCAL
81 CC02 128 818 320
82 CC01 128 0 REITA CR. LOCAL
83 CC02 128 830 18
84 CC01 132 0 MEDLEY R. LOCAL
85 CC02 132 807 130
86 CC01 134 0 BEAVER R. REITA CR. TO BORDER
87 CC02 134 830 280
88 *** LAKE EVAPORATION ***
89 CC01 804 0 CARIBOU EVAPO
90 CC02 804 800 08
91 CC01 816 0 TUCKER LAKE EVA. 1.3% (800)
92 CC02 816 800 13
93 CC01 810 0 MOOSE LAKE EVA. 7.8% (800)
94 CC02 810 800 78
95 CC01 812 0 MANATOKAN LAKE EVA. 0.9% (800)
96 CC02 812 800 08
97 CC01 818 0 BURNT LAKE EVA. 0.9% (800)
98 CC02 818 800 08
99 CC01 818 0 MAY LAKE EVA. 0.8% (800)
100 CC02 818 800 08
101 CC01 820 0 MARIE LAKE EVA. 7.0% (800)
102 CC02 820 800 70
103 CC01 821 0 ETHEL LAKE EVA. 0.9% (800)
104 CC02 821 800 08
105 CC01 823 0 MURIEL LAKE EVA. 13.2% (800)
106 CC02 823 800 132
107 CC01 824 0 CHARLOTTE LAKE EVA. 2.1% (800)
108 CC02 824 800 21
109 CC01 827 0 REITA LAKE EVA. 2.4% (800)
110 CC02 827 800 24
111 CC01 828 0 ANGLING LAKE EVA. 1.1% (800)
112 CC02 828 800 11
113 *** DUMMY RESERVOIR ***
114 CC01 7080 0 CALC. SAND FLOWS
115 CC01 7081 0 SIGN CHANGES
116 CC02 7081 1
117 CL01 7082 0 0 DUM RES-SUM CAR
118 CL02 7082 800 -800
119 CL01 7082 -1000 -1013333 1000
120 CC01 7180 0 CALC. FLOWS AT JACKFISH CR.

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JOB CONTROL

A

STREAMFLOW

DATA STATIONS



SUM POINTS



LAKE EVAPORATION



LAKE EVAPORATION



DUMMY
RESERVOIR
DATA

1013333

240	C111	228	182709	.04	510	182727	.625	800	
241	C112	228	182788	25	1820	182887	80	2720	
242	C113	228	182887	75	3180	182903	100	3600	
243	C114	228	182929	150	3770	182948	200	4000	
244	C115	228	183021	500	5180	183113	1000	8500	
245	C116	228	183241	2000	8400	183343	3000	9900	
246	C117	228	183803	8000	123000	8588888888888888	8888888888		
247	C101	231	2 0		PRIMROSE LAKE				
248	C102	231			33058	31842			
249	C110	231	31842	.001	-720000	32288	.001	10 PRIM	
250	C111	231	32288	5	80000	32375	25	80000	
251	C112	231	32434	75	145000	32484	150	174000	
252	C113	231	32497	300	313000	32529	800	250000	
253	C114	231	32585	1000	310000	32594	2000	420000	
254	C115	231	32635	4000	580000	33058	8000	838000	
255	C116	231	8588888888888888		8588888888				
256	C101	233	2 0		COLD LAKE				
257	C102	233			175248	174888			
258	C110	233	174888	.001	-212000	174987	.001	-122888 COLD	
259	C111	233	175285	20	175300	175393	30	282800	
260	C112	233	175442	140	308100	175492	325	382700	
261	C113	233	175541	875	387200	175590	1100	426000	
262	C114	233	175656	1825	488000	175721	2085	854500	
263	C115	233	175787	2470	812000	175852	2825	870000	
264	C116	233	176248	3530	1000000	8588888888888888	8888888888		
265	*** RIVER CHANNEL ROUTING CHARACTERISTICS ***								
266	CRO1	304	0 0		SAND R D/S CARISBU				
267	CRO2	304			SAND R D/S WOLF				
268	CRO1	307	0 0		JACKFISH CR.				
269	CRO2	307			BEAVER RIVER GODDRIDGE TO U/S MOOSE				
270	CRO1	318	0 0		SAND RIVER SAND R. GAUGE TO MOUTH				
271	CRO2	318			MOOSE RIVER MOOSE LAKE TO MOUTH				
272	CRO1	302	0 0		BEAVER RIVER D/S MANATOKAN TO U/S JACKFISH				
273	CRO2	302			BEAVER RIVER D/S JACKFISH TO U/S MARIE				
274	CRO1	308	0 0		MARIE CR.				
275	CRO2	308			MURIEL CR.				
276	CRO1	310	0 0		REITA CR.				
277	CRO2	310			BEAVER RIVER D/S REITA CR. TO BORDER				
278	CRO1	314	0 0		MARTINEAU RIVER				
279	CRO2	314							
280	CRO1	317	0 0						
281	CRO2	317							
282	CRO1	321	0 0						
283	CRO2	321							
284	CRO1	324	0 0						
285	CRO2	324							
286	CRO1	328	0 0						
287	CRO2	328							
288	CRO1	330	0 0						
289	CRO2	330							
290	CRO1	331	0 0						
291	CRO2	331							
292	*** CONFIGURATION ***								
293	P1	507							
294	P2	530							
295	P3	500							
296	P4	104	204						
297	P5	404	204						
298	P7	204	304						
299	P8	304	505						

LAKE
ELEVATION-
DISCHARGE-
STORAGE
TABLES

RIVER

CHANNEL
ROUTING
CHARACTERISTICS

300	P9	105	505	
301	P10	505	507	
302	P11	107	507	
303	P12	505	507	
304	P13	507	307	
305	P14	307	7080	
306	P15	108	7080	
307	P16	7080	7081	
308	P17	7081	7082	
309	P18	508	7082	308
310	P19	7082		
311	P20	518	517	
312	P21	518	218	
313	P22	518	318	
314	P23	318	7180	
315	P24	117	7180	
316	P25	7180	7181	
317	P26	7181	7182	
318	P27	7182		
319	P28	502	302	
320	P29	302	503	
321	P30	103	503	
322	P31	503	511	
323	P32	308	508	
324	P33	108	509	
325	P34	509	311	
326	P35	110	210	
327	P36	210	310	
328	P37	310	510	
329	P38	111	510	
330	P39	510	511	
331	P40	511	512	
332	P41	512	212	
333	P42	212	512	
334	P43	314	515	
335	P44	515	516	
336	P45	516	517	
337	P46	517	317	
338	P47	517	517	
339	P48	517	517	
340	P49	517	517	
341	P50	517	517	
342	P51	517	517	
343	P52	517	517	
344	P53	517	517	
345	P54	517	517	
346	P55	517	517	
347	P56	517	517	
348	P57	517	517	
349	P58	517	517	
350	P59	517	517	
351	P60	517	517	
352	P61	517	517	
353	P62	517	517	
354	P63	517	517	
355	P64	517	517	
356	P65	517	517	
357	P66	517	517	
358	P67	517	517	
359	P68	517	517	

CONFIDENTIAL
DATA

480	SD	8028120 8 8881	708	885	830	885	846	812	483	459
481	SD	802812017 8881	420	417	408	384	372	351	338	327
482	SD	802712028 8881	321	309	300	294	288	278	265	0
483	SD	8028120 1 8881	281	303	312	324	338	346	351	360
484	SD	8028120 8 8881	357	344	342	330	318	312	297	281
485	SD	802812017 8881	279	270	275	324	411	831	830	724
486	SD	802812028 8881	757	802	822	838	820	778	0	0
487	SD	8028120 110881	724	885	848	809	573	831	801	468
488	SD	8028120 810881	483	428	408	384	378	354	342	330
489	SD	80281201710881	318	306	297	288	278	272	264	258
490	SD	80271202810881	248	238	223	215	212	208	205	0
491	SD	8028120 111881	271	281	300	312	338	327	243	281
492	SD	8028120 811881	257	286	255	228	207	208	197	190
493	SD	80281201711881	182	183	185	178	176	171	170	162
494	SD	80281202811881	158	198	182	148	142	143	0	0
495	SD	8028120 112881	140	133	128	112	103	83	88	73
496	SD	8028120 812881	88	82	82	80	80	88	87	80
497	SD	80281201712881	80	88	88	88	88	88	88	87
498	SD	80271202812881	88	88	88	88	88	88	88	0
499	SD	8028120 1 1871	84	82	47	48	44	42	43	42
500	SD	8028120 8 1871	38	38	34	30	27	24	23	21
501	SD	802812017 1871	18	19	18	18	18	18	14	13
502	SD	802712028 1871	12	11	9	8	8	8	8	0
503	SD	8028120 1 2871	8	8	18	20	17	18	14	14
504	SD	8028120 8 2871	12	12	11	10	8	10	10	8
505	SD	802812017 2871	8	8	8	8	8	8	2	2
506	SD	802812028 2871	2	2	1	1	0	0	0	0
507	SD	8028120 1 3871	1	1	1	2	4	8	9	13
508	SD	8028120 8 3871	14	18	18	14	13	13	13	12
509	SD	802812017 3871	12	12	12	13	13	13	14	18
510	SD	802712028 3871	18	18	18	18	24	28	32	0
511	SD	8028120 1 4871	37	84	78	91	121	162	174	184
512	SD	8028120 8 4871	186	177	178	173	181	184	183	184
513	SD	802812017 4871	212	228	248	432	474	800	887	892
514	SD	802812028 4871	884	784	814	888	870	821	0	0
515	SD	8028120 1 5871	588	588	528	507	488	474	459	447
516	SD	8028120 8 5871	428	417	408	380	372	378	372	368
517	SD	802812017 5871	383	387	342	338	312	308	287	288
518	SD	802712028 5871	277	288	284	287	244	238	231	0
519	SD	8028120 1 6871	222	218	212	208	200	184	180	181
520	SD	8028120 8 6871	187	183	182	178	178	173	187	189
521	SD	802812017 6871	182	188	182	138	138	134	130	127
522	SD	802812028 6871	128	122	128	128	120	118	118	0
523	SD	8028120 1 7871	121	128	128	128	127	143	148	148
524	SD	8028120 8 7871	148	143	140	138	134	132	134	138
525	SD	802812017 7871	138	138	138	134	138	137	148	148
526	SD	802712028 7871	148	182	188	180	148	142	139	0
527	SD	8028120 1 8871	138	131	128	124	121	117	111	111
528	SD	8028120 8 8871	108	107	108	112	120	118	117	118
529	SD	802812017 8871	114	112	108	108	101	101	98	100
530	SD	802712028 8871	108	100	101	101	100	88	88	0
531	SD	8028120 1 8871	88	84	88	101	101	101	101	103
532	SD	8028120 8 8871	103	108	108	108	108	112	108	108
533	SD	802812017 8871	108	103	88	88	88	88	81	78
534	SD	802812028 8871	88	84	82	80	78	78	0	0
535	SD	8028120 110871	77	87	103	103	101	108	118	121
536	SD	8028120 810871	122	122	127	132	142	181	180	171
537	SD	80281201710871	188	187	128	212	288	388	388	242
538	SD	80271202810871	248	288	303	287	281	241	203	0
539	SD	8028120 111871	182	180	183	184	148	123	108	114

540	SD	8028120 811871	123	127	138	138	138	134	124	114
541	SD	80281201711871	83	84	78	68	84	88	83	80
542	SD	80281202811871	81	81	87	82	88	88	0	0
543	SD	8028120 112871	82	82	82	81	82	82	82	81
544	SD	8028120 812871	82	82	82	84	82	81	48	48
545	SD	80281201712871	47	48	48	44	44	48	48	48
546	SD	80271202812871	48	42	42	41	41	38	38	42
547	SD	8028120 1 1881	38	39	41	41	41	41	41	38
548	SD	8028120 8 1881	40	40	41	41	42	40	38	38
549	SD	802812017 1881	38	38	38	38	38	38	38	38
550	SD	802712028 1881	40	40	44	38	38	38	38	0
551	SD	8028120 1 2881	38	37	37	38	38	38	34	34
552	SD	8028120 8 2881	33	34	33	32	30	30	30	30
553	SD	802812017 2881	30	30	30	28	28	28	28	28
554	SD	802412028 2881	28	27	27	27	0	0	0	0
555	SD	8028120 1 3881	28	28	28	28	28	28	28	28
556	SD	8028120 8 3881	27	28	28	27	27	28	30	31
557	SD	802812017 3881	31	32	34	24	34	34	38	38
558	SD	802712028 3881	44	41	43	43	48	48	87	0
559	SD	8028120 1 4881	107	178	282	347	581	787	783	1008
560	SD	8028120 8 4881	1204	1074	1074	1078	1083	1128	1188	1188
561	SD	802812017 4881	1204	1188	1188	1171	1182	1188	1188	1188
562	SD	802812028 4881	1144	1123	1042	944	843	818	0	0
563	SD	8028120 1 5881	888	823	780	780	727	712	888	884
564	SD	8028120 8 5881	838	803	878	831	813	482	488	435
565	SD	802812017 5881	408	388	381	388	387	348	333	324
566	SD	802712028 5881	312	308	287	284	300	300	300	0
567	SD	8028120 1 6881	300	288	278	288	288	240	228	212
568	SD	8028120 8 6881	204	197	188	182	171	184	180	187
569	SD	802812017 6881	181	148	138	140	142	138	133	128
570	SD	802812028 6881	128	120	117	114	112	108	0	0
571	SD	8028120 1 7881	104	100	84	88	88	84	78	74
572	SD	8028120 8 7881	72	87	82	88	87	88	82	48
573	SD	802812017 7881	48	42	40	40	38	38	38	34
574	SD	802712028 7881	33	33	33	32	32	30	28	0
575	SD	8028120 1 8881	28	24	24	22	24	24	23	22
576	SD	8028120 8 8881	21	20	18	18	18	14	12	12
577	SD	802812017 8881	12	11	10	8	8	8	8	8
578	SD	802712028 8881	8	8	8	8	8	8	8	0
579	SD	8028120 1 8881	8	8	8	3	3	3	2	2
580	SD	8028120 8 8881	2	2	13	17	22	33	51	59
581	SD	802812017 8881	78	81	118	128	127	132	138	137
582	SD	802812028 8881	138	133	128	128	124	118	0	0
583	SD	8028120 110881	118	118	118	118	108	103	108	98
584	SD	8028120 810881	88	88	88	81	81	82	81	84
585	SD	80281201710881	88	83	84	86	87	70	67	0
586	SD	8028120 111881	88	88	70	71	80	88	88	83
587	SD	8028120 811881	82	72	73	88	88	42	37	38
588	SD	80281201711881	33	30	28	23	18	11	8	7
589	SD	80281202811881	4	3	3	2	3	4	0	0
590	SD	8028120 112881	4	4	3	2	1	0	0	0
591	SD	8028120 812881	0	0	0	0	0	0	0	0
592	SD	80281201712881	0	0	0	0	0	0	0	0
593	SD	80271202812881	0	0	0	0	0	0	0	0
594	SD	8028120 1 1881	0	0	0	0	0	0	0	0
595	SD	8028120 8 1881	0	0	0	0	0	0	0	0
596	SD	802812017 1881	0	0	0	0	0	0	0	0
597	SD	802712028 1881	0	0	0	0	0	0	0	0
598	SD	8028120 1 2881	0	0	0	0	0	0	0	0

719	8D	5088120 1 8571	288	288	288	282	243	240	236	228
720	8D	5088120 8 8571	218	218	218	212	210	220	223	229
721	8D	508812017 8571	228	228	223	221	208	208	205	202
722	8D	508712028 8571	189	200	210	200	202	202	200	0
723	8D	5088120 1 8571	189	197	193	190	198	202	201	201
724	8D	5088120 8 8571	202	208	208	208	211	218	218	220
725	8D	508812017 8571	218	211	208	208	199	197	194	193
726	8D	508812028 8571	184	182	177	173	171	168	0	0
727	8D	5088120 110571	189	183	182	179	208	208	202	218
728	8D	5088120 910571	224	228	228	225	244	284	284	284
729	8D	50881201710571	289	317	340	343	348	387	384	488
730	8D	50871202810571	449	424	448	478	524	524	514	0
731	8D	5088120 111871	432	370	353	331	337	339	380	238
732	8D	5088120 811871	213	223	238	248	282	282	280	287
733	8D	50881201711871	240	223	188	174	188	142	141	143
734	8D	50881202811871	140	138	138	138	130	122	0	0
735	8D	5088120 112871	128	133	137	137	139	138	137	137
736	8D	5088120 312871	137	138	137	138	139	124	122	119
737	8D	50881201712871	118	118	112	110	110	109	108	110
738	8D	50871202812871	110	109	108	108	108	103	102	0
739	8D	5088120 1 1581	100	88	87	100	102	103	102	102
740	8D	5088120 8 1581	102	100	101	101	102	103	104	101
741	8D	508812017 1581	100	89	88	89	88	88	88	88
742	8D	508712028 1581	100	100	101	101	100	100	89	0
743	8D	5088120 1 2581	84	88	88	87	84	84	84	84
744	8D	5088120 8 2581	82	82	80	81	89	88	85	84
745	8D	508812017 2581	88	85	88	88	83	83	82	83
746	8D	508812028 2581	83	82	81	81	0	0	0	0
747	8D	5088120 1 3581	80	78	78	78	77	78	77	77
748	8D	5088120 8 3581	74	78	78	78	78	78	81	82
749	8D	508812017 3581	84	87	87	89	82	82	82	82
750	8D	508712028 3581	84	84	88	103	106	107	109	0
751	8D	5088120 1 4581	110	128	212	224	489	572	1008	1330
752	8D	5088120 8 4581	1340	1898	2017	1778	1780	1805	1834	1889
753	8D	508812017 4581	1987	2002	2017	2002	1978	1983	1948	1938
754	8D	508812028 4581	1838	1838	1818	1884	1780	1871	0	0
755	8D	5088120 1 5581	1587	1847	1883	1388	1328	1285	1231	1208
756	8D	5088120 8 5581	1587	1117	1083	1028	888	810	880	848
757	8D	508812017 5581	108	781	707	687	682	638	623	603
758	8D	508712028 5581	583	559	549	548	524	518	528	0
759	8D	5088120 1 6581	528	528	528	508	484	472	488	420
760	8D	5088120 8 6581	410	388	372	359	348	338	317	308
761	8D	508812017 6581	298	294	283	274	283	285	270	284
762	8D	508812028 6581	284	243	241	232	228	222	0	0
763	8D	5088120 1 7581	220	214	207	201	190	180	178	173
764	8D	5088120 8 7581	184	187	183	148	137	132	129	128
765	8D	508812017 7581	121	118	110	108	102	101	87	88
766	8D	508712028 7581	83	82	80	88	88	88	0	0
767	8D	5088120 1 8581	84	78	78	78	74	72	78	74
768	8D	5088120 8 8581	73	72	70	68	68	61	58	58
769	8D	508812017 8581	58	58	58	53	51	51	49	49
770	8D	508712028 8581	48	48	50	51	48	48	48	0
771	8D	5088120 1 9581	48	48	38	48	48	41	41	40
772	8D	5088120 8 9581	38	38	38	38	37	37	37	88
773	8D	508812017 9581	118	122	188	184	231	243	248	284
774	8D	508812028 9581	288	201	280	285	248	243	0	0
775	8D	5088120 110581	238	229	228	228	228	228	208	208
776	8D	5088120 810581	189	187	183	182	188	188	188	187
777	8D	50881201710581	184	178	177	178	188	188	182	173
778	8D	50871202810581	184	187	188	138	132	127	128	0

778	8D	5088120 111881	180	148	144	147	180	182	118	132
780	8D	5088120 811881	128	123	138	183	188	148	127	108
781	8D	50881201711881	84	82	88	88	79	73	88	84
782	8D	50881202811881	51	47	43	41	40	40	0	0
783	8D	5088120 112881	40	42	42	43	41	39	38	38
784	8D	5088120 812881	38	34	34	34	34	35	38	38
785	8D	50881201712881	38	38	33	31	32	33	38	38
786	8D	50871202812881	34	38	38	34	33	32	32	0
787	8D	5088120 1 1581	31	30	28	28	27	28	24	24
788	8D	5088120 8 1581	23	23	23	23	23	21	21	21
789	8D	508812017 1581	20	20	18	18	20	20	20	20
790	8D	508712028 1581	20	20	20	21	21	22	22	0
791	8D	5088120 1 2581	22	22	22	22	20	20	20	20
792	8D	5088120 8 2581	20	20	18	18	18	18	18	18
793	8D	508812017 2581	17	17	17	17	18	18	18	18
794	8D	508812028 2581	18	20	21	23	0	0	0	0
795	8D	5088120 1 3581	28	27	28	30	32	33	34	38
796	8D	5088120 8 3581	40	41	40	40	38	38	37	37
797	8D	508812017 3581	37	37	37	37	37	37	37	38
798	8D	508712028 3581	38	48	48	48	48	48	88	0
799	8D	5088120 1 4581	87	103	107	107	108	107	108	114
800	8D	5088120 8 4581	181	288	304	344	388	412	418	417
801	8D	508812017 4581	413	421	460	488	480	488	428	382
802	8D	508812028 4581	288	338	321	308	300	280	0	0
803	8D	5088120 1 5581	274	287	288	287	281	248	244	238
804	8D	5088120 8 5581	238	244	283	274	288	288	288	283
805	8D	508812017 5581	288	277	274	268	248	227	218	214
806	8D	508712028 5581	211	208	201	200	182	180	173	0
807	8D	5088120 1 6581	188	181	180	188	188	188	188	183
808	8D	5088120 8 6581	182	182	188	188	180	181	171	218
809	8D	508812017 6581	224	244	270	380	412	488	483	804
810	8D	508812028 6581	804	804	483	484	471	488	0	0
811	8D	5088120 1 7581	443	433	408	388	428	574	818	862
812	8D	5088120 8 7581	892	732	732	737	737	707	832	818
813	8D	508812017 7581	883	834	473	428	410	381	342	301
814	8D	508712028 7581	278	283	238	228	212	200	180	0
815	8D	5088120 1 8581	178	178	183	188	148	138	138	128
816	8D	5088120 8 8581	128	121	128	128	128	128	118	118
817	8D	508812017 8581	117	117	118	128	130	184	218	222
818	8D	508712028 8581	228	241	287	288	438	712	778	0
819	8D	5088120 1 9581	848	838	1004	1083	1088	1088	1083	1083
820	8D	5088120 8 9581	1023	894	829	870	811	742	702	872
821	8D	508812017 9581	833	898	884	834	489	488	488	484
822	8D	508812028 9581	441	443	471	482	804	844	0	0
823	8D	5088120 110881	883	813	872	892	707	707	702	702
824	8D	5088120 810881	882	872	888	833	882	883	888	888
825	8D	50881201710881	880	874	883	874	888	848	848	838
826	8D	50871202810881	814	804	488	482	488	804	804	0
827	8D	5088120 111881	804	804	488	482	484	478	471	488
828	8D	5088120 811881	808	824	834	484	473	438	402	380
829	8D	50881201711881	398	398	371	382	337	330	323	311
830	8D	50881202811881	288	283	270	280	248	238	0	0
831	8D	5088120 112881	228	218	204	193	184	174	187	181
832	8D	5088120 812881	188	147	138	131	128	117	112	108
833	8D	50881201712881	108	83	80	80	88	88	87	87
834	8D	50871202812881	88	84	83	81	89	81	81	0
834.2	8D	*** 5088002 - WOLF R. AT OUTLET OF WOLF LAKE ***								
835	8D	5088120 1 1881	4	4	4	4	4	4	4	4
836	8D	5088120 8 1881	4	4	4	4	4	4	4	4
837	8D	508812017 1881	4	4	4	4	4	4	4	4

958	8D	508712025 7881	12	12	12	11	11	11	11	0
959	8D	5088120 1 8881	11	10	10	10	10	10	8	8
960	8D	5088120 8 8881	9	9	8	9	9	9	8	8
961	8D	508812017 8881	8	8	7	7	7	7	8	8
962	8D	508712025 8881	8	8	8	8	8	8	8	0
963	8D	5088120 1 8881	8	8	8	8	8	8	8	8
964	8D	5088120 8 8881	8	8	8	8	8	8	8	8
965	8D	508812017 8881	8	7	7	8	10	14	18	18
966	8D	508812025 8881	22	27	28	28	30	30	10	0
967	8D	5088120 110881	31	30	30	29	28	28	27	28
968	8D	5088120 810881	27	26	26	24	24	23	23	23
969	8D	50881201710881	23	22	22	22	22	22	21	21
970	8D	50871202510881	21	22	22	21	20	19	18	0
971	8D	5088120 111881	17	18	18	18	18	18	17	17
972	8D	5088120 811881	17	18	18	18	18	18	17	18
973	8D	50881201711881	18	18	17	18	12	11	11	10
974	8D	50881202511881	10	9	9	7	6	6	0	0
975	8D	5088120 112881	8	8	8	8	8	8	8	8
976	8D	5088120 812881	8	8	8	8	8	8	8	8
977	8D	50881201712881	4	4	4	4	4	4	4	4
978	8D	50871202512881	4	4	4	4	4	4	4	0
979	8D	5088120 1 1881	4	4	4	4	4	4	4	3
980	8D	5088120 8 1881	3	3	3	3	3	3	3	3
981	8D	508812017 1881	3	3	3	3	3	3	3	3
982	8D	508712025 1881	2	2	2	2	2	2	2	0
983	8D	5088120 1 2881	2	2	3	3	3	3	3	3
984	8D	5088120 8 2881	3	2	2	2	2	2	2	2
985	8D	508812017 2881	2	2	2	2	2	2	2	2
986	8D	508712025 2881	2	2	2	2	0	0	0	0
987	8D	5088120 1 3881	2	2	2	3	3	3	3	3
988	8D	5088120 8 3881	4	4	4	4	4	4	4	4
989	8D	508812017 3881	5	5	4	4	4	4	4	4
990	8D	508712025 3881	4	4	4	4	4	5	5	0
991	8D	5088120 1 4881	8	8	8	10	10	12	12	13
992	8D	5088120 8 4881	13	12	12	12	13	18	30	36
993	8D	508812017 4881	40	43	48	48	48	48	49	52
994	8D	508712025 4881	37	37	38	38	46	43	0	0
995	8D	5088120 1 5881	38	38	38	38	33	32	31	31
996	8D	5088120 8 5881	30	29	28	29	28	28	29	31
997	8D	508812017 5881	32	33	34	34	34	33	32	32
998	8D	508712025 5881	31	29	27	28	28	28	24	0
999	8D	5088120 1 6881	24	23	21	21	20	20	19	19
1000	8D	5088120 8 6881	19	18	18	18	18	18	18	18
1001	8D	508812017 6881	18	18	19	20	25	28	29	32
1002	8D	508712025 6881	42	48	54	58	58	59	0	0
1003	8D	5088120 1 7881	59	58	57	58	54	52	49	48
1004	8D	5088120 8 7881	48	50	57	72	77	81	88	88
1005	8D	508812017 7881	88	86	83	74	72	68	62	55
1006	8D	508712025 7881	80	48	48	40	35	32	31	0
1007	8D	5088120 1 8881	28	28	28	23	22	21	21	19
1008	8D	5088120 8 8881	18	17	18	16	15	15	14	18
1009	8D	508812017 8881	15	18	15	14	14	14	14	14
1010	8D	508712025 8881	15	15	23	25	28	27	28	0
1011	8D	5088120 1 9881	31	34	31	32	31	29	110	117
1012	8D	5088120 8 9881	123	127	127	128	128	121	118	108
1013	8D	508812017 9881	102	85	87	82	78	74	70	68
1014	8D	508812025 9881	82	88	87	85	83	82	0	0
1015	8D	5088120 110881	82	85	88	89	84	88	72	79
1016	8D	5088120 810881	81	83	83	82	82	80	78	77
1017	8D	50881201710881	74	89	88	87	85	83	87	88

1018	8D	50871202510881	87	85	84	84	83	80	59	0
1019	8D	5088120 111881	88	88	87	88	88	88	89	88
1020	8D	5088120 811881	84	87	88	85	88	80	81	82
1021	8D	50881201711881	88	88	81	47	44	47	48	43
1022	8D	50881202511881	42	39	39	38	38	35	0	0
1023	8D	5088120 112881	33	32	30	29	28	26	25	24
1024	8D	5088120 812881	23	22	20	20	18	18	17	18
1025	8D	50881201712881	18	18	14	13	13	13	11	11
1026	8D	50871202512881	11	11	10	10	10	10	10	0
1026.2		*** 08AC001 - JACKPISH CREEK NEAR LACOREY ***								
1027	8D	5188120 1 1881	2	2	2	2	2	2	2	2
1028	8D	5188120 8 1881	2	2	2	2	2	2	2	2
1029	8D	518812017 1881	2	2	2	2	2	2	2	2
1030	8D	518712025 1881	2	2	2	2	2	3	3	0
1031	8D	5188120 1 2881	3	3	3	3	3	3	3	3
1032	8D	5188120 8 2881	3	3	3	3	3	3	3	3
1033	8D	518812017 2881	3	3	3	3	3	3	3	3
1034	8D	518812025 2881	3	3	3	3	3	3	3	3
1035	8D	5188120 1 3881	3	3	3	3	3	3	3	3
1036	8D	5188120 8 3881	3	3	3	3	3	3	3	3
1037	8D	518812017 3881	3	3	3	3	3	3	3	3
1038	8D	518712025 3881	3	3	3	3	3	3	3	0
1039	8D	5188120 1 4881	4	4	4	4	4	4	4	4
1040	8D	5188120 8 4881	4	4	4	4	4	4	4	4
1041	8D	518812017 4881	42	71	108	127	118	132	18	30
1042	8D	518812025 4881	213	214	202	185	185	171	208	208
1043	8D	5188120 1 5881	162	158	151	144	138	127	122	117
1044	8D	5188120 8 5881	113	111	108	104	100	85	84	91
1045	8D	518812017 5881	88	88	83	78	77	73	71	88
1046	8D	518712025 5881	84	82	80	78	76	74	74	0
1047	8D	5188120 1 6881	84	82	81	80	48	48	45	44
1048	8D	5188120 8 6881	42	40	38	38	34	32	31	32
1049	8D	518812017 6881	33	38	39	41	44	48	55	78
1050	8D	518812025 6881	86	108	120	135	180	182	0	0
1051	8D	5188120 1 7881	170	177	182	185	179	174	186	183
1052	8D	5188120 8 7881	141	138	127	118	113	106	104	102
1053	8D	518812017 7881	88	88	82	88	88	81	77	72
1054	8D	518712025 7881	88	88	82	82	81	81	80	0
1055	8D	5188120 1 8881	87	85	88	88	81	87	71	73
1056	8D	5188120 8 8881	73	71	69	64	62	57	54	51
1057	8D	518812017 8881	48	48	43	42	40	38	37	38
1058	8D	518712025 8881	34	33	32	31	30	30	28	0
1059	8D	5188120 1 9881	28	29	30	31	32	32	34	35
1060	8D	5188120 8 9881	35	36	36	38	34	33	32	32
1061	8D	518812017 9881	30	30	28	20	28	33	41	52
1062	8D	518812025 9881	82	71	74	78	81	81	0	0
1063	8D	5188120 110881	80	71	67	67	63	59	58	52
1064	8D	5188120 810881	80	48	45	43	41	38	38	38
1065	8D	51881201710881	34	33	32	31	30	29	28	28
1066	8D	51871202510881	27	27	28	24	23	23	22	0
1067	8D	5188120 111881	24	28	28	20	20	32	34	24
1068	8D	5188120 811881	25	28	28	27	28	24	22	21
1069	8D	51881201711881	21	20	19	18	20	18	18	18
1070	8D	51881202511881	18	17	17	17	17	18	0	0
1071	8D	5188120 112881	18	18	18	18	14	13	12	11
1072	8D	5188120 812881	10	8	8	8	8	8	8	8
1073	8D	51881201712881	8	8	8	8	8	8	8	8
1074	8D	51871202512881	8	7	7	7	7	7	7	0
1075	8D	5188120 1 1871	7	7	7	7	7	8	8	8
1076	8D	5188120 8 1871	8	8	8	8	8	8	8	8

1197	8D	818812017 7881	34	31	27	24	24	22	20	17
1198	8D	818712028 7881	18	15	14	13	12	12	11	0
1199	8D	8188120 8 8881	10	10	8	8	8	8	8	7
1200	8D	8188120 8 8881	7	7	7	7	7	7	7	7
1201	8D	818812017 8881	7	7	7	7	8	11	13	13
1202	8D	818712028 8881	13	14	15	17	25	41	45	0
1203	8D	8188120 1 8881	48	54	58	61	63	63	63	63
1204	8D	8188120 8 8881	59	57	54	50	47	43	40	38
1205	8D	818812017 8881	38	34	32	31	28	28	27	25
1206	8D	818812028 8881	25	28	27	28	29	31	0	0
1207	8D	8188120 110881	34	38	38	40	41	41	40	40
1208	8D	8188120 810881	38	38	38	38	34	34	33	32
1209	8D	81881201710881	32	33	34	33	32	32	32	31
1210	8D	81871202810881	30	29	28	28	28	28	28	0
1211	8D	8188120 111881	29	29	29	27	28	27	27	28
1212	8D	8188120 811881	29	30	31	28	27	28	27	23
1213	8D	81881201711881	23	23	21	21	18	19	18	18
1214	8D	81881202811881	17	18	18	18	14	14	0	0
1215	8D	8188120 112881	13	12	12	11	11	10	10	9
1216	8D	8188120 812881	8	8	8	8	7	7	6	6
1217	8D	81881201712881	8	8	8	8	8	8	8	8
1218	8D	81871202812881	8	8	8	8	8	8	8	0
1218.2		*** OSADOOB - BEAVER R. AT COLD LAKE RESERVE ***								
1219	8D	8268120 1 1581	70	74	74	75	77	78	74	74
1220	8D	8268120 8 1581	78	78	74	78	74	74	74	75
1221	8D	826812017 1581	78	78	74	73	74	74	77	78
1222	8D	826712028 1581	79	82	82	85	88	87	91	0
1223	8D	8268120 1 2581	84	84	84	85	84	84	82	82
1224	8D	8268120 8 2581	82	82	82	84	82	82	84	84
1225	8D	826812017 2581	84	85	85	83	85	88	88	83
1226	8D	826812028 2581	82	82	82	84	84	0	0	0
1227	8D	8268120 1 3581	88	87	88	88	88	88	88	88
1228	8D	8268120 8 3581	101	102	105	103	97	98	102	102
1229	8D	826812017 3581	98	98	98	87	87	87	100	104
1230	8D	826712028 3581	102	100	98	87	88	84	82	0
1231	8D	8268120 1 4581	81	83	83	84	84	82	85	118
1232	8D	8268120 8 4581	154	200	288	311	388	484	574	1040
1233	8D	826812017 4581	1820	2480	3700	4810	6050	8040	7240	7240
1234	8D	826812028 4581	7840	7840	7840	8520	8520	8520	8520	0
1235	8D	8268120 1 5581	5880	5880	5880	5880	4730	4480	4280	4120
1236	8D	8268120 8 5581	3880	3880	3880	3870	3800	3330	3300	3200
1237	8D	826812017 5581	3080	2980	2820	2780	2710	2580	2480	2380
1238	8D	826712028 5581	2280	2170	2120	1980	1880	1800	1880	0
1239	8D	8268120 1 6581	1880	1820	1780	1740	1710	1680	1680	1850
1240	8D	8268120 8 6581	1480	1410	1340	1280	1210	1130	1090	1110
1241	8D	826812017 6581	1180	1180	1280	1420	1530	1620	1870	2870
1242	8D	826812028 6581	3370	3810	4220	4730	5280	5880	0	0
1243	8D	8268120 1 7581	8880	8220	8380	8800	8310	8120	8830	8380
1244	8D	8268120 8 7581	4980	4730	4480	4140	3870	3740	3580	3580
1245	8D	826812017 7581	3480	3320	3230	3110	2980	2820	2720	2840
1246	8D	826712028 7581	2400	2320	2180	2180	2180	2180	2110	0
1247	8D	8268120 1 8581	2020	1880	1880	2100	2180	2380	2500	2880
1248	8D	8268120 8 8581	2880	2800	2800	2280	2170	2020	1880	1780
1249	8D	826812017 8581	1880	1880	1820	1480	1420	1380	1310	1240
1250	8D	826712028 8581	1200	1180	1140	1100	1070	1050	1020	0
1251	8D	8268120 1 9581	997	1020	1040	1080	1110	1150	1180	1230
1252	8D	8268120 8 9581	1240	1270	1280	1230	1210	1170	1120	1110
1253	8D	826812017 9581	1080	1040	1000	970	988	1180	1440	1840
1254	8D	826812028 9581	2170	2480	2880	2740	2840	2880	0	0
1255	8D	8268120 110881	2880	2880	2480	2380	2220	2100	1980	1840
1256	8D	8268120 910881	1740	1830	1880	1800	1430	1380	1320	1250
1257	8D	82681201710881	1210	1170	1120	1080	1080	1030	887	878
1258	8D	82671202810881	881	831	885	854	814	798	782	0
1259	8D	8268120 111881	1200	1020	974	1040	1070	1110	1180	828
1260	8D	8268120 811881	881	808	827	858	819	832	781	754
1261	8D	82681201711881	729	704	679	682	681	688	688	641
1262	8D	82681202811881	838	812	888	888	879	888	0	0
1263	8D	8268120 112881	848	848	837	814	888	488	414	380
1264	8D	8268120 811881	3887	313	287	279	277	273	273	268
1265	8D	82681201712881	283	271	271	289	289	288	288	288
1266	8D	82671202812881	288	281	288	289	288	288	288	0
1267	8D	8268120 1 1571	288	288	280	244	228	222	218	218
1268	8D	8268120 8 1571	218	213	187	180	184	173	181	180
1269	8D	826812017 1571	148	140	136	134	134	133	133	128
1270	8D	826712028 1571	117	114	112	107	102	102	101	0
1271	8D	8268120 1 2571	901	88	88	100	133	139	129	122
1272	8D	8268120 8 2571	118	118	118	110	109	108	104	104
1273	8D	826812017 2571	104	88	100	100	88	87	88	88
1274	8D	826812028 2571	78	78	78	78	0	0	0	0
1275	8D	8268120 1 3571	74	74	78	78	78	78	84	81
1276	8D	8268120 8 3571	101	118	118	123	120	117	118	118
1277	8D	826812017 3571	114	113	112	113	113	114	118	118
1278	8D	826712028 3571	118	124	128	128	132	138	182	0
1279	8D	8268120 1 4571	184	178	188	188	328	478	612	612
1280	8D	8268120 8 4571	882	718	691	681	688	648	672	688
1281	8D	826812017 4571	882	718	778	828	882	1510	1880	2070
1282	8D	826812028 4571	1880	2240	2910	2580	2780	2380	0	0
1283	8D	8268120 1 5571	2300	2140	2020	1830	1830	1780	1680	1880
1284	8D	8268120 8 5571	1800	1860	1480	1480	1430	1370	1310	1330
1285	8D	826812017 5571	1310	1300	1280	1280	1210	1200	1110	1080
1286	8D	826712028 5571	1080	1030	884	888	881	827	888	0
1287	8D	8268120 1 6571	888	840	810	782	778	788	716	716
1288	8D	8268120 8 6571	704	707	684	682	678	687	688	648
1289	8D	826812017 6571	628	601	578	588	548	532	525	518
1290	8D	826812028 6571	608	488	468	478	474	471	0	0
1291	8D	8268120 1 7571	471	488	478	481	488	491	494	548
1292	8D	8268120 8 7571	588	588	588	548	528	525	517	512
1293	8D	826812017 7571	818	838	830	822	822	818	822	827
1294	8D	826712028 7571	888	888	888	888	888	888	888	0
1295	8D	8268120 1 8571	543	538	522	508	491	488	474	481
1296	8D	8268120 8 8571	441	441	434	428	424	444	471	484
1297	8D	826812017 8571	481	488	481	448	438	421	414	408
1298	8D	826712028 8571	402	404	424	404	409	408	404	0
1299	8D	8268120 1 9571	402	389	380	388	387	408	407	407
1300	8D	8268120 8 9571	409	414	418	421	428	438	438	444
1301	8D	826812017 9571	438	428	421	414	402	389	382	380
1302	8D	826812028 9571	373	388	388	388	348	338	0	0
1303	8D	8268120 110871	338	330	328	382	418	414	409	438
1304	8D	8268120 910871	484	478	478	478	484	514	543	574
1305	8D	82681201710871	804	841	888	894	887	778	888	843
1306	8D	82671202810871	808	877	900	888	1080	1080	1040	0
1307	8D	8268120 111871	873	748	712	670	682	688	688	481
1308	8D	8268120 811871	431	481	481	488	530	530	528	518
1309	8D	82681201711871	488	481	380	382	320	287	288	288
1310	8D	82681202811871	283	273	278	278	283	248	0	0
1311	8D	8268120 112871	288	288	277	277	279	278	277	277
1312	8D	8268120 812871	277	278	277	278	278	280	248	241
1313	8D	82681201712871	234	232	227	223	222	218	218	222
1314	8D	82671202812871	222	220	218	218	213	208	207	0
1315	8D	8268120 1 1581	202	198	187	202	207	208	207	207

1436	8D	8338120 1 7581	880	1010	1030	1030	1030	1030	1030	1030	1026
1438	8D	8338120 8 7581	1018	1014	1014	1014	1014	1014	1008	857	
1437	8D	833712017 7581	888	882	878	888	882	878	827	827	
1438	8D	833712025 7581	827	827	827	827	827	827	827	0	
1438	8D	8338120 1 8581	827	827	827	827	828	848	858	840	
1440	8D	8338120 8 8581	828	810	888	888	878	888	852	838	
1441	8D	833812017 8581	824	808	794	778	764	768	774	778	
1442	8D	833712025 8581	784	781	778	778	762	748	738	0	
1443	8D	8338120 1 8581	722	688	684	628	628	628	628	628	
1444	8D	8338120 8 8581	628	628	628	632	638	648	682	648	
1445	8D	833812017 8581	638	632	640	680	688	740	740	740	
1446	8D	833812025 8581	740	712	688	681	638	648	0	0	
1447	8D	8338120 110581	688	688	687	644	640	632	632	632	
1448	8D	8338120 810581	632	624	617	608	602	602	602	602	
1449	8D	83381201710581	683	685	677	688	678	668	668	678	
1450	8D	83371202510581	677	678	678	678	678	678	688	0	
1451	8D	8338120 111581	641	624	607	488	484	473	471	470	
1452	8D	8338120 811581	488	488	483	480	487	448	433	421	
1453	8D	83381201711581	410	420	430	403	403	403	403	403	
1454	8D	83381202511581	388	388	383	378	378	372	0	0	
1455	8D	8338120 112581	388	384	380	388	347	338	332	327	
1456	8D	8338120 812581	327	327	327	484	473	462	431	410	
1457	8D	83381201712581	380	388	348	331	334	338	343	348	
1458	8D	83371202512581	383	388	383	388	372	374	377	0	
1459	8D	8338120 1 1571	380	383	388	388	384	378	374	368	
1460	8D	8338120 8 1571	384	388	388	381	348	342	338	338	
1461	8D	833812017 1571	333	331	328	327	328	324	323	322	
1462	8D	833712025 1571	348	348	348	348	348	348	348	0	
1463	8D	8338120 1 2571	381	383	388	387	388	382	388	388	
1464	8D	8338120 8 2571	371	373	378	378	374	373	372	371	
1465	8D	833812017 2571	368	368	373	378	383	378	378	373	
1466	8D	833412025 2571	371	388	388	382	0	0	0	0	
1467	8D	8338120 1 3571	383	383	384	388	387	384	388	371	
1468	8D	8338120 8 3571	373	378	388	380	382	384	387	388	
1469	8D	833812017 3571	383	382	388	382	388	384	383	381	
1470	8D	833712025 3571	348	388	387	387	348	348	348	0	
1471	8D	8338120 1 4571	384	384	383	382	383	383	383	383	
1472	8D	8338120 8 4571	383	383	383	383	383	383	383	388	
1473	8D	833812017 4571	388	388	394	393	382	380	388	388	
1474	8D	833812025 4571	400	408	411	418	424	434	0	0	
1475	8D	8338120 1 5571	444	488	488	478	488	487	607	607	
1476	8D	8338120 8 5571	607	607	607	607	607	607	607	607	
1477	8D	833812017 5571	602	605	608	608	602	608	608	602	
1478	8D	833712025 5571	608	608	612	603	608	608	608	0	
1479	8D	8338120 1 6571	608	603	610	617	628	618	613	608	
1480	8D	8338120 8 6571	602	602	602	602	602	602	602	613	
1481	8D	833812017 6571	628	622	618	618	612	608	608	602	
1482	8D	833812025 6571	688	688	688	688	688	688	0	0	
1483	8D	8338120 1 7571	688	688	681	608	612	618	620	628	
1484	8D	8338120 8 7571	628	628	628	628	628	628	628	620	
1485	8D	833812017 7571	618	618	618	623	690	733	778	784	
1486	8D	833712025 7571	784	784	784	778	788	788	808	0	
1487	8D	8338120 1 8571	808	808	808	808	807	808	803	801	
1488	8D	8338120 8 8571	801	801	801	801	781	783	774	788	
1489	8D	833812017 8571	784	780	748	740	738	731	714	688	
1490	8D	833712025 8571	680	688	688	688	688	688	688	0	
1491	8D	8338120 1 9571	683	681	688	688	672	688	688	688	
1492	8D	8338120 8 9571	688	688	688	684	683	682	682	682	
1493	8D	833812017 9571	631	610	688	688	670	671	672	673	
1494	8D	833812025 9571	678	688	688	648	637	628	0	0	

1495	8D	8338120 110571	534	641	647	688	682	687	682	648
1496	8D	8338120 810571	648	648	648	648	638	628	614	627
1497	8D	83381201710571	641	688	688	648	621	612	603	498
1498	8D	83371202510571	487	487	487	487	487	487	487	0
1499	8D	8338120 111571	487	487	487	487	487	487	477	467
1500	8D	8338120 811571	487	488	488	488	424	424	424	424
1501	8D	83381201711571	418	414	410	403	388	388	383	376
1502	8D	83381202511571	388	382	382	382	382	382	0	0
1503	8D	8338120 112571	382	382	348	330	320	310	313	318
1504	8D	8338120 812571	318	322	308	288	283	308	308	308
1505	8D	83381201712571	308	308	302	288	284	280	247	288
1506	8D	83371202512571	283	281	278	281	283	288	288	0
1507	8D	8338120 1 1581	280	282	288	287	288	301	304	308
1508	8D	8338120 8 1581	308	310	312	312	318	318	318	312
1509	8D	833812017 1581	303	307	312	317	322	327	331	332
1510	8D	833712025 1581	332	332	333	333	334	338	338	0
1511	8D	8338120 1 2581	338	338	337	338	340	341	342	347
1512	8D	8338120 8 2581	352	358	363	388	384	380	388	382
1513	8D	833812017 2581	348	354	388	388	371	377	383	388
1514	8D	833412025 2581	388	388	388	388	0	0	0	0
1515	8D	8338120 1 3581	388	407	418	428	438	444	420	418
1516	8D	8338120 8 3581	403	388	388	387	388	388	384	382
1517	8D	833812017 3581	383	381	378	378	377	378	374	373
1518	8D	833712025 3581	371	370	368	367	384	383	382	0
1519	8D	8338120 1 4581	382	382	382	382	382	382	382	380
1520	8D	8338120 8 4581	388	418	433	481	488	487	513	640
1521	8D	833812017 4581	688	683	618	648	672	687	722	748
1522	8D	833812025 4581	773	798	628	681	684	618	0	0
1523	8D	8338120 1 5581	821	824	827	841	888	987	980	883
1524	8D	8338120 8 5581	1008	1028	1047	1088	1080	1087	1084	1081
1525	8D	833812017 5581	1078	1077	1076	1072	1081	1088	1088	1088
1526	8D	833712025 5581	1100	1102	1104	1108	1118	1124	1138	0
1527	8D	8338120 1 6581	1148	1148	1142	1138	1138	1132	1134	1138
1528	8D	8338120 8 6581	1128	1140	1128	1118	1108	1088	1088	1077
1529	8D	833812017 6581	1088	1088	1084	1047	1030	1008	888	867
1530	8D	833812025 6581	947	943	838	834	830	828	0	0
1531	8D	8338120 1 7581	922	918	814	810	888	880	881	881
1532	8D	8338120 8 7581	881	881	881	828	808	784	788	784
1533	8D	833812017 7581	740	727	718	703	680	678	688	688
1534	8D	833712025 7581	884	880	848	832	818	803	887	0
1535	8D	8338120 1 8581	872	867	843	828	848	862	888	848
1536	8D	8338120 8 8581	841	834	828	824	820	817	814	808
1537	8D	833812017 8581	487	488	480	480	480	480	480	488
1538	8D	833712025 8581	488	418	480	487	488	488	488	0
1539	8D	8338120 1 9581	287	388	383	382	380	388	388	388
1540	8D	8338120 8 9581	388	388	382	338	383	371	388	382
1541	8D	833812017 9581	378	388	382	388	348	342	338	333
1542	8D	833812025 9581	330	327	324	322	318	317	0	0
1543	8D	8338120 110581	314	312	308	308	308	308	308	304
1544	8D	8338120 810581	301	288	288	282	288	287	287	287
1545	8D	83381201710581	287	287	287	287	288	284	283	278
1546	8D	83371202510581	287	288	287	287	284	281	247	0
1547	8D	8338120 111581	247	247	247	243	238	238	231	227
1548	8D	8338120 811581	222	218	213	208	204	200	188	188
1549	8D	83381201711581	184	183	183	182	181	181	181	172
1550	8D	83381202511581	182	183	143	142	141	140	0	0
1551	8D	8338120 112581	138	138	138	138	137	138	138	138
1552	8D	8338120 812581	138	138	138	142	148	188	181	187
1553	8D	83381201712581	173	178	177	178	174	173	170	188
1554	8D	83371202512581	188	188	183	181	188	183	0	

1874	8D	830812025 8871	88	87	88	102	104	102	0	0
1875	8D	8308120 1 7871	109	122	114	108	112	148	182	128
1876	8D	8308120 8 7871	118	113	107	103	102	105	114	128
1877	8D	830812017 7871	120	107	110	112	114	118	139	134
1878	8D	830712025 7871	128	138	138	118	104	103	108	0
1879	8D	8308120 1 8871	104	88	83	88	88	81	83	80
1880	8D	8308120 8 8871	98	81	81	108	128	113	88	100
1881	8D	830812017 8871	98	88	81	83	83	88	88	81
1882	8D	830712025 8871	108	82	87	88	80	80	80	0
1883	8D	8308120 1 8871	84	83	88	107	88	82	88	88
1884	8D	8308120 8 8871	89	88	102	108	104	108	88	88
1885	8D	830812017 8871	88	88	83	88	88	88	77	78
1886	8D	830812025 8871	78	74	77	78	78	78	0	0
1887	8D	8308120 110871	78	103	141	120	89	110	128	128
1888	8D	8308120 810871	118	107	117	131	142	184	180	188
1889	8D	83081201710871	188	170	148	203	285	272	208	143
1890	8D	83071202510871	177	237	308	384	188	78	8	0
1891	8D	8308120 111871	87	100	127	181	82	3	28	87
1892	8D	8308120 811871	132	132	140	132	112	108	87	88
1893	8D	83081201711871	34	34	47	38	88	74	71	82
1894	8D	83081202511871	88	71	82	81	88	81	0	0
1895	8D	8308120 112871	78	74	72	88	88	71	70	88
1896	8D	8308120 812871	70	73	72	80	88	88	84	83
1897	8D	83081201712871	87	87	88	88	88	84	83	88
1898	8D	83071202512871	88	87	87	88	88	84	83	0
1899	8D	8308120 1 1881	83	88	83	81	88	87	88	88
1900	8D	8308120 8 1881	87	88	88	80	81	88	84	84
1901	8D	830812017 1881	88	87	87	88	88	87	88	88
1902	8D	830712025 1881	88	88	87	87	88	88	88	0
1903	8D	8308120 1 2881	88	84	83	83	88	88	83	82
1904	8D	8308120 8 2881	82	84	82	80	48	47	51	83
1905	8D	830812017 2881	82	82	48	48	50	81	81	80
1906	8D	830412025 2881	48	48	48	48	0	0	0	0
1907	8D	8308120 1 3881	48	48	47	48	48	81	81	80
1908	8D	8308120 8 3881	82	80	48	81	83	84	88	88
1909	8D	830812017 3881	88	88	88	87	84	84	88	88
1910	8D	830712025 3881	82	87	88	84	83	88	88	0
1911	8D	8308120 1 4881	204	338	487	820	811	1173	832	1014
1912	8D	8308120 8 4881	1438	721	383	688	783	783	881	882
1913	8D	830812017 4881	774	737	888	883	887	888	708	718
1914	8D	830812025 4881	887	838	472	401	421	448	0	0
1915	8D	8308120 1 5881	418	388	387	374	384	372	372	328
1916	8D	8308120 8 5881	333	388	278	224	228	287	233	184
1917	8D	830812017 5881	172	297	213	388	204	201	188	188
1918	8D	830712025 5881	182	180	183	183	218	211	120	113
1919	8D	8308120 1 6881	211	183	183	182	148	132	120	113
1920	8D	8308120 8 6881	118	123	118	118	108	88	110	118
1921	8D	830812017 6881	108	100	84	108	128	118	88	87
1922	8D	830812025 6881	84	83	80	81	81	88	0	0
1923	8D	8308120 1 7881	83	80	72	88	70	78	88	81
1924	8D	8308120 8 7881	88	82	88	88	88	81	88	83
1925	8D	830812017 7881	80	48	80	88	81	48	81	81
1926	8D	830712025 7881	81	81	82	82	82	47	38	48
1927	8D	8308120 1 8881	41	48	48	44	48	80	48	48
1928	8D	8308120 8 8881	44	42	38	38	37	38	38	38
1929	8D	830812017 8881	40	38	38	37	37	37	38	40
1930	8D	830712025 8881	41	41	37	32	34	37	38	0
1931	8D	8308120 1 9881	37	38	38	33	32	38	38	34
1932	8D	8308120 8 9881	34	38	82	88	82	81	113	113
1933	8D	830812017 9881	120	141	178	188	123	123	127	122

1734	8D	830812025 8881	118	107	88	87	100	88	0	0
1735	8D	8308120 110881	88	100	103	102	78	74	82	84
1736	8D	8308120 810881	84	88	88	81	88	80	88	77
1737	8D	83081201710881	78	81	88	88	82	70	82	81
1738	8D	83071202510881	84	84	88	84	88	88	80	0
1739	8D	8308120 111881	87	78	81	80	28	48	74	88
1740	8D	8308120 811881	84	88	88	88	37	18	28	43
1741	8D	83081201711881	48	48	38	38	32	21	31	31
1742	8D	83081202511881	28	30	34	38	38	38	0	0
1743	8D	8308120 112881	38	38	38	31	32	31	30	31
1744	8D	8308120 812881	31	32	34	34	34	34	34	34
1745	8D	83081201712881	30	28	30	38	37	38	32	34
1746	8D	83071202512881	34	31	31	30	28	28	27	0
1747	8D	8308120 1 1881	27	28	23	21	21	22	21	21
1748	8D	8308120 8 1881	23	22	20	19	20	19	18	18
1749	8D	830812017 1881	18	17	20	20	20	18	18	20
1750	8D	830712025 1881	20	21	22	22	21	21	21	0
1751	8D	8308120 1 2881	21	21	18	17	18	20	18	18
1752	8D	8308120 8 2881	17	17	18	18	18	18	18	18
1753	8D	830812017 2881	17	17	18	18	18	18	18	20
1754	8D	830812025 2881	23	28	27	28	0	0	0	0
1755	8D	8308120 1 3881	33	33	32	34	38	41	42	38
1756	8D	8308120 8 3881	37	38	33	34	33	33	34	38
1757	8D	830812017 3881	38	34	38	38	38	38	38	38
1758	8D	830712025 3881	47	44	41	40	80	80	77	0
1759	8D	8308120 1 4881	80	84	88	80	88	70	122	280
1760	8D	8308120 8 4881	288	238	218	238	228	178	183	178
1761	8D	830812017 4881	322	282	237	171	108	83	80	83
1762	8D	830812025 4881	82	108	103	82	83	104	0	0
1763	8D	8308120 1 5881	107	103	88	101	88	100	114	104
1764	8D	8308120 8 5881	142	180	148	139	137	131	113	104
1765	8D	830812017 5881	108	108	81	83	70	88	80	87
1766	8D	830712025 5881	83	88	87	88	74	88	87	0
1767	8D	8308120 1 6881	71	78	73	78	77	73	72	78
1768	8D	8308120 8 6881	78	80	84	88	84	188	188	138
1769	8D	830812017 6881	188	271	313	288	288	242	223	202
1770	8D	830812025 6881	187	174	187	188	180	130	0	0
1771	8D	8308120 1 7881	130	132	183	418	438	380	248	388
1772	8D	8308120 8 7881	323	280	280	243	138	183	178	128
1773	8D	830812017 7881	77	81	104	112	74	47	82	77
1774	8D	830712025 7881	84	88	74	87	87	80	88	0
1775	8D	8308120 1 8881	88	88	88	88	82	87	88	87
1776	8D	8308120 8 8881	87	78	72	88	84	87	81	81
1777	8D	830812017 8881	88	78	81	183	180	128	112	127
1778	8D	830712025 8881	188	183	288	288	840	881	528	0
1779	8D	8308120 1 9881	843	818	488	448	418	328	278	278
1780	8D	8308120 8 9881	247	208	180	184	181	181	178	180
1781	8D	830812017 9881	187	147	133	148	188	181	181	188
1782	8D	830812025 9881	222	248	238	270	312	311	0	0
1783	8D	8308120 110881	388	348	310	288	287	288	248	232
1784	8D	8308120 810881	231	207	187	180	188	188	211	242
1785	8D	83081201710881	288	228	188	182	207	203	188	188
1786	8D	83071202510881	188	188	188	218	218	202	202	0
1787	8D	8308120 111881	188	142	180	203	177	238	242	238
1788	8D	8308120 811881	338	188	128	122	83	87	182	178
1789	8D	83081201711881	124	117	107	108	123	113	100	84
1790	8D	83081202511881	83	81	84	80	83	79	0	0
1791	8D	8308120 112881	73	88	87	84	84	88	84	88
1792	8D	8308120 812881	84	82	88	48	47	83	88	38
1793	8D	83081201712881	34	80	84	81	48	81	80	48

1913	80	907812017 8581	212	211	202	198	189	193	199	198
1914	80	907812025 8581	188	180	177	187	184	181	0	0
1915	80	9078120 1 7581	181	158	181	148	138	127	129	123
1916	80	9078120 9 7581	118	111	110	104	98	82	91	89
1917	80	907812017 7581	88	83	78	74	72	72	69	68
1918	80	907712025 7581	82	87	88	88	88	88	88	0
1919	80	9078120 1 8581	82	85	88	84	83	82	88	88
1920	80	9078120 9 8581	85	84	82	80	87	84	42	41
1921	80	907812017 8581	40	39	40	39	37	37	38	38
1922	80	907712025 8581	38	37	38	39	37	34	33	0
1923	80	9078120 1 8581	33	32	33	34	34	30	30	29
1924	80	9078120 9 8581	28	28	29	28	48	83	62	80
1925	80	907812017 8581	110	118	144	187	210	214	213	218
1926	80	907812025 8581	214	205	201	198	189	181	0	0
1927	80	9078120 110881	178	187	184	187	185	187	184	181
1928	80	9078120 910881	144	143	139	143	140	138	139	141
1929	80	90781201710881	138	133	132	131	141	142	139	130
1930	80	90771202510881	122	112	100	85	80	87	81	0
1931	80	9078120 111881	118	112	112	116	119	118	83	87
1932	80	9078120 811881	91	87	102	128	124	118	88	72
1933	80	90781201711881	80	88	84	88	83	80	37	33
1934	80	90781202511881	30	28	28	28	27	27	0	0
1935	80	9078120 112881	29	31	32	32	31	29	28	28
1936	80	9078120 912881	24	24	24	25	25	28	28	27
1937	80	90781201712881	27	27	28	23	24	25	27	27
1938	80	90771202512881	27	27	27	28	28	24	23	0
1939	80	9078120 1 1581	23	22	21	20	19	18	17	17
1940	80	9078120 9 1581	18	18	17	17	18	18	18	18
1941	80	907812017 1581	18	18	14	14	15	18	18	18
1942	80	907712025 1581	18	18	19	18	17	17	17	0
1943	80	9078120 1 2581	17	17	17	17	18	18	18	18
1944	80	9078120 9 2581	18	18	14	14	14	14	12	13
1945	80	907812017 2581	12	12	13	13	14	14	14	15
1946	80	907812025 2581	15	16	17	19	0	0	0	0
1947	80	9078120 1 3581	20	22	25	25	28	27	27	31
1948	80	9078120 9 3581	22	23	23	22	20	29	27	27
1949	80	907812017 3581	27	28	28	28	28	40	28	29
1950	80	907712025 3581	30	28	28	40	40	78	0	0
1951	80	9078120 1 4581	78	81	85	85	85	85	81	88
1952	80	9078120 9 4581	125	229	278	318	338	378	384	384
1953	80	907812017 4581	330	333	381	385	389	370	325	284
1954	80	907812025 4581	248	217	208	206	205	192	0	0
1955	80	9078120 1 5581	183	180	180	185	183	182	180	174
1956	80	9078120 9 5581	178	184	203	218	228	231	238	230
1957	80	907812017 5581	219	208	208	195	175	158	181	148
1958	80	907712025 5581	147	147	148	148	131	128	123	0
1959	80	9078120 1 6581	120	113	118	118	114	118	117	114
1960	80	9078120 9 6581	114	116	118	119	123	124	134	181
1961	80	907812017 6581	187	205	231	319	360	411	424	438
1962	80	907812025 6581	417	405	381	388	380	344	0	0
1963	80	9078120 1 7581	322	304	289	280	313	487	518	564
1964	80	9078120 9 7581	887	820	888	888	878	840	487	442
1965	80	907812017 7581	408	387	303	273	281	241	213	187
1966	80	907712025 7581	173	184	143	143	140	134	127	0
1967	80	9078120 1 8581	118	121	112	107	88	87	84	80
1968	80	9078120 9 8581	88	88	81	85	88	88	80	88
1969	80	907812017 8581	88	88	88	87	101	188	191	184
1970	80	907712025 8581	188	210	220	242	385	857	718	0
1971	80	9078120 1 9581	781	858	888	882	901	884	886	882
1972	80	9078120 9 9581	780	732	857	806	547	493	483	447

1973	80	907812017 9581	423	402	385	385	357	334	328	318
1974	80	907812025 9581	312	323	384	378	388	438	0	0
1975	80	9078120 110781	478	488	584	571	578	587	554	540
1976	80	9078120 910781	518	502	488	484	424	418	407	401
1977	80	90781201710781	407	431	443	437	424	414	411	398
1978	80	90771202510781	378	388	387	380	388	380	383	0
1979	80	9078120 1 11581	384	388	382	341	383	384	380	379
1980	80	9078120 911581	388	407	420	380	383	318	278	281
1981	80	90781201711581	280	282	285	285	248	234	228	222
1982	80	90781202511581	209	202	180	182	170	184	0	0
1983	80	9078120 1 12581	187	181	141	134	127	120	118	112
1984	80	9078120 912581	108	103	98	91	87	80	77	76
1985	80	90781201712581	78	82	82	83	83	81	84	85
1986	80	90771202512581	83	82	82	80	88	80	80	0
1986.2	80	***	***	***	***	***	***	***	***	***
1987	80	9008120 1 1581	-33	-33	-33	-33	-33	-33	-33	-33
1988	80	9008120 9 1581	-33	-33	-33	-33	-33	-33	-33	-33
1989	80	900812017 1581	-33	-33	-33	-33	-33	-33	-33	-33
1990	80	900712025 1581	1748	30	-33	-33	-33	-33	-33	1891
1991	80	9008120 1 2581	-85	-85	-85	-85	-85	-85	-85	-85
1992	80	9008120 9 2581	188	188	188	114	80	80	8	114
1993	80	900812017 2581	-85	-85	-85	-85	1448	158	-85	-85
1994	80	900412025 2581	-85	-85	-85	-85	-85	-85	-85	-85
1995	80	9008120 1 3581	-85	-85	2381	803	118	-85	-85	-85
1996	80	9008120 9 3581	-85	-85	-32	10	-85	-85	708	-85
1997	80	900812017 3581	-85	-85	-85	74	-85	-85	74	-85
1998	80	900712025 3581	815	540	180	-85	74	10	-85	0
1999	80	9008120 1 4581	-287	-287	-287	80	-287	-287	-287	-287
2000	80	9008120 9 4581	80	-287	-287	-287	-287	-287	-287	-287
2001	80	900812017 4581	-287	-287	-287	-287	-287	-287	-287	-287
2002	80	900812025 4581	-287	-287	-287	-287	-287	-287	-287	-287
2003	80	9008120 1 5581	-409	-409	-409	-409	-409	-409	-409	-409
2004	80	9008120 9 5581	-409	-409	-134	-409	120	-409	-409	-409
2005	80	900812017 5581	-409	-409	-409	-409	-409	-409	-409	-409
2006	80	900712025 5581	-409	-409	-409	-409	1780	-409	-409	-409
2007	80	9008120 1 6581	-877	-877	-877	-877	-877	-877	-877	-877
2008	80	9008120 9 6581	-877	-877	-877	-877	-877	-877	-877	-877
2009	80	900812017 6581	-814	-877	-877	-877	4318	3187	-877	3133
2010	80	900812025 6581	-877	-877	288	-877	783	339	0	0
2011	80	9008120 1 7581	-780	-780	-780	-780	-780	130	-483	-780
2012	80	9008120 9 7581	-780	-483	-780	4878	-811	-780	-387	-148
2013	80	900812017 7581	-780	-780	-780	-780	-780	-780	-780	-878
2014	80	900712025 7581	3822	-780	2228	288	-780	-780	-780	0
2015	80	9008120 1 8581	-488	-802	1082	2418	-802	-802	-802	-802
2016	80	9008120 9 8581	-802	-802	-802	-802	-802	-802	-802	-802
2017	80	900812017 8581	-802	-802	-802	-802	-802	-802	-802	-802
2018	80	900712025 8581	-833	-802	870	-802	-802	-802	-802	85
2019	80	9008120 1 9581	2184	1009	-788	-484	-788	-788	-788	-788
2020	80	9008120 9 9581	-788	-788	-788	-884	-788	-800	-788	-788
2021	80	900812017 9581	-788	-788	-788	-788	8280	-788	-788	-788
2022	80	900812025 9581	-788	-788	-788	-788	-788	-788	0	0
2023	80	9008120 1 10581	-483	-702	-483	-483	-483	-483	-483	-483
2024	80	9008120 9 10581	-483	-483	-483	-483	-483	-483	-483	-483
2025	80	90081201710581	-483	-483	-483	-483	-483	-483	-483	-483
2026	80	90071202510581	278	48	48	-483	-483	-314	-483	0
2027	80	9008120 1 11581	-88	-88	-88	-88	891	-88	-88	-88
2028	80	9008120 9 11581	-28	-88	-88	804	293	-28	-88	-88
2029	80	90081201711581	1308	293	-88	-88	17	-88	-88	-88
2030	80	90081202511581	-88	-88	-88	-88	-88	0	0	0
2031	80	9008120 1 12581	84	-41	681	170	-41	-41	-41	832

2152	SD	8008120 8 8591	-190	7220	-878	-147	-878	2880	-878	827
2153	SD	800812017 8591	818	-878	-878	-878	-878	-190	-878	-878
2154	SD	800812025 8591	-878	-878	-878	-878	-878	-190	-878	-878
2155	SD	8008120 1 7591	-778	8303	-188	-813	22	-878	0	0
2156	SD	8008120 8 7591	407	-778	-778	-778	-365	-778	-778	-778
2157	SD	800812017 7591	-778	-778	-778	-778	2018	-778	-778	-778
2158	SD	800712025 7591	-778	-887	-887	-803	-778	-778	-778	0
2159	SD	8008120 1 8591	-800	-800	-38	-888	-800	-800	-800	-208
2160	SD	8008120 8 8591	-800	3455	173	-800	-800	-800	-800	-271
2161	SD	800812017 8591	-800	854	-737	703	2311	218	-800	-888
2162	SD	800712025 8591	-208	3842	1401	1020	-800	-800	218	0
2163	SD	8008120 1 9591	-788	-788	-327	-788	-788	-788	-788	-788
2164	SD	8008120 8 9591	-788	-788	-788	-788	-788	-788	-788	-788
2165	SD	800812017 8591	-788	489	-899	-899	-483	-883	-788	-788
2166	SD	800812025 8591	-387	1118	-387	-788	-387	-788	0	0
2167	SD	8008120 110591	183	-482	-482	-377	-418	-482	-482	-482
2168	SD	8008120 810591	3487	840	-482	-418	-377	-207	-482	-482
2169	SD	80081201710591	-482	-482	-482	-482	-482	-418	-482	-482
2170	SD	80071202510591	-482	-482	4	-271	-313	-377	-482	0
2171	SD	8008120 111591	718	283	-88	-88	-88	335	-88	-88
2172	SD	8008120 811591	-88	-88	-88	-88	17	-88	-88	-88
2173	SD	80081201711591	388	-88	-88	-88	885	-88	-88	885
2174	SD	80081202511591	-88	-88	-88	-88	-88	-88	0	0
2175	SD	8008120 112591	-41	-41	-41	22	84	-41	-41	-41
2176	SD	8008120 812591	-41	-41	-41	-41	-41	32	-41	-41
2177	SD	80081201712591	-41	-41	-41	-41	-41	-41	-41	-41
2178	SD	80071202512591	857	783	170	-41	-41	-41	-41	0
2178.2		*** COMPUTED NET LOCAL 131 TO PRIMROSE L ***								
2179	SD	1318120 1 1591	-1	732	877	700	723	881	182	-2
2180	SD	1318120 8 1591	-3	139	-31	148	142	146	291	133
2181	SD	131812017 1591	284	285	146	444	448	448	447	375
2182	SD	131712025 1591	380	448	482	7	-142	8	4	0
2183	SD	1318120 1 2591	4	4	4	3	279	301	302	303
2184	SD	1318120 8 2591	444	285	148	149	182	182	183	183
2185	SD	131812017 2591	188	188	188	188	87	146	188	188
2186	SD	131412025 2591	7	188	8	8	184	0	0	0
2187	SD	1318120 1 3591	7	188	88	277	147	305	305	187
2188	SD	1318120 8 3591	308	187	188	183	187	187	128	187
2189	SD	131812017 3591	308	307	308	301	309	309	303	310
2190	SD	131712025 3591	128	307	181	182	185	188	480	0
2191	SD	1318120 1 4591	488	488	470	488	324	474	474	875
2192	SD	1318120 8 4591	482	328	178	172	320	188	184	287
2193	SD	131812017 4591	131	88	200	11	183	1811	1874	1814
2194	SD	131812025 4591	1848	1881	1898	1887	483	817	0	0
2195	SD	1318120 1 5591	842	848	883	884	878	889	847	771
2196	SD	1318120 8 5591	778	733	778	788	700	729	888	728
2197	SD	131812017 5591	748	781	788	427	420	383	439	441
2198	SD	131712025 5591	448	538	281	287	218	1718	1812	0
2199	SD	1318120 1 6591	1748	1842	343	334	388	-409	-411	-418
2200	SD	1318120 8 6591	-418	870	788	880	883	888	845	845
2201	SD	131812017 6591	354	382	347	2882	2883	2778	2940	4086
2202	SD	131812025 6591	4155	4308	1128	1154	1089	1105	0	0
2203	SD	1318120 1 7591	1882	1788	1788	433	443	418	484	188
2204	SD	1318120 817591	88	83	807	212	812	823	-97	-43
2205	SD	131812017 7591	-88	111	42	-108	-888	-704	489	0
2206	SD	131712025 7591	338	808	385	489	810	808	810	0
2207	SD	1318120 1 8591	803	817	443	388	1115	1112	1178	-489
2208	SD	1318120 8 8591	-808	-813	-247	-178	-284	-182	-820	-825
2209	SD	131812017 8591	-484	-837	-844	-878	-880	778	782	885
2210	SD	131712025 8591	880	288	188	321	-487	-494	-447	0

2211	SD	1318120 1 9591	-888	-2138	-2038	-2084	382	380	388	388
2212	SD	1318120 8 9591	388	387	388	838	887	880	883	-208
2213	SD	131812017 9591	-210	-1320	1081	1218	882	8094	417	401
2214	SD	131812025 9591	388	-1808	-1488	-1808	-1820	1070	0	0
2215	SD	1318120 110591	887	1119	-415	-414	-331	-332	339	342
2216	SD	1318120 810591	348	-324	-244	-330	-280	338	338	339
2217	SD	13181201710591	-415	-337	-340	-427	1182	1283	-1428	1184
2218	SD	13171202510591	218	227	227	333	334	327	-1092	0
2219	SD	1318120 111591	-1138	-1142	-1181	-880	-784	-872	72	188
2220	SD	1318120 811591	80	81	80	30	-28	-788	-788	-774
2221	SD	13181201711591	-782	1048	1088	-2034	211	217	217	217
2222	SD	13181202511591	-370	-374	-283	-129	-47	-132	0	0
2223	SD	1318120 112591	-83	-218	-181	-147	-880	-884	-811	-887
2224	SD	1318120 812591	128	147	188	14719	-1811	-1482	-1808	-1818
2225	SD	13181201712591	-1448	-1842	-1470	-1873	831	889	810	813
2226	SD	13171202512591	818	814	822	708	481	378	488	0
2227	SD	1318120 1 1571	484	484	488	470	-218	-212	-287	-207
2228	SD	1318120 8 1571	-210	-213	-132	-138	-230	-138	-140	-87
2229	SD	131812017 1571	-108	-107	-108	-111	36	-111	-112	1873
2230	SD	131712025 1571	1728	200	181	200	200	200	200	0
2231	SD	1318120 1 2671	483	371	389	389	373	485	480	483
2232	SD	1318120 8 2671	480	382	488	125	133	130	132	131
2233	SD	131812017 2671	47	208	880	823	838	-117	-47	-80
2234	SD	131412028 2671	48	-38	-83	-40	0	0	0	0
2235	SD	1318120 1 3671	287	207	281	381	289	289	300	382
2236	SD	1318120 8 3671	374	428	379	388	389	380	478	383
2237	SD	131812017 3671	312	388	313	314	381	38	148	82
2238	SD	131712025 3671	80	143	142	226	141	140	223	0
2239	SD	1318120 1 4671	148	228	143	224	222	221	214	212
2240	SD	1318120 8 4671	212	208	210	213	214	214	214	717
2241	SD	131812017 4671	804	138	133	124	127	20	87	838
2242	SD	131812025 4671	884	871	871	788	871	1088	0	0
2243	SD	1318120 1 5671	1040	1128	1088	1104	1081	1184	1088	288
2244	SD	1318120 8 5671	288	287	289	283	238	1782	1831	1889
2245	SD	131812017 5671	1889	884	884	321	878	877	882	829
2246	SD	131712025 5671	882	888	878	-412	-272	321	338	0
2247	SD	1318120 1 6671	348	838	839	843	1023	-138	-138	-88
2248	SD	1318120 8 6671	-144	388	388	348	388	388	387	1280
2249	SD	131812017 6671	1271	81	88	118	3	104	88	88
2250	SD	131812025 6671	-23	182	324	343	384	389	0	0
2251	SD	1318120 1 7671	332	338	718	783	874	822	874	724
2252	SD	1318120 8 7671	380	377	314	380	381	214	288	213
2253	SD	131812017 7671	87	377	380	3318	3221	3288	3332	-338
2254	SD	131712025 7671	480	480	388	1188	1208	1188	1208	0
2255	SD	1318120 1 8671	480	481	481	470	387	232	338	384
2256	SD	1318120 8 8671	488	378	484	-184	-88	-133	-134	-134
2257	SD	131812017 8671	-138	87	122	113	104	181	-710	-828
2258	SD	131712025 8671	-847	-738	385	410	410	122	0	0
2259	SD	1318120 1 8671	328	241	227	2047	-170	-98	-237	386
2260	SD	1318120 8 8671	408	308	237	236	318	317	304	382
2261	SD	131812017 8671	-1383	-1374	-1368	-1388	812	428	439	640
2262	SD	131812025 8671	828	-481	-402	-481	-413	-418	0	0
2263	SD	1318120 110571	832	888	781	8784	848	-81	-88	-8
2264	SD	1318120 810571	328	328	328	328	-882	-808	-813	1383
2265	SD	13181201710571	1884	1881	1413	-1808	-1740	-472	-482	-387
2266	SD	13171202510571	-381	277	277	288	272	280	273	0
2267	SD	1318120 111571	282	287	288	270	270	286	-874	-871
2268	SD	1318120 811571	-878	-872	-888	-881	223	223	80	181
2269	SD	13181201711571	-184	-308	-108	-388	-384	-384	-384	-388
2270	SD	13181202511571	-373	-377	188	208	207	172	0	0

2390	80	133712025 8561	292	388	174	172	251	1212	1279	0
2391	80	1338120 8 8561	1216	1287	201	212	203	-343	-348	-348
2392	80	1338120 9 8561	-390	438	802	448	447	481	493	882
2393	80	133812017 8561	213	206	204	2122	2208	2081	2134	0
2394	80	133812025 8561	2951	3971	829	768	822	818	0	0
2395	80	1338120 1 7561	1170	1222	1230	288	272	311	300	89
2396	80	1338120 9 7561	30	82	319	821	333	320	-83	-80
2397	80	133812017 7561	-112	32	-18	31	31	-852	-858	318
2398	80	133712025 7561	488	320	428	388	321	320	321	0
2399	80	1338120 1 8561	331	321	388	427	784	782	801	-414
2400	80	1338120 8 8561	-420	-428	-232	-183	-220	-188	-420	-432
2401	80	133812017 8561	-388	-442	-447	-432	-458	810	813	487
2402	80	133712025 8561	873	132	188	178	-408	-411	-287	0
2403	80	1338120 1 8561	-271	-1488	-1828	-1828	211	209	208	0
2404	80	1338120 9 8561	204	208	308	842	874	848	847	-201
2405	80	133812017 8561	-203	-144	708	830	1012	4282	251	240
2406	80	133812025 8561	231	-1141	-1108	-1142	-1182	728	0	0
2407	80	1338120 110881	880	848	-338	-338	-278	-278	210	213
2408	80	1338120 910881	218	-388	-211	-274	-218	208	209	210
2409	80	13381201710881	-338	-378	-281	-344	807	873	-1087	808
2410	80	13371202510881	170	182	182	208	207	213	-828	0
2411	80	1338120 111881	-828	-828	-828	-478	-808	-888	81	120
2412	80	1338120 911881	80	87	-8	14	8	-892	-887	-882
2413	80	13381201711881	-488	778	771	-1474	188	188	188	188
2414	80	13381202511881	-288	-272	-213	-88	-38	-97	0	0
2415	80	1338120 112881	-38	-181	-82	-88	-408	-411	-373	-378
2416	80	1338120 912881	178	184	131	10888	-1080	-1084	-1084	-1102
2417	80	13381201712881	-1080	-1120	-1087	-1142	488	424	429	441
2418	80	13371202512881	442	448	447	810	321	271	340	0
2419	80	1338120 1 1871	334	338	337	338	-138	-88	-182	0
2420	80	1338120 9 1871	-184	-188	-97	-88	-181	-102	-103	-84
2421	80	133812017 1871	-78	-78	-80	-78	28	-82	-83	1210
2422	80	133712025 1371	1280	142	180	143	143	143	143	0
2423	80	1338120 1 2871	324	388	288	270	288	328	329	331
2424	80	1338120 9 2871	328	272	338	100	83	84	82	81
2425	80	133812017 2871	30	184	394	487	488	-84	-14	-14
2426	80	133812025 2871	31	-38	-18	-32	0	0	0	0
2427	80	1338120 1 3871	397	182	212	288	208	208	208	270
2428	80	1338120 9 3871	280	388	280	272	274	278	327	277
2429	80	133812017 3871	218	278	318	218	322	82	84	37
2430	80	133712025 3871	38	88	88	188	84	84	184	0
2431	80	1338120 1 4871	88	148	87	188	144	143	142	138
2432	80	1338120 9 4871	138	134	138	137	138	138	138	802
2433	80	133812017 4871	888	81	79	83	78	-3	83	882
2434	80	133812025 4871	412	488	388	530	488	728	0	0
2435	80	1338120 1 5871	730	814	748	887	783	827	772	188
2436	80	1338120 9 5871	188	170	171	188	200	1288	1288	1380
2437	80	133812017 5871	1382	388	480	208	383	384	388	488
2438	80	133712025 5871	422	408	488	-322	-183	232	220	0
2439	80	1338120 1 6871	212	828	848	848	718	-138	-141	-82
2440	80	1338120 9 6871	-148	288	217	327	218	218	218	887
2441	80	133812017 6871	882	107	-18	48	44	8	43	80
2442	80	133812025 6871	18	88	248	232	223	219	0	0
2443	80	1338120 1 7871	241	238	874	821	880	838	488	823
2444	80	1338120 9 7871	322	224	278	222	322	102	170	101
2445	80	133812017 7871	207	220	242	2347	2330	2388	2388	-288
2446	80	133712025 7871	280	280	344	818	818	847	843	0
2447	80	1338120 1 8871	388	388	288	317	202	308	218	200
2448	80	1338120 9 8871	287	388	318	-180	-72	-147	-184	0
2449	80	133812017 8871	-188	-18	31	38	78	-847	-478	0

2450	80	133812025 8871	-828	-488	281	241	241	241	224	0
2451	80	1338120 1 8871	180	118	128	1888	-178	-117	-133	288
2452	80	1338120 9 8871	238	188	117	118	178	174	318	241
2453	80	133812017 8871	-1042	-1081	-1088	-1088	328	273	283	283
2454	80	133812025 8871	324	-403	-347	-411	-384	-388	0	0
2455	80	1338120 110871	881	871	800	8408	888	-88	-88	-39
2456	80	1338120 910871	202	202	202	202	-828	-472	-478	878
2457	80	13381201710871	1041	1048	890	-1188	-1227	-348	-388	-318
2458	80	13371202510871	-317	187	187	171	183	174	184	0
2459	80	1338120 111871	183	187	302	190	188	180	-412	-418
2460	80	1338120 911871	-422	-482	-488	-488	173	173	288	202
2461	80	13381201711871	-138	-122	-74	-283	-288	-283	-208	-248
2462	80	13381202511871	-277	-278	182	143	143	178	0	0
2463	80	1338120 112871	181	223	-888	-1010	-831	-982	448	488
2464	80	1338120 912871	470	480	-1274	-1278	-1288	2888	121	122
2465	80	13381201712871	122	148	-288	-271	-282	-300	-208	-102
2466	80	13371202512871	-88	-88	-104	328	348	438	328	0
2467	80	1338120 1 1881	330	331	440	333	334	338	443	337
2468	80	1338120 9 1881	338	388	382	481	343	342	481	-880
2469	80	133812017 1881	-848	888	888	884	788	888	882	288
2470	80	133712025 1881	138	132	243	137	283	303	182	0
2471	80	1338120 1 2881	210	138	212	233	281	200	188	442
2472	80	1338120 9 2881	444	847	487	818	-184	-88	-97	-93
2473	80	133812017 2881	-101	808	481	878	813	818	818	820
2474	80	133812025 2881	180	180	188	188	0	0	0	0
2475	80	1338120 1 3881	718	707	721	724	783	774	-831	-872
2476	80	1338120 9 3881	-888	-888	88	88	88	84	84	88
2477	80	133812017 3881	184	32	31	88	100	82	28	88
2478	80	133712025 3881	27	88	80	38	34	60	187	0
2479	80	1338120 1 4881	127	138	182	128	-118	107	81	1188
2480	80	1338120 9 4881	1174	1117	1187	1182	1134	1260	1704	1778
2481	80	133812017 4881	1738	1828	1732	1813	1788	1478	1422	1480
2482	80	133812025 4881	1488	1801	1820	1828	1884	2000	0	0
2483	80	1338120 1 5881	418	488	481	1000	1038	818	874	880
2484	80	1338120 9 5881	880	1338	1418	1428	1484	388	388	288
2485	80	133812017 5881	308	308	210	310	787	818	888	488
2486	80	133712025 5881	477	843	830	488	883	1011	1088	0
2487	80	1338120 1 6881	1081	288	288	218	288	271	833	838
2488	80	1338120 9 6881	838	838	-180	-100	-104	-148	-17	-71
2489	80	133812017 6881	-128	-108	840	-384	-428	-831	-840	-848
2490	80	133812025 6881	-808	183	181	283	188	202	0	0
2491	80	1338120 1 7881	183	201	180	181	-388	-384	-400	-387
2492	80	1338120 9 7881	318	318	318	-801	-781	-770	-438	-443
2493	80	133812017 7881	-402	-388	-318	-320	-273	-282	-308	-383
2494	80	133712025 7881	-48	-2	-83	-83	-821	-888	-748	0
2495	80	1338120 1 8881	-702	-708	-843	-872	1217	1338	-237	-223
2496	80	1338120 9 8881	-207	-238	-172	-88	-80	8	-2	-388
2497	80	133812017 8881	-311	-378	-318	170	184	188	188	-747
2498	80	133712025 8881	-888	-780	-827	-772	17	138	10	0
2499	80	1338120 1 9881	121	12	11	71	10	88	130	130
2500	80	1338120 9 9881	180	130	-1808	-1487	8128	1283	1214	-288
2501	80	133812017 9881	-188	-308	-308	-318	-320	-282	-288	-178
2502	80	133812025 9881	-218	-228	-228	-122	-230	-124	0	0
2503	80	1338120 110881	-214	-112	-221	-222	-100	180	118	-118
2504	80	1338120 910881	-222	-224	-228	-227	-228	-121	84	84
2505	80	13381201710881	84	104	104	84	-122	-18	-18	-770
2506	80	13371202510881	-772	-237	-348	-238	-240	-241	-380	0
2507	80	1338120 111881	84	84	101	-337	-238	-238	-341	-343
2508	80	1338120 911881	-482	-388	-488	-382	-481	-388	-483	77
2509	80	13381201711881	-32	-33	78	-22	-34	74	-1	-888

APPENDIX B

VERIFICATION PLOTS - BEAVER RIVER AT COLD LAKE

This Appendix contains graphical output from the SSARR model for the Beaver River at Cold Lake.

Three hydrographs for the Beaver River at Cold Lake are presented in this Appendix for the period 1956 to 1980:

1. Computed streamflows - these flows were computed by routing the local area runoff flows through the lake and channel system to the downstream location, as described in the text.
2. Recorded streamflows - from Water Survey of Canada records.
3. Simulation error - the cumulative difference between the recorded and computed streamflows is plotted as inches of runoff over the basin. The simulation error is calculated with the dummy reservoirs as described in the text. This graph indicates that the computed streamflows exceeded the recorded streamflows by about 1.5 inches over the 25-year period, or 2.6% of the recorded runoff.

PLOT STATION NAME
CHARACTER

STATION
NUMBER CONTROL

C-CALC BEAVER RIVER FLOW AT COLD LAKE
R-BEAVER R AT COLD LAKE 0840008

726.0 0

FLOW CFS

0

1500

3000

4500

6000

7500

9000

10500

12000

13500

15000

*-DUMMY RES (DIFF) AT BEAVER RIVER AT COLD LA

726.2 8

ELEV FT

-5

-4

-3

-2

-1

0

1

2

3

4

5

6

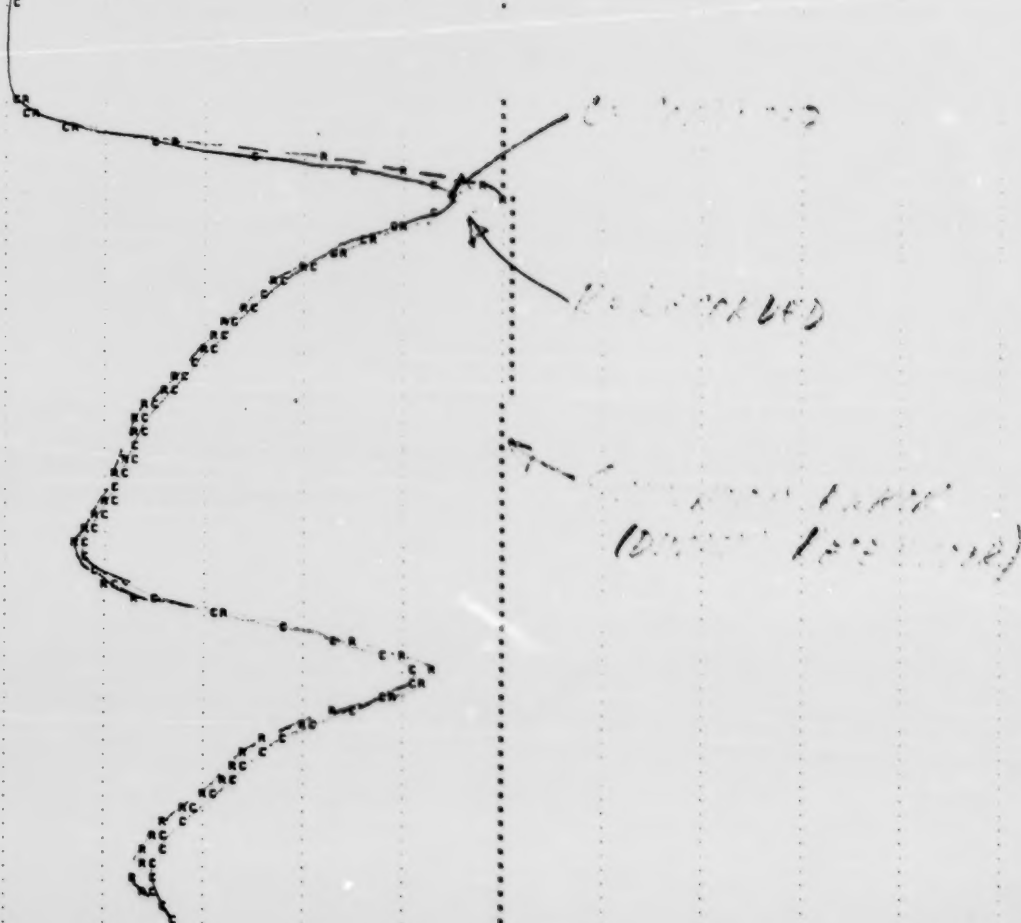
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1 JAN 58 1200 RC
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6 AUG 58 1200 RC
8 AUG 58 1200 RC



A diagram showing a curved path of points labeled 'C' and 'RC' on a grid. The path starts at the bottom left, moves up and right, then curves around to the top right, and finally moves down and left. The labels 'C' and 'RC' are placed at various points along this path.

[illegible]

7	APR	57	1200	
8	APR	57	1200	CR
11	APR	57	1200	
13	APR	57	1200	CR
16	APR	57	1200	CR
17	APR	57	1200	C
18	APR	57	1200	
21	APR	57	1200	RC
23	APR	57	1200	
25	APR	57	1200	C
27	APR	57	1200	RC
9	APR	57	1200	RC
1	MAY	57	1200	C
3	MAY	57	1200	
5	MAY	57	1200	C
7	MAY	57	1200	C
9	MAY	57	1200	C
11	MAY	57	1200	C
13	MAY	57	1200	C
15	MAY	57	1200	
17	MAY	57	1200	C
18	MAY	57	1200	C
21	MAY	57	1200	C
23	MAY	57	1200	RC
25	MAY	57	1200	C
27	MAY	57	1200	C
29	MAY	57	1200	RC
31	MAY	57	1200	C
2	JUN	57	1200	C
4	JUN	57	1200	RC
6	JUN	57	1200	C
8	JUN	57	1200	
10	JUN	57	1200	C
12	JUN	57	1200	C
14	JUN	57	1200	RC
16	JUN	57	1200	
18	JUN	57	1200	C
20	JUN	57	1200	C
22	JUN	57	1200	C
24	JUN	57	1200	RC
26	JUN	57	1200	C
28	JUN	57	1200	C
30	JUN	57	1200	C
2	JUL	57	1200	C
4	JUL	57	1200	C
6	JUL	57	1200	C
8	JUL	57	1200	CR
10	JUL	57	1200	C
12	JUL	57	1200	
14	JUL	57	1200	RC
16	JUL	57	1200	C
18	JUL	57	1200	CR
20	JUL	57	1200	C
22	JUL	57	1200	RC
24	JUL	57	1200	C
26	JUL	57	1200	C
28	JUL	57	1200	C
30	JUL	57	1200	C
1	AUG	57	1200	C
3	AUG	57	1200	RC

5	AUG	57	1200	C
7	AUG	57	1200	C
9	AUG	57	1200	C
11	AUG	57	1200	C
13	AUG	57	1200	C
15	AUG	57	1200	C
17	AUG	57	1200	C
19	AUG	57	1200	C
21	AUG	57	1200	C
23	AUG	57	1200	C
25	AUG	57	1200	C
27	AUG	57	1200	C
29	AUG	57	1200	C
31	AUG	57	1200	C
2	SEP	57	1200	C
4	SEP	57	1200	C
6	SEP	57	1200	C
10	SEP	57	1200	C
12	SEP	57	1200	C
14	SEP	57	1200	C
16	SEP	57	1200	C
18	SEP	57	1200	C
20	SEP	57	1200	C
22	SEP	57	1200	C
24	SEP	57	1200	C
26	SEP	57	1200	C
30	SEP	57	1200	C
2	OCT	57	1200	C
4	OCT	57	1200	C
6	OCT	57	1200	CR
8	OCT	57	1200	C
10	OCT	57	1200	C
12	OCT	57	1200	C
14	OCT	57	1200	C
16	OCT	57	1200	CR
18	OCT	57	1200	C
20	OCT	57	1200	CR
22	OCT	57	1200	C
24	OCT	57	1200	CR
26	OCT	57	1200	C
13	NOV	57	1200	C
30	OCT	57	1200	CR
1	NOV	57	1200	C
3	NOV	57	1200	C
5	NOV	57	1200	CR
7	NOV	57	1200	C
9	NOV	57	1200	CR
11	NOV	57	1200	C
13	NOV	57	1200	CR
15	NOV	57	1200	C
17	NOV	57	1200	C
19	NOV	57	1200	C
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23	NOV	57	1200	C
25	NOV	57	1200	C
27	NOV	57	1200	C
29	NOV	57	1200	C
1	DEC	57	1200	C

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11 FEB 58 1200 RC
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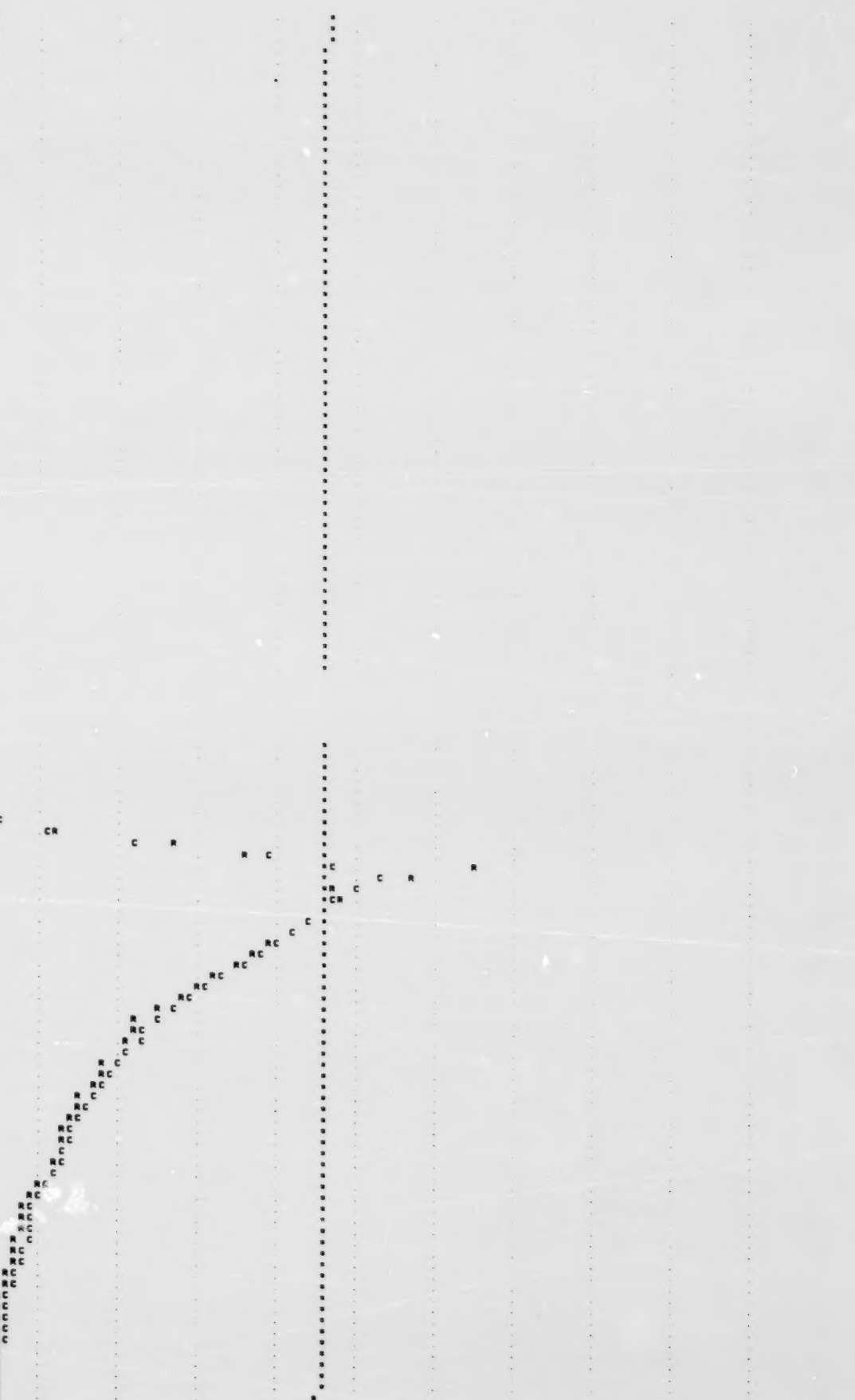
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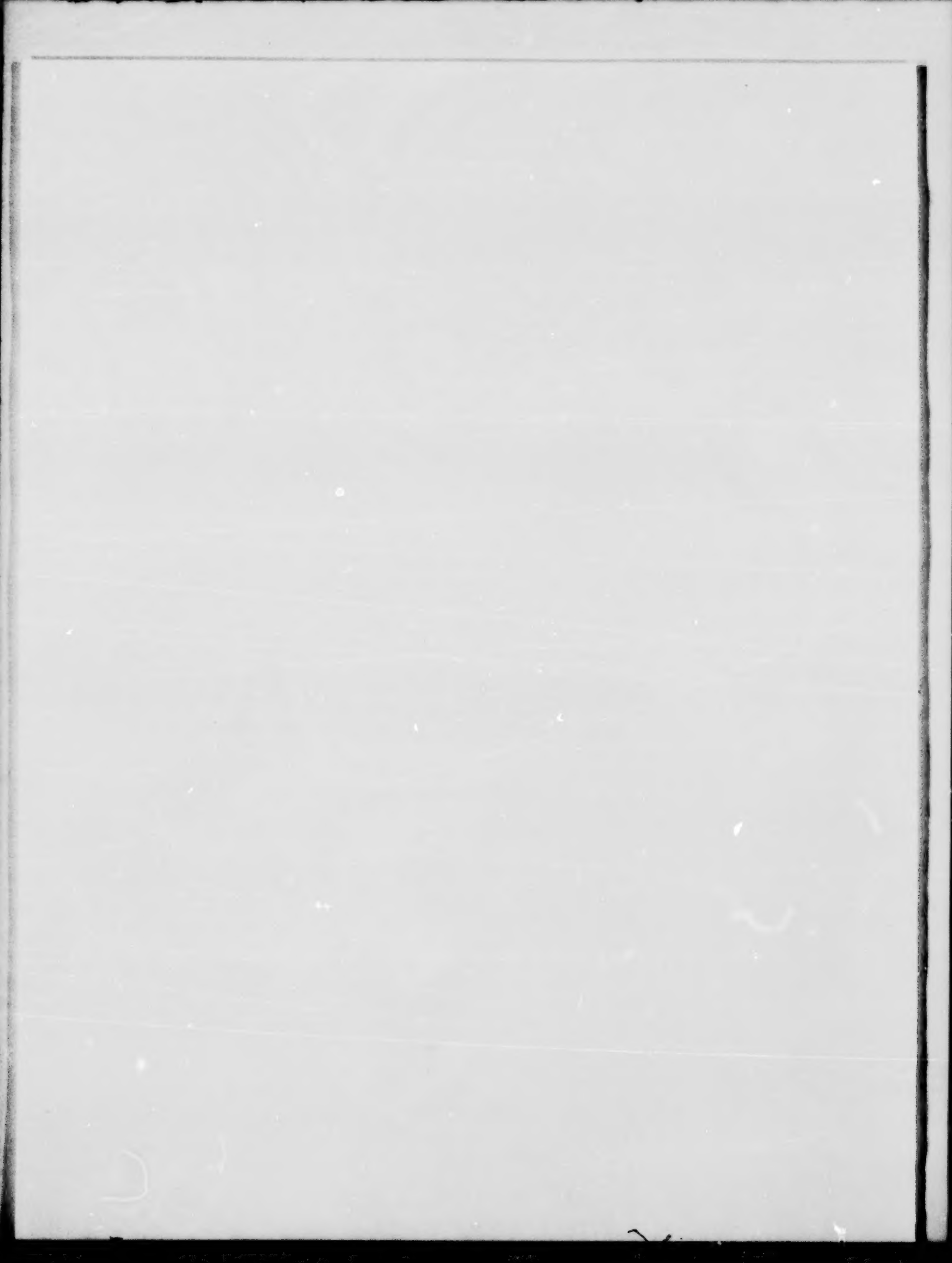
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5	APR	67	1200	C
7	APR	67	1200	C
9	APR	67	1200	C
11	APR	67	1200	C
13	APR	67	1200	C
15	APR	67	1200	C
17	APR	67	1200	C
19	APR	67	1200	C
21	APR	67	1200	C
23	APR	67	1200	C
25	APR	67	1200	C
27	APR	67	1200	C
29	APR	67	1200	C
1	MAY	67	1200	C
3	MAY	67	1200	C
5	MAY	67	1200	C
7	MAY	67	1200	C
9	MAY	67	1200	C
11	MAY	67	1200	C
13	MAY	67	1200	C
15	MAY	67	1200	C
17	MAY	67	1200	C
19	MAY	67	1200	C
21	MAY	67	1200	C
23	MAY	67	1200	C
25	MAY	67	1200	C
27	MAY	67	1200	C
29	MAY	67	1200	C
31	MAY	67	1200	C
1	JUN	67	1200	C
4	JUN	67	1200	C
6	JUN	67	1200	C
8	JUN	67	1200	C
10	JUN	67	1200	C
12	JUN	67	1200	C
14	JUN	67	1200	C
16	JUN	67	1200	C
18	JUN	67	1200	C
20	JUN	67	1200	C
22	JUN	67	1200	C
24	JUN	67	1200	C
26	JUN	67	1200	C
28	JUN	67	1200	C
30	JUN	67	1200	C
2	JUL	67	1200	C
4	JUL	67	1200	C
6	JUL	67	1200	C
8	JUL	67	1200	C
10	JUL	67	1200	C
12	JUL	67	1200	C
14	JUL	67	1200	C
16	JUL	67	1200	C
18	JUL	67	1200	C
20	JUL	67	1200	C
22	JUL	67	1200	C
24	JUL	67	1200	C

28 JUL 87 1200 C
28 JUL 87 1200 C
30 JUL 87 1200 C
1 AUG 87 1200 C
3 AUG 87 1200 CR
5 AUG 87 1200 CR
7 AUG 87 1200 CR
9 AUG 87 1200 CR
11 AUG 87 1200 C
13 AUG 87 1200 C
15 AUG 87 1200 C
17 AUG 87 1200 C
19 AUG 87 1200 C
21 AUG 87 1200 C
23 AUG 87 1200 C
25 AUG 87 1200 C
27 AUG 87 1200 C
29 AUG 87 1200 CR
31 AUG 87 1200 C
2 SEP 87 1200 C
4 SEP 87 1200 C
6 SEP 87 1200 C
8 SEP 87 1200 C
10 SEP 87 1200 C
12 SEP 87 1200 C
14 SEP 87 1200 C
16 SEP 87 1200 C
18 SEP 87 1200 C
20 SEP 87 1200 C
22 SEP 87 1200 C
24 SEP 87 1200 C
26 SEP 87 1200 C
28 SEP 87 1200 C
30 SEP 87 1200 C
2 OCT 87 1200 C
4 OCT 87 1200 C
6 OCT 87 1200 C
8 OCT 87 1200 C
10 OCT 87 1200 C
12 OCT 87 1200 C
14 OCT 87 1200 C
16 OCT 87 1200 C
18 OCT 87 1200 C
20 OCT 87 1200 C
22 OCT 87 1200 C
24 OCT 87 1200 C
26 OCT 87 1200 C
28 OCT 87 1200 C
30 OCT 87 1200 C
1 NOV 87 1200 C
3 NOV 87 1200 C
5 NOV 87 1200 C
7 NOV 87 1200 C
9 NOV 87 1200 C
11 NOV 87 1200 C
13 NOV 87 1200 C
15 NOV 87 1200 C
17 NOV 87 1200 C
19 NOV 87 1200 C
21 NOV 87 1200 C

23 NOV 87 1200 C
25 NOV 87 1200 C
27 NOV 87 1200 C
29 NOV 87 1200 C
1 DEC 87 1200 CR
3 DEC 87 1200 CR
5 DEC 87 1200 C
7 DEC 87 1200 C
9 DEC 87 1200 C
11 DEC 87 1200 C
13 DEC 87 1200 C
15 DEC 87 1200 C
17 DEC 87 1200 C
19 DEC 87 1200 C
21 DEC 87 1200 C
23 DEC 87 1200 C
25 DEC 87 1200 C
27 DEC 87 1200 C
29 DEC 87 1200 C
31 DEC 87 1200 C

[illegible]

[illegible]

8 DEC 72 1200 RC
10 DEC 72 1200 RC
12 DEC 72 1200 RC
14 DEC 72 1200 RC
16 DEC 72 1200 RC
18 DEC 72 1200 RC
20 DEC 72 1200 RC
22 DEC 72 1200 RC
24 DEC 72 1200 RC
26 DEC 72 1200 C
28 DEC 72 1200 C
30 DEC 72 1200 C
1 JAN 73 1200 C
3 JAN 73 1200 C
5 JAN 73 1200 C
7 JAN 73 1200 C
9 JAN 73 1200 C
11 JAN 73 1200 C
13 JAN 73 1200 C
15 JAN 73 1200 RC
17 JAN 73 1200 C
19 JAN 73 1200 C
21 JAN 73 1200 C
23 JAN 73 1200 C
25 JAN 73 1200 C
27 JAN 73 1200 C
29 JAN 73 1200 C
31 JAN 73 1200 C
2 FEB 73 1200 C
4 FEB 73 1200 C
6 FEB 73 1200 C
8 FEB 73 1200 C
10 FEB 73 1200 C
12 FEB 73 1200 C
14 FEB 73 1200 C
16 FEB 73 1200 C
18 FEB 73 1200 C
20 FEB 73 1200 C
22 FEB 73 1200 C
24 FEB 73 1200 C
26 FEB 73 1200 CR
28 FEB 73 1200 CR
2 MAR 73 1200 CR
4 MAR 73 1200 C
6 MAR 73 1200 C
8 MAR 73 1200 C
10 MAR 73 1200 C
12 MAR 73 1200 C
14 MAR 73 1200 C
16 MAR 73 1200 C
18 MAR 73 1200 C
20 MAR 73 1200 CR
22 MAR 73 1200 CR
24 MAR 73 1200 C
26 MAR 73 1200 C
28 MAR 73 1200 C
30 MAR 73 1200 C
1 APR 73 1200 C
3 APR 73 1200 CR
5 APR 73 1200 C

7 APR 73 1200 RC
9 APR 73 1200 RC
11 APR 73 1200 C
13 APR 73 1200 CR
15 APR 73 1200 C
17 APR 73 1200 C
19 APR 73 1200 C
21 APR 73 1200 C
23 APR 73 1200 C
25 APR 73 1200 C
27 APR 73 1200 C
29 APR 73 1200 C
1 MAY 73 1200 C
3 MAY 73 1200 C
5 MAY 73 1200 RC
7 MAY 73 1200 C
9 MAY 73 1200 C
11 MAY 73 1200 C
13 MAY 73 1200 RC
15 MAY 73 1200 RC
17 MAY 73 1200 RC
19 MAY 73 1200 RC
21 MAY 73 1200 RC
23 MAY 73 1200 RC
25 MAY 73 1200 C
27 MAY 73 1200 RC
29 MAY 73 1200 RC
31 MAY 73 1200 C
2 JUN 73 1200 C
4 JUN 73 1200 RC
6 JUN 73 1200 C
8 JUN 73 1200 RC
10 JUN 73 1200 RC
12 JUN 73 1200 RC
14 JUN 73 1200 RC
16 JUN 73 1200 RC
18 JUN 73 1200 RC
20 JUN 73 1200 C
22 JUN 73 1200 C
24 JUN 73 1200 C
26 JUN 73 1200 C
28 JUN 73 1200 RC
30 JUN 73 1200 C
2 JUL 73 1200 C
4 JUL 73 1200 C
6 JUL 73 1200 RC
8 JUL 73 1200 RC
10 JUL 73 1200 C
12 JUL 73 1200 C
14 JUL 73 1200 RC
16 JUL 73 1200 RC
18 JUL 73 1200 C
20 JUL 73 1200 C
22 JUL 73 1200 RC
24 JUL 73 1200 RC
26 JUL 73 1200 RC
28 JUL 73 1200 C
30 JUL 73 1200 C
1 AUG 73 1200 RC
3 AUG 73 1200 RC

A diagram showing a vertical sequence of letters (R, C, E) arranged in a zig-zag pattern, representing a specific sequence or structure.

[illegible]

28	NOV	74	1200	RC
30	NOV	74	1200	RC
2	DEC	74	1200	RC
4	DEC	74	1200	RC
6	DEC	74	1200	RC
8	DEC	74	1200	RC
10	DEC	74	1200	RC
12	DEC	74	1200	RC
14	DEC	74	1200	RC
16	DEC	74	1200	RC
8	DEC	74	1200	RC
20	DEC	74	1200	RC
22	DEC	74	1200	C
24	DEC	74	1200	C
26	DEC	74	1200	C
28	DEC	74	1200	C
30	DEC	74	1200	C
1	JAN	75	1200	C
3	JAN	75	1200	RC
5	JAN	75	1200	RC
7	JAN	75	1200	RC
9	JAN	75	1200	RC
11	JAN	75	1200	RC
13	JAN	75	1200	RC
15	JAN	75	1200	RC
17	JAN	75	1200	RC
19	JAN	75	1200	RC
21	JAN	75	1200	RC
23	JAN	75	1200	RC
25	JAN	75	1200	C
27	JAN	75	1200	C
31	JAN	75	1200	C
2	FEB	75	1200	C
4	FEB	75	1200	RC
6	FEB	75	1200	RC
8	FEB	75	1200	RC
10	FEB	75	1200	C
12	FEB	75	1200	C
14	FEB	75	1200	C
16	FEB	75	1200	C
18	FEB	75	1200	C
20	FEB	75	1200	C
22	FEB	75	1200	C
24	FEB	75	1200	RC
26	FEB	75	1200	RC
28	FEB	75	1200	C
2	MAR	75	1200	C
4	MAR	75	1200	C
6	MAR	75	1200	RC
8	MAR	75	1200	RC
10	MAR	75	1200	RC
12	MAR	75	1200	RC
14	MAR	75	1200	RC
16	MAR	75	1200	RC
18	MAR	75	1200	C
20	MAR	75	1200	C
22	MAR	75	1200	C
24	MAR	75	1200	C
26	MAR	75	1200	C

20	MAR	75	1200	RC
30	MAR	75	1200	RC
1	APR	75	1200	RC
3	APR	75	1200	RC
5	APR	75	1200	RC
7	APR	75	1200	RC
9	APR	75	1200	RC
11	APR	75	1200	RC
13	APR	75	1200	C
15	APR	75	1200	C
17	APR	75	1200	RC
19	APR	75	1200	C
21	APR	75	1200	
23	APR	75	1200	
25	APR	75	1200	
27	APR	75	1200	
29	APR	75	1200	
1	MAY	75	1200	
3	MAY	75	1200	
5	MAY	75	1200	
7	MAY	75	1200	
9	MAY	75	1200	
11	MAY	75	1200	
13	MAY	75	1200	
15	MAY	75	1200	
17	MAY	75	1200	
19	MAY	75	1200	
21	MAY	75	1200	
23	MAY	75	1200	
25	MAY	75	1200	
27	MAY	75	1200	
29	MAY	75	1200	
31	MAY	75	1200	
2	JUN	75	1200	
4	JUN	75	1200	
6	JUN	75	1200	
8	JUN	75	1200	
10	JUN	75	1200	
12	JUN	75	1200	
14	JUN	75	1200	
16	JUN	75	1200	
18	JUN	75	1200	
20	JUN	75	1200	
22	JUN	75	1200	
24	JUN	75	1200	
26	JUN	75	1200	
28	JUN	75	1200	
2	JUL	75	1200	
4	JUL	75	1200	
6	JUL	75	1200	
8	JUL	75	1200	
10	JUL	75	1200	
12	JUL	75	1200	
14	JUL	75	1200	
16	JUL	75	1200	
18	JUL	75	1200	
20	JUL	75	1200	
22	JUL	75	1200	
24	JUL	75	1200	

A scatter plot showing the relationship between the number of children and the number of children who are not in the sample. The x-axis is labeled "Number of children" and ranges from 0 to 10. The y-axis is labeled "Number of children not in sample" and ranges from 0 to 10. The data points are represented by "C" and "R" characters. A diagonal line from (0,0) to (10,10) is shown. Points are clustered along this line, with "C" points generally above and "R" points generally below the line.

PLOT CHARACTER		STATION NAME		STATION NUMBER CONTROL	
		C-CALC BEAVER RIVER FLOW AT COLD LAKE			
		R-BEAVER R AT COLD LAKE 0540005			
FLOW CFS	0	1500	3000	4500	6000
				7500	9000
				10500	12000
					13500
					15000
ELEV FT	-5	-4	-3	-2	-1
1 JAN 75 1200	C				
3 JAN 75 1200	C				
5 JAN 75 1200	C				
7 JAN 75 1200	C				
9 JAN 75 1200	C				
11 JAN 75 1200	C				
13 JAN 75 1200	C				
15 JAN 75 1200	C				
17 JAN 75 1200	C				
19 JAN 75 1200	C				
21 JAN 75 1200	C				
23 JAN 75 1200	C				
25 JAN 75 1200	C				
27 JAN 75 1200	C				
29 JAN 75 1200	C				
31 JAN 75 1200	C				
2 FEB 75 1200	C				
4 FEB 75 1200	C				
6 FEB 75 1200	C				
8 FEB 75 1200	C				
10 FEB 75 1200	C				
12 FEB 75 1200	C				
14 FEB 75 1200	C				
16 FEB 75 1200	C				
18 FEB 75 1200	C				
20 FEB 75 1200	C				
22 FEB 75 1200	C				
24 FEB 75 1200	C				
26 FEB 75 1200	C				
28 FEB 75 1200	RC				
1 MAR 75 1200	RC				
3 MAR 75 1200	RC				
5 MAR 75 1200	RC				
7 MAR 75 1200	RC				
9 MAR 75 1200	RC				
11 MAR 75 1200	RC				
13 MAR 75 1200	RC				
15 MAR 75 1200	RC				
17 MAR 75 1200	RC				
19 MAR 75 1200	RC				
21 MAR 75 1200	RC				
23 MAR 75 1200	RC				
25 MAR 75 1200	C				
27 MAR 75 1200	C				
29 MAR 75 1200	C				
31 MAR 75 1200	C				
3 APR 75 1200	C				
5 APR 75 1200	C				
7 APR 75 1200	C				
9 APR 75 1200	C				
10 APR 75 1200	C				

A scatter plot showing the relationship between the number of children and the number of children who are also parents. The x-axis is labeled 'Number of children' and ranges from 0 to 10. The y-axis is labeled 'Number of children who are also parents' and ranges from 0 to 10. The data points are represented by letters: 'C' for children only and 'RC' for children who are also parents. The points form a diagonal line from (0,0) to (10,10), with some points deviating from the line, particularly at higher values.

[illegible]

8 DEC 76 1200 .RC
10 DEC 76 1200 .RC
12 DEC 76 1200 .RC
14 DEC 76 1200 .RC
16 DEC 76 1200 .RC
18 DEC 76 1200 .RC
20 DEC 76 1200 .RC
22 DEC 76 1200 .RC
24 DEC 76 1200 .RC
26 DEC 76 1200 .RC
28 DEC 76 1200 .C
30 DEC 76 1200 .C
1 JAN 77 1200 .C
3 JAN 77 1200 .C
5 JAN 77 1200 .C
7 JAN 77 1200 .C
9 JAN 77 1200 .C
11 JAN 77 1200 .C
13 JAN 77 1200 .C
15 JAN 77 1200 .C
17 JAN 77 1200 .C
19 JAN 77 1200 .C
21 JAN 77 1200 .C
23 JAN 77 1200 .C
25 JAN 77 1200 .C
27 JAN 77 1200 .C
29 JAN 77 1200 .C
31 JAN 77 1200 .C
2 FEB 77 1200 .C
4 FEB 77 1200 .C
6 FEB 77 1200 .C
8 FEB 77 1200 .C
10 FEB 77 1200 .C
12 FEB 77 1200 .C
14 FEB 77 1200 .C
16 FEB 77 1200 .C
18 FEB 77 1200 .C
20 FEB 77 1200 .C
22 FEB 77 1200 .C
24 FEB 77 1200 .C
26 FEB 77 1200 .C
28 FEB 77 1200 .C
29 FEB 77 1200 .C
2 MAR 77 1200 .C
4 MAR 77 1200 .C
6 MAR 77 1200 .C
8 MAR 77 1200 .C
10 MAR 77 1200 .C
12 MAR 77 1200 .C
14 MAR 77 1200 .C
16 MAR 77 1200 .C
18 MAR 77 1200 .C
20 MAR 77 1200 .C
22 MAR 77 1200 .C
24 MAR 77 1200 .C
26 MAR 77 1200 .C
28 MAR 77 1200 .C
30 MAR 77 1200 .C
1 APR 77 1200 .C
3 APR 77 1200 .C
5 APR 77 1200 .C

7 APR 77 1200 .C
9 APR 77 1200 .RC
11 APR 77 1200 .RC
13 APR 77 1200 .RC
15 APR 77 1200 .RC
17 APR 77 1200 .RC
19 APR 77 1200 .RC
21 APR 77 1200 .RC
23 APR 77 1200 .RC
25 APR 77 1200 .RC
27 APR 77 1200 .RC
29 APR 77 1200 .RC
1 MAY 77 1200 .RC
3 MAY 77 1200 .RC
5 MAY 77 1200 .RC
7 MAY 77 1200 .RC
9 MAY 77 1200 .RC
11 MAY 77 1200 .RC
13 MAY 77 1200 .RC
15 MAY 77 1200 .RC
17 MAY 77 1200 .RC
19 MAY 77 1200 .RC
21 MAY 77 1200 .RC
23 MAY 77 1200 .RC
25 MAY 77 1200 .RC
27 MAY 77 1200 .RC
29 MAY 77 1200 .RC
31 MAY 77 1200 .RC
2 JUN 77 1200 .RC
4 JUN 77 1200 .RC
6 JUN 77 1200 .RC
8 JUN 77 1200 .RC
10 JUN 77 1200 .RC
12 JUN 77 1200 .RC
14 JUN 77 1200 .RC
16 JUN 77 1200 .RC
18 JUN 77 1200 .RC
20 JUN 77 1200 .RC
22 JUN 77 1200 .RC
24 JUN 77 1200 .RC
26 JUN 77 1200 .RC
28 JUN 77 1200 .RC
30 JUN 77 1200 .RC
2 JUL 77 1200 .RC
4 JUL 77 1200 .RC
6 JUL 77 1200 .RC
8 JUL 77 1200 .RC
10 JUL 77 1200 .RC
12 JUL 77 1200 .RC
14 JUL 77 1200 .RC
16 JUL 77 1200 .RC
18 JUL 77 1200 .RC
20 JUL 77 1200 .RC
22 JUL 77 1200 .RC
24 JUL 77 1200 .RC
26 JUL 77 1200 .RC
28 JUL 77 1200 .RC
30 JUL 77 1200 .RC
1 AUG 77 1200 .RC
3 AUG 77 1200 .RC

5	AUG	77	1200	CR
7	AUG	77	1200	C
9	AUG	77	1200	C
11	AUG	77	1200	C
13	AUG	77	1200	CR
15	AUG	77	1200	C
17	AUG	77	1200	CR
19	AUG	77	1200	C
21	AUG	77	1200	C
23	AUG	77	1200	C
25	AUG	77	1200	C
27	AUG	77	1200	C
29	AUG	77	1200	C
31	AUG	77	1200	CR
2	SEP	77	1200	CR
4	SEP	77	1200	CR
6	SEP	77	1200	C
8	SEP	77	1200	C
10	SEP	77	1200	C
12	SEP	77	1200	C
14	SEP	77	1200	C
16	SEP	77	1200	C
18	SEP	77	1200	C
20	SEP	77	1200	CR
22	SEP	77	1200	CR
24	SEP	77	1200	CR
26	SEP	77	1200	CR
28	SEP	77	1200	CR
30	SEP	77	1200	C
2	OCT	77	1200	C
4	OCT	77	1200	CR
6	OCT	77	1200	CR
8	OCT	77	1200	CR
10	OCT	77	1200	CR
12	OCT	77	1200	CR
14	OCT	77	1200	C
16	OCT	77	1200	C
18	OCT	77	1200	C
20	OCT	77	1200	CR
22	OCT	77	1200	CR
24	OCT	77	1200	CR
26	OCT	77	1200	C
28	OCT	77	1200	C
30	OCT	77	1200	C
1	NOV	77	1200	C
3	NOV	77	1200	CR
5	NOV	77	1200	CR
7	NOV	77	1200	CR
9	NOV	77	1200	C
11	NOV	77	1200	C
13	NOV	77	1200	C
15	NOV	77	1200	RC
17	NOV	77	1200	RC
19	NOV	77	1200	C
21	NOV	77	1200	RC
23	NOV	77	1200	RC
25	NOV	77	1200	RC
27	NOV	77	1200	RC
29	NOV	77	1200	RC
1	DEC	77	1200	RC

3	DEC	77	1200	RC
5	DEC	77	1200	RC
7	DEC	77	1200	RC
9	DEC	77	1200	RC
11	DEC	77	1200	RC
13	DEC	77	1200	RC
15	DEC	77	1200	RC
17	DEC	77	1200	RC
19	DEC	77	1200	RC
21	DEC	77	1200	RC
23	DEC	77	1200	RC
25	DEC	77	1200	RC
27	DEC	77	1200	RC
29	DEC	77	1200	RC
31	DEC	77	1200	RC
2	JAN	78	1200	RC
4	JAN	78	1200	RC
6	JAN	78	1200	RC
8	JAN	78	1200	RC
10	JAN	78	1200	RC
12	JAN	78	1200	C
14	JAN	78	1200	C
16	JAN	78	1200	C
18	JAN	78	1200	C
20	JAN	78	1200	C
22	JAN	78	1200	C
24	JAN	78	1200	C
26	JAN	78	1200	C
28	JAN	78	1200	C
30	JAN	78	1200	C
1	FEB	78	1200	C
3	FEB	78	1200	C
5	FEB	78	1200	C
7	FEB	78	1200	C
9	FEB	78	1200	C
11	FEB	78	1200	C
13	FEB	78	1200	C
15	FEB	78	1200	C
17	FEB	78	1200	C
19	FEB	78	1200	C
21	FEB	78	1200	C
23	FEB	78	1200	C
25	FEB	78	1200	C
27	FEB	78	1200	C
1	MAR	78	1200	C
3	MAR	78	1200	C
5	MAR	78	1200	C
7	MAR	78	1200	C
9	MAR	78	1200	C
11	MAR	78	1200	C
13	MAR	78	1200	C
15	MAR	78	1200	C
17	MAR	78	1200	C
19	MAR	78	1200	C
21	MAR	78	1200	C
23	MAR	78	1200	C
25	MAR	78	1200	C
27	MAR	78	1200	C
29	MAR	78	1200	C
31	MAR	78	1200	C

[illegible]

26 JUL 79	1200	C
28 JUL 79	1200	RC
30 JUL 79	1200	C
1 AUG 79	1200	C
3 AUG 79	1200	C
5 AUG 79	1200	C
7 AUG 79	1200	C
9 AUG 79	1200	C
11 AUG 79	1200	RC
13 AUG 79	1200	RC
15 AUG 79	1200	C
17 AUG 79	1200	C
19 AUG 79	1200	C
21 AUG 79	1200	C
23 AUG 79	1200	RC
25 AUG 79	1200	C
27 AUG 79	1200	C
29 AUG 79	1200	C
31 AUG 79	1200	C
2 SEP 79	1200	C
4 SEP 79	1200	C
6 SEP 79	1200	C
8 SEP 79	1200	C
10 SEP 79	1200	CR
12 SEP 79	1200	C
14 SEP 79	1200	C
16 SEP 79	1200	CR
18 SEP 79	1200	C
20 SEP 79	1200	C
22 SEP 79	1200	C
24 SEP 79	1200	C
26 SEP 79	1200	C
28 SEP 79	1200	C
30 SEP 79	1200	C
2 OCT 79	1200	C
4 OCT 79	1200	C
6 OCT 79	1200	C
8 OCT 79	1200	C
10 OCT 79	1200	RC
12 OCT 79	1200	C
14 OCT 79	1200	C
16 OCT 79	1200	C
18 OCT 79	1200	C
20 OCT 79	1200	C
22 OCT 79	1200	C
24 OCT 79	1200	C
26 OCT 79	1200	C
28 OCT 79	1200	C
30 OCT 79	1200	CR
1 NOV 79	1200	CP
3 NOV 79	1200	C
5 NOV 79	1200	C
7 NOV 79	1200	C
9 NOV 79	1200	C
11 NOV 79	1200	C
13 NOV 79	1200	C
15 NOV 79	1200	C
17 NOV 79	1200	C
19 NOV 79	1200	C
21 NOV 79	1200	C

23 NOV 79	1200	RC
25 NOV 79	1200	RC
27 NOV 79	1200	RC
29 NOV 79	1200	RC
1 DEC 79	1200	RC
3 DEC 79	1200	RC
5 DEC 79	1200	RC
7 DEC 79	1200	RC
9 DEC 79	1200	RC
11 DEC 79	1200	RC
13 DEC 79	1200	RC
15 DEC 79	1200	RC
17 DEC 79	1200	RC
19 DEC 79	1200	RC
21 DEC 79	1200	RC
23 DEC 79	1200	RC
25 DEC 79	1200	C
27 DEC 79	1200	C
29 DEC 79	1200	C
31 DEC 79	1200	C

[illegible]

C
C
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C
C
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C
C
C

12 APR 88 1200 C
14 APR 88 1200 C
16 APR 88 1200 C
18 APR 88 1200 C
20 APR 88 1200 C
22 APR 88 1200 CR
24 APR 88 1200 CR
26 APR 88 1200 C
28 APR 88 1200 C
30 APR 88 1200 C
2 MAY 88 1200 C
4 MAY 88 1200 C
6 MAY 88 1200 C
8 MAY 88 1200 C
10 MAY 88 1200 C
12 MAY 88 1200 C
14 MAY 88 1200 C
16 MAY 88 1200 C
18 MAY 88 1200 C
20 MAY 88 1200 C
22 MAY 88 1200 CR
24 MAY 88 1200 CR
26 MAY 88 1200 C
28 MAY 88 1200 C
30 MAY 88 1200 C
1 JUN 88 1200 C
3 JUN 88 1200 C
5 JUN 88 1200 C
7 JUN 88 1200 C
9 JUN 88 1200 C
11 JUN 88 1200 C
13 JUN 88 1200 C
15 JUN 88 1200 CR
17 JUN 88 1200 CR
19 JUN 88 1200 C
21 JUN 88 1200 C
23 JUN 88 1200 C
25 JUN 88 1200 C
27 JUN 88 1200 C
29 JUN 88 1200 CR
1 JUL 88 1200 C
3 JUL 88 1200 C
5 JUL 88 1200 C
7 JUL 88 1200 C
9 JUL 88 1200 C
11 JUL 88 1200 C
13 JUL 88 1200 C
15 JUL 88 1200 C
17 JUL 88 1200 C
19 JUL 88 1200 C
21 JUL 88 1200 C
23 JUL 88 1200 C
25 JUL 88 1200 C
27 JUL 88 1200 C
29 JUL 88 1200 C
31 JUL 88 1200 C
2 AUG 88 1200 C
4 AUG 88 1200 C
6 AUG 88 1200 C
8 AUG 88 1200 C

10 AUG 88 1200 C
12 AUG 88 1200 C
14 AUG 88 1200 CR
16 AUG 88 1200 C
18 AUG 88 1200 C
20 AUG 88 1200 C
22 AUG 88 1200 C
24 AUG 88 1200 C
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1 SEP 88 1200 C
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15 SEP 88 1200 CR
17 SEP 88 1200 C
19 SEP 88 1200 C
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23 SEP 88 1200 CR
25 SEP 88 1200 C
27 SEP 88 1200 C
29 SEP 88 1200 C
1 OCT 88 1200 C
3 OCT 88 1200 C
5 OCT 88 1200 C
7 OCT 88 1200 C
9 OCT 88 1200 CR
11 OCT 88 1200 CR
13 OCT 88 1200 CR
15 OCT 88 1200 CR
17 OCT 88 1200 CR
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25 OCT 88 1200 C
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29 OCT 88 1200 CR
31 OCT 88 1200 CR
2 NOV 88 1200 CR
4 NOV 88 1200 C
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12 NOV 88 1200 CR
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26 NOV 88 1200 C
28 NOV 88 1200 CR
30 NOV 88 1200 C
2 DEC 88 1200 C
4 DEC 88 1200 C
6 DEC 88 1200 C

9	APR	69	1200	
11	APR	69	1200	
13	APR	69	1200	
15	APR	69	1200	
17	APR	69	1200	
19	APR	69	1200	
21	APR	69	1200	
23	APR	69	1200	
25	APR	69	1200	
27	APR	69	1200	
29	APR	69	1200	
1	MAY	69	1200	
3	MAY	69	1200	
5	MAY	69	1200	
7	MAY	69	1200	
9	MAY	69	1200	
11	MAY	69	1200	
13	MAY	69	1200	
15	MAY	69	1200	
17	MAY	69	1200	
19	MAY	69	1200	
21	MAY	69	1200	
23	MAY	69	1200	
25	MAY	69	1200	
27	MAY	69	1200	
1	JUN	69	1200	
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25	JUN	69	1200	
27	JUN	69	1200	
29	JUN	69	1200	
1	JUL	69	1200	
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19	JUL	69	1200	
21	JUL	69	1200	
23	JUL	69	1200	
25	JUL	69	1200	
27	JUL	69	1200	
29	JUL	69	1200	
31	AUG	69	1200	
1	AUG	69	1200	

5 AUG 69 1200 CR
7 AUG 69 1200 CR
9 AUG 69 1200 RC
11 AUG 69 1200 R C
13 AUG 69 1200 RC
15 AUG 69 1200 RC
17 AUG 69 1200 C
19 AUG 69 1200 C
21 AUG 69 1200 C
23 AUG 69 1200 C
25 AUG 69 1200 C
27 AUG 69 1200 C
29 AUG 69 1200 RC
31 AUG 69 1200 C
2 SEP 69 1200 C
4 SEP 69 1200 C
6 SEP 69 1200 C
8 SEP 69 1200 C
10 SEP 69 1200 CR
12 SEP 69 1200 C
14 SEP 69 1200 C
16 SEP 69 1200 C
18 SEP 69 1200 C
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28 SEP 69 1200 C
30 SEP 69 1200 C
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10 OCT 69 1200 C
12 OCT 69 1200 RC
14 OCT 69 1200 RC
16 OCT 69 1200 RC
18 OCT 69 1200 C
20 OCT 69 1200 RC
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28 OCT 69 1200 RC
30 OCT 69 1200 C
1 NOV 69 1200 CR
3 NOV 69 1200 C
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7 NOV 69 1200 C
9 NOV 69 1200 RC
11 NOV 69 1200 RC
13 NOV 69 1200 RC
15 NOV 69 1200 RC
17 NOV 69 1200 RC
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27 NOV 69 1200 RC
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1 DEC 69 1200 RC

3 DEC 69 1200 C
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19 DEC 69 1200 C
21 DEC 69 1200 C
23 DEC 69 1200 C
25 DEC 69 1200 RC
27 DEC 69 1200 RC
29 DEC 69 1200 RC
31 DEC 69 1200 C
2 JAN 70 1200 RC
4 JAN 70 1200 RC
6 JAN 70 1200 RC
8 JAN 70 1200 RC
10 JAN 70 1200 RC
12 JAN 70 1200 RC
14 JAN 70 1200 C
16 JAN 70 1200 C
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27 FEB 70 1200 C
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A scatter plot showing the relationship between the number of species (S) on the x-axis and the number of individuals (N) on the y-axis. The x-axis ranges from 0 to 100, and the y-axis ranges from 0 to 100. Data points are labeled with 'C' for common species and 'R' for rare species. A diagonal line represents the 1:1 relationship. The plot shows a positive correlation between S and N, with a dense cluster of points at low values and a few points at higher values.

28	JUL	71	1200
28	JUL	71	1200
30	JUL	71	1200
1	AUG	71	1200
3	AUG	71	1200
5	AUG	71	1200
7	AUG	71	1200
9	AUG	71	1200
11	AUG	71	1200
13	AUG	71	1200
15	AUG	71	1200
17	AUG	71	1200
19	AUG	71	1200
21	AUG	71	1200
23	AUG	71	1200
25	AUG	71	1200
27	AUG	71	1200
29	AUG	71	1200
31	AUG	71	1200
2	SEP	71	1200
4	SEP	71	1200
6	SEP	71	1200
8	SEP	71	1200
10	SEP	71	1200
12	SEP	71	1200
14	SEP	71	1200
16	SEP	71	1200
18	SEP	71	1200
20	SEP	71	1200
22	SEP	71	1200
24	SEP	71	1200
26	SEP	71	1200
28	SEP	71	1200
30	SEP	71	1200
2	OCT	71	1200
4	OCT	71	1200
6	OCT	71	1200
8	OCT	71	1200
10	OCT	71	1200
12	OCT	71	1200
14	OCT	71	1200
16	OCT	71	1200
18	OCT	71	1200
20	OCT	71	1200
22	OCT	71	1200
24	OCT	71	1200
26	OCT	71	1200
28	OCT	71	1200
30	OCT	71	1200
31	NOV	71	1200
5	NOV	71	1200
7	NOV	71	1200
9	NOV	71	1200
11	NOV	71	1200
13	NOV	71	1200
15	NOV	71	1200
17	NOV	71	1200
19	NOV	71	1200
21	NOV	71	1200

R R C
R R C
R R C
R R C
R R C
R R C
C C C
C C C
C C C
R R C
R R C
C C C
C C C
C C C
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PLOT CHARACTER		STATION NAME		STATION NUMBER CONTROL	
C-CALC. BEAVER RIVER FLOW AT COLD LAKE				728 0 0	
R-BEAVER R AT COLD LAKE 0840005				82 8 0	
5500 8000 7500 8000 10500				12000. 13500 15000	
*-DUMMY RES (DIFF) AT BEAVER RIVER AT COLD LA				726 2 E	
				3 4 5	
FLOW CFS	0	1500	3000		
ELEV FT.	-5	-4	-3	-2	-1
1 JAN 72 1200	RC				
3 JAN 72 1200	RC				
5 JAN 72 1200	RC				
7 JAN 72 1200	RC				
9 JAN 72 1200	RC				
11 JAN 72 1200	RC				
13 JAN 72 1200	RC				
15 JAN 72 1200	RC				
17 JAN 72 1200	RC				
19 JAN 72 1200	RC				
21 JAN 72 1200	RC				
23 JAN 72 1200	RC				
25 JAN 72 1200	RC				
27 JAN 72 1200	RC				
29 JAN 72 1200	C				
31 JAN 72 1200	C				
2 FEB 72 1200	C				
4 FEB 72 1200	C				
6 FEB 72 1200	C				
8 FEB 72 1200	C				
10 FEB 72 1200	C				
12 FEB 72 1200	C				
14 FEB 72 1200	C				
16 FEB 72 1200	C				
18 FEB 72 1200	C				
20 FEB 72 1200	C				
22 FEB 72 1200	C				
24 FEB 72 1200	C				
26 FEB 72 1200	C				
28 FEB 72 1200	C				
1 MAR 72 1200	C				
3 MAR 72 1200	C				
5 MAR 72 1200	C				
7 MAR 72 1200	C				
9 MAR 72 1200	C				
11 MAR 72 1200	C				
13 MAR 72 1200	C				
15 MAR 72 1200	C				
17 MAR 72 1200	C				
19 MAR 72 1200	C				
21 MAR 72 1200	C				
23 MAR 72 1200	C				
25 MAR 72 1200	C				
27 MAR 72 1200	C				
29 MAR 72 1200	C				
31 MAR 72 1200	C				
2 APR 72 1200	C				
4 APR 72 1200	C				
6 APR 72 1200	CR				
8 APR 72 1200	CR				
10 APR 72 1200	CR				

12	APR	75	1200
14	APR	75	1200
16	APR	75	1200
20	APR	75	1200
22	APR	75	1200
24	APR	75	1200
26	APR	75	1200
28	APR	75	1200
30	APR	75	1200
2	MAY	75	1200
4	MAY	75	1200
6	MAY	75	1200
8	MAY	75	1200
10	MAY	75	1200
12	MAY	75	1200
14	MAY	75	1200
16	MAY	75	1200
20	MAY	75	1200
22	MAY	75	1200
24	MAY	75	1200
26	MAY	75	1200
28	MAY	75	1200
30	MAY	75	1200
1	JUN	75	1200
3	JUN	75	1200
5	JUN	75	1200
7	JUN	75	1200
9	JUN	75	1200
11	JUN	75	1200
13	JUN	75	1200
16	JUN	75	1200
17	JUN	75	1200
18	JUN	75	1200
21	JUN	75	1200
23	JUN	75	1200
25	JUN	75	1200
27	JUN	75	1200
29	JUN	75	1200
1	JUL	75	1200
3	JUL	75	1200
5	JUL	75	1200
9	JUL	75	1200
11	JUL	75	1200
12	JUL	75	1200
16	JUL	75	1200
17	JUL	75	1200
21	JUL	75	1200
23	JUL	75	1200
25	JUL	75	1200
27	JUL	75	1200
29	JUL	75	1200
31	JUL	75	1200
3	AUG	75	1200
5	AUG	75	1200
8	AUG	75	1200
8	AUG	75	1200

1. The first step is to identify the problem or question that needs to be addressed. This involves understanding the context and the specific requirements of the task.

2. Next, it is important to gather relevant information and resources. This can include researching existing solutions, consulting with experts, and collecting data.

3. Once the information is gathered, the next step is to analyze it. This involves identifying the key factors and determining the most effective approach to solve the problem.

4. After analysis, a plan should be developed. This plan should outline the steps to be taken, the resources needed, and the timeline for completion.

5. The final step is to implement the plan. This involves executing the steps outlined in the plan and monitoring progress to ensure that the problem is solved effectively.

8 DEC 78 1200 AC
10 DEC 78 1200 AC
12 DEC 78 1200 AC
14 DEC 78 1200 AC
16 DEC 78 1200 AC
18 DEC 78 1200 AC
20 DEC 78 1200 AC
22 DEC 78 1200 AC
24 DEC 78 1200 AC
26 DEC 78 1200 AC
28 DEC 78 1200 C
30 DEC 78 1200 C
1 JAN 77 1200 C
3 JAN 77 1200 C
5 JAN 77 1200 C
7 JAN 77 1200 C
9 JAN 77 1200 C
11 JAN 77 1200 C
13 JAN 77 1200 C
15 JAN 77 1200 C
17 JAN 77 1200 C
19 JAN 77 1200 C
21 JAN 77 1200 C
23 JAN 77 1200 C
25 JAN 77 1200 C
27 JAN 77 1200 C
29 JAN 77 1200 C
31 JAN 77 1200 E
2 FEB 77 1200 E
4 FEB 77 1200 E
6 FEB 77 1200 C
8 FEB 77 1200 C
10 FEB 77 1200 C
12 FEB 77 1200 C
14 FEB 77 1200 C
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28 FEB 77 1200 C
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1 APR 77 1200 C
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9 APR 77 1200 AC
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29 APR 77 1200 AC
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30 JUL 77 1200 AC
1 AUG 77 1200 AC
3 AUG 77 1200 AC

5	AUG	77	1200	
7	AUG	77	1200	CR
9	AUG	77	1200	CR
11	AUG	77	1200	CR
13	AUG	77	1200	CR
15	AUG	77	1200	CR
17	AUG	77	1200	CR
19	AUG	77	1200	CR
21	AUG	77	1200	CR
23	AUG	77	1200	CR
25	AUG	77	1200	CR
27	AUG	77	1200	CR
29	AUG	77	1200	CR
31	AUG	77	1200	CR
2	SEP	77	1200	CR
4	SEP	77	1200	CR
6	SEP	77	1200	CR
8	SEP	77	1200	CR
10	SEP	77	1200	CR
12	SEP	77	1200	CR
14	SEP	77	1200	CR
16	SEP	77	1200	CR
18	SEP	77	1200	CR
20	SEP	77	1200	CR
22	SEP	77	1200	CR
24	SEP	77	1200	CR
26	SEP	77	1200	CR
28	SEP	77	1200	CR
30	SEP	77	1200	CR
2	OCT	77	1200	CR
4	OCT	77	1200	CR
6	OCT	77	1200	CR
8	OCT	77	1200	CR
10	OCT	77	1200	CR
12	OCT	77	1200	CR
14	OCT	77	1200	CR
16	OCT	77	1200	CR
18	OCT	77	1200	CR
20	OCT	77	1200	CR
22	OCT	77	1200	CR
24	OCT	77	1200	CR
26	OCT	77	1200	CR
28	OCT	77	1200	CR
30	OCT	77	1200	CR
1	NOV	77	1200	CR
3	NOV	77	1200	CR
5	NOV	77	1200	CR
7	NOV	77	1200	CR
9	NOV	77	1200	CR
11	NOV	77	1200	CR
13	NOV	77	1200	CR
15	NOV	77	1200	CR
17	NOV	77	1200	CR
19	NOV	77	1200	CR
21	NOV	77	1200	CR
23	NOV	77	1200	CR
25	NOV	77	1200	CR
27	NOV	77	1200	CR
29	NOV	77	1200	CR
1	DEC	77	1200	CR

3	DEC	77	1200	CR
5	DEC	77	1200	CR
7	DEC	77	1200	CR
9	DEC	77	1200	CR
11	DEC	77	1200	CR
13	DEC	77	1200	CR
15	DEC	77	1200	CR
17	DEC	77	1200	CR
19	DEC	77	1200	CR
21	DEC	77	1200	CR
23	DEC	77	1200	CR
25	DEC	77	1200	CR
27	DEC	77	1200	CR
29	DEC	77	1200	CR
31	DEC	77	1200	CR
2	JAN	78	1200	CR
4	JAN	78	1200	CR
6	JAN	78	1200	CR
8	JAN	78	1200	CR
10	JAN	78	1200	CR
12	JAN	78	1200	CR
14	JAN	78	1200	CR
16	JAN	78	1200	CR
18	JAN	78	1200	CR
20	JAN	78	1200	CR
22	JAN	78	1200	CR
24	JAN	78	1200	CR
26	JAN	78	1200	CR
28	JAN	78	1200	CR
30	JAN	78	1200	CR
1	FEB	78	1200	CR
3	FEB	78	1200	CR
5	FEB	78	1200	CR
7	FEB	78	1200	CR
9	FEB	78	1200	CR
11	FEB	78	1200	CR
13	FEB	78	1200	CR
15	FEB	78	1200	CR
17	FEB	78	1200	CR
19	FEB	78	1200	CR
21	FEB	78	1200	CR
23	FEB	78	1200	CR
25	FEB	78	1200	CR
27	FEB	78	1200	CR
1	MAR	78	1200	CR
3	MAR	78	1200	CR
5	MAR	78	1200	CR
7	MAR	78	1200	CR
9	MAR	78	1200	CR
11	MAR	78	1200	CR
13	MAR	78	1200	CR
15	MAR	78	1200	CR
17	MAR	78	1200	CR
19	MAR	78	1200	CR
21	MAR	78	1200	CR
23	MAR	78	1200	CR
25	MAR	78	1200	CR
27	MAR	78	1200	CR
29	MAR	78	1200	CR
31	MAR	78	1200	CR

1	APR	78	1200
6	APR	78	1200
8	APR	78	1200
9	APR	78	1200
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28	APR	78	1200
30	APR	78	1200
2	MAY	78	1200
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31 JUL 78	1200	C
2 AUG 78	1200	C
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30 AUG 78	1200	C
1 SEP 78	1200	C
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5 SEP 78	1200	C
7 SEP 78	1200	C
9 SEP 78	1200	C
11 SEP 78	1200	CR
13 SEP 78	1200	CR
15 SEP 78	1200	CR
17 SEP 78	1200	
19 SEP 78	1200	CR
21 SEP 78	1200	
23 SEP 78	1200	C
25 SEP 78	1200	CR
27 SEP 78	1200	CR
29 SEP 78	1200	CR
1 OCT 78	1200	C
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7 OCT 78	1200	CR
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27 OCT 78	1200	C
29 OCT 78	1200	CR
31 OCT 78	1200	C
2 NOV 78	1200	C
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10 NOV 78	1200	C
12 NOV 78	1200	CR
14 NOV 78	1200	C
16 NOV 78	1200	C
18 NOV 78	1200	CR
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22 NOV 78	1200	CR
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APPENDIX B

GROUND WATER RESOURCES

A Report Prepared By
MLM GROUND-WATER ENGINEERING

MLM Ground-Water Engineering

#209 - 200 Carnegie Drive, St. Albert, Alberta, Canada T8N 3K1

Phone: (403) 458-8383

May 30, 1983

Alberta Environment
Planning Division
9820 - 106th Street
EDMONTON, AB
T5K 2J6

Attention: Ron Hicks, P.Eng.

Dear Mr. Hicks:

Re: Ground-Water Resources
Cold Lake - Beaver River Basin

We have the pleasure of submitting our final report entitled, "Ground-Water Resources, Cold Lake - Beaver River Basin". The report presents a summary of the information regarding the geology and hydrogeology of the major aquifers in the area. Estimates of ground-water availability are given along with the adequacy of the supplies for the various users.

We trust this report is satisfactory for your needs at this time.

Yours truly,

MLM GROUND-WATER ENGINEERING


M. G. Mateyk, P. Geol.
Project Manager

11c

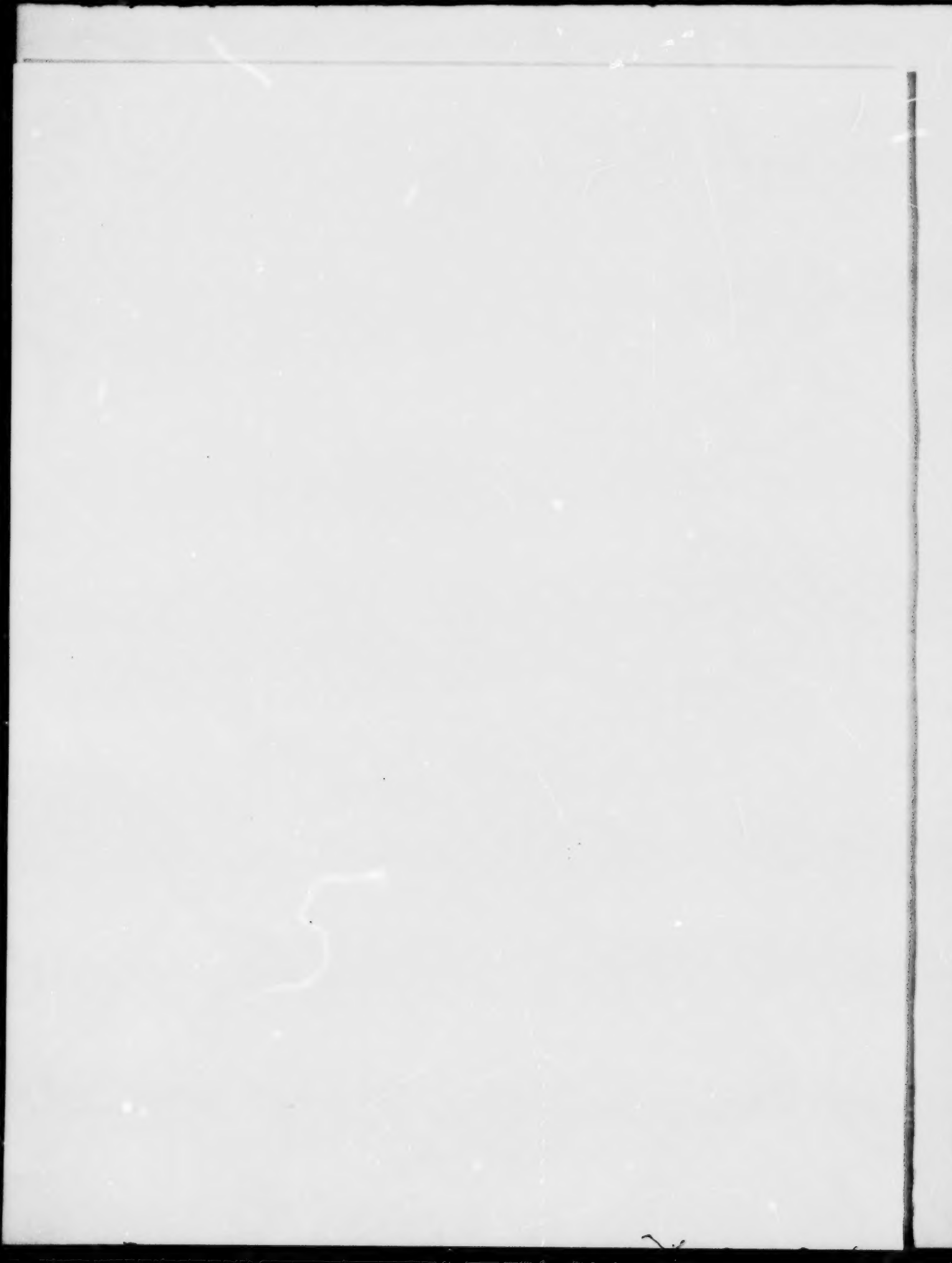


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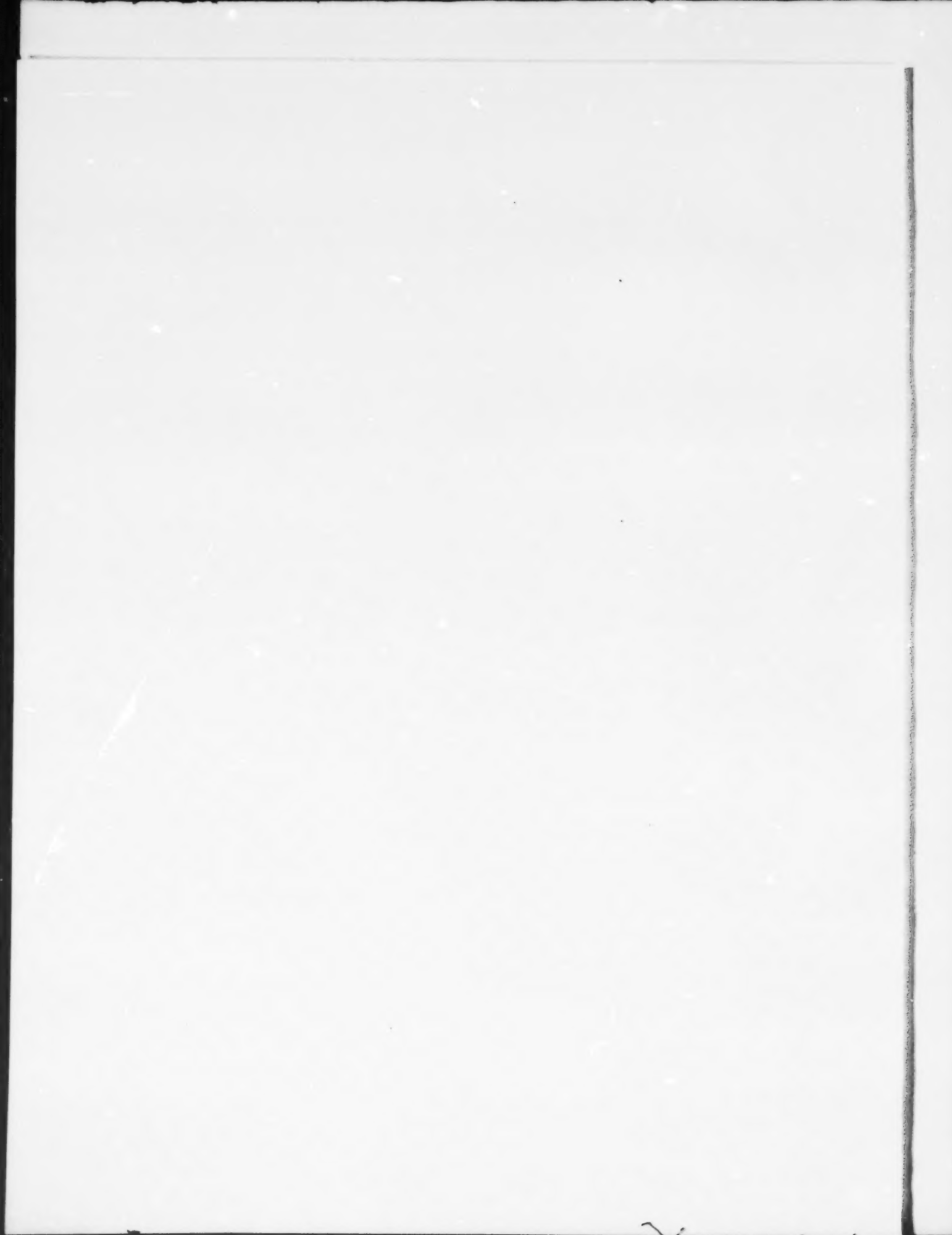
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APPENDIX A

Water Quality Analyses



1.

1.0 INTRODUCTION

1.1 Purpose of Study

Water use is becoming a critical issue within the Cold Lake - Beaver River Basin as a result of the proposed development of the Cold Lake oil sands. Combining this with the associated population growth and increasing recreational land use, sometimes conflicting demands may be placed on the surface and ground-water resources of the basin. In order to facilitate orderly management of the water resources, Alberta Environment is developing a long-term framework plan for the Cold Lake - Beaver River Basin.

In order to address the various issues of the management plan, a multi-component study was formulated by Alberta Environment. This report deals with the ground-water resources part of the water supply component. The terms of reference, under which the study was conducted, stipulated that MLM should:

- Review all available ground-water data, including any preliminary results of Alberta Environment's 1981 test drilling and observation well program.

- Prepare an overview of ground water as a water supply source in the study area and submit this overview in a letter report.

- Following the completion of Alberta Environment's 1981 ground-water program, assess the locations and rate of regional ground-water recharge and discharge, the direction and rate of regional ground-water movement, and the potential location, quality and quantity of ground-water.

2.

- Review the various water withdrawal demand scenarios prepared for time horizons 1985, 2000 and 2015.

- Determine the existing and potential significance of ground water as a water supply source on a local and regional scale and identify any future data collection requirements where supply is significant.

- Devise a procedure and account for ground-water/surface water interaction where data is adequate and the interaction is critical to the surface water supply balance.

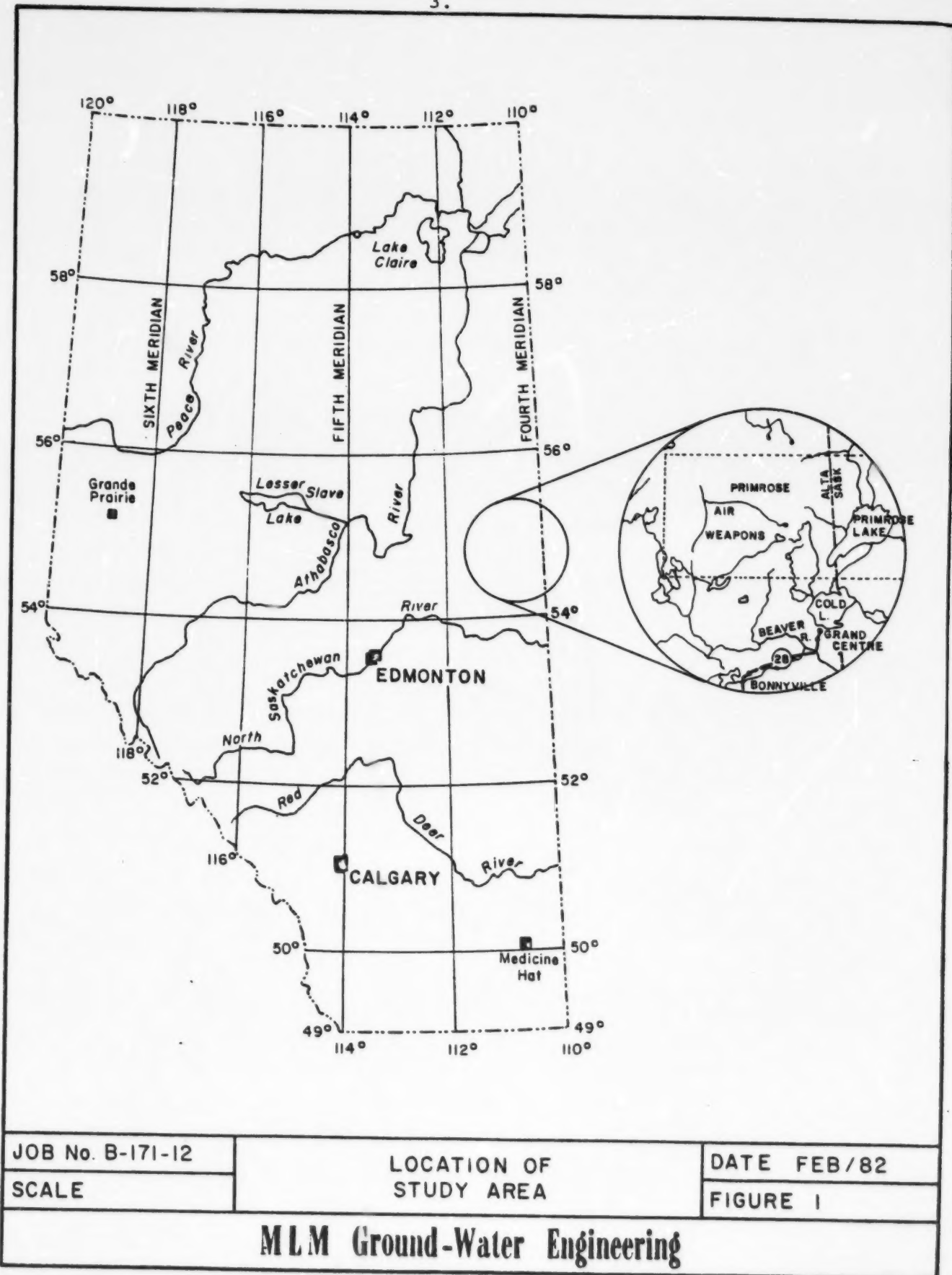
1.2 Location

The Cold Lake - Beaver River Basin is located in East Central Alberta (Figure 1). For the purpose of this study, ground-water resources were studied for the portions of the study area within Alberta.

1.3 Data Base

Several ground-water investigations have been previously conducted within the study area. The Alberta Research Council has completed a regional overview of the hydrogeology of the area as part of their hydrogeological reconnaissance map series. These maps display ground-water probabilities (expected average well yields), chemistry, and certain aspects of the flow system for map areas delineated by the NTS. The three reports which each cover a portion of the study area are:

- Hydrogeology of the Tawatinaw Area, Alberta (NTS 83I)
- Hydrogeology of the Waterways - Winefred Lake Area, Alberta (NTS 74D and 73M)
- Hydrogeology of the Sand River Area, Alberta (NTS 73L)



A number of detailed site specific ground-water investigations have been conducted by various consulting companies for heavy oil pilot plants and municipal use. These studies provided data on aquifer hydraulic parameters. Alberta Environment has also conducted a number of site specific investigations, mainly as a part of the Northern Water Supply Program. These reports contain data on hydraulic parameters and ground-water quality.

Alberta Environment installed two observation wells in the vicinity of Bonnyville (1-15-62-5-4) in 1977. Two additional wells were installed near Bourque Lake (4-26-65-4-4) in 1980. The hydrographs for these observation wells were provided by the Ground-Water Branch.

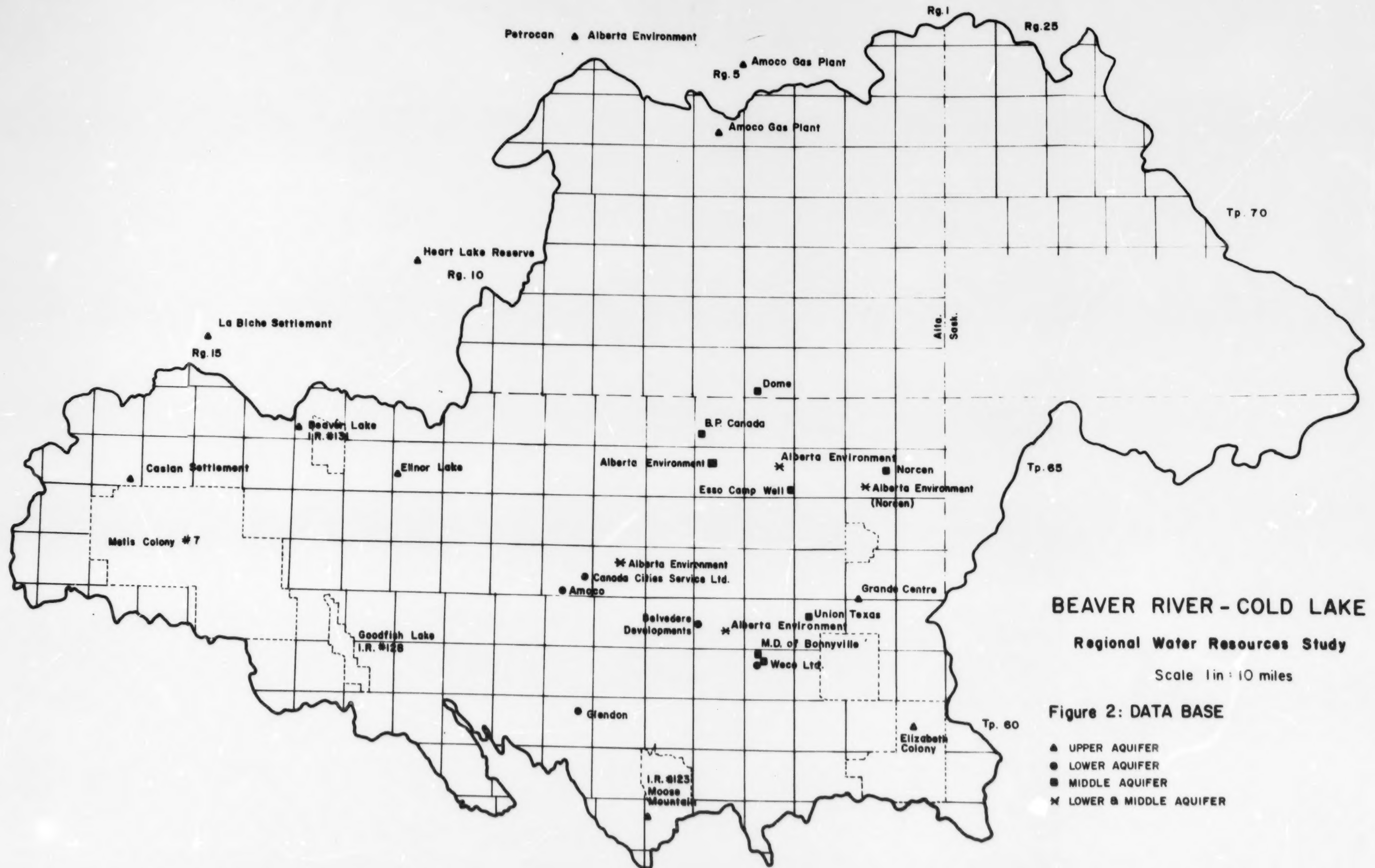
Recently, the Ground-Water Branch, under the direction of Chris Gold, completed an inventory and compilation of geologic data in the study area. The objective of this study is to delineate the bedrock channels in the region and to define the various surficial aquifers associated with the channels. Currently, the program is at a stage where test drilling is being conducted to fill in various gaps in the data base.

The locations of the most important site specific studies are presented in Figure 2, while the type of information derived from the reports is summarized in Table 1.

1.4 Acknowledgements

Special thanks for assistance in preparing this report is due to a number of individuals. Mr. Ron Hicks of Planning Division, ensured that data from other components of the study needed for this report were forthcoming. Mr. Vern Carlson, Branch Head of the Ground-water





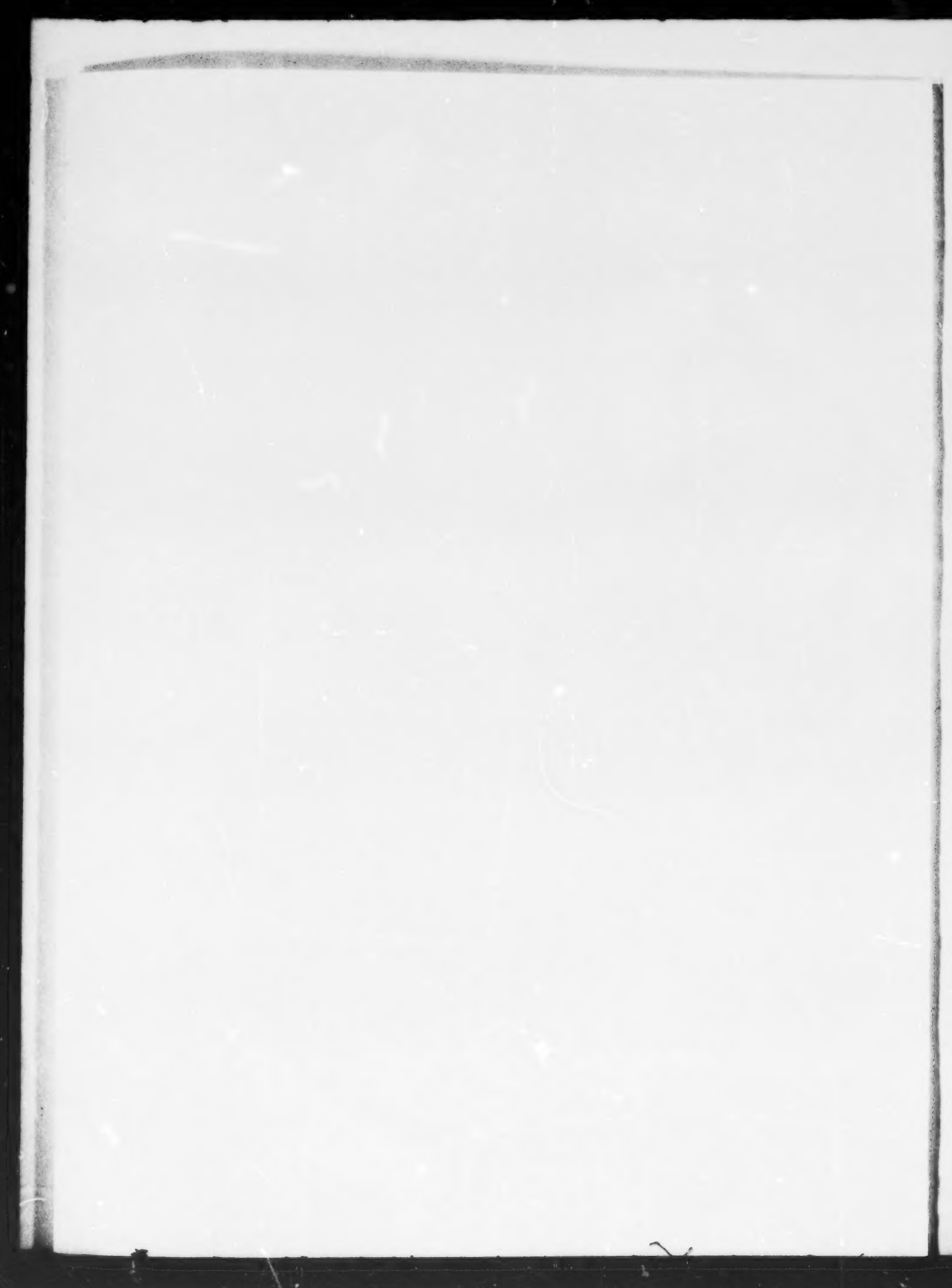


TABLE 1

SUMMARY OF SITE SPECIFIC
GROUND-WATER INVESTIGATIONS

<u>Well Owner</u>	<u>Location</u>	<u>Potential Water Use</u>	<u>Work Conducted by</u>	<u>Type of Information</u>
Amoco	7-4-63-8-4	Pilot Plant	Consultant	Test drilling, aquifer hydraulic properties, water quality
Canada Cities Ltd.	6-13-63-8-4	Pilot Plant	Consultant	"
M.D. of Bonnyville	SE32-61-4-4	Municipal	Consultant	"
Suncor (Weco)	SW38-61-4-4	Pilot Plant	"	"
Belvedere Developments	SW19-62-5-4	Industrial	"	"
Village of Glendon	SE26-60-8-4	Municipal	"	"
Union Texas	13&14-20-62-3-4	Pilot Plant	"	"
Imperial Oil	NEL-65-4-4	Construction Camp	"	"
B.P.	9-7-66-5-4	Pilot Plant	"	"
Norcen	12-24-65-2-4	Pilot Plant	"	"
Alberta Environment (Norcen)	SW9-65-2-4	Observation Well	Alberta Environment	Aquifer hydraulic properties
Dome	NE5-67-4-4	Pilot Plant	Consultant	Test drilling, aquifer hydraulic properties, water quality

Table 1 continued...

<u>Well Owner</u>	<u>Location</u>	<u>Potential Water Use</u>	<u>Work Conducted by</u>	<u>Type of Information</u>
Petro Canada	10-19-23-7-4	Pilot Plant	Consultant	Aquifer hydraulic properties
Alberta Environment	10-19-23-7-4	Ground-Water Investigation	Alberta Environment	Test drilling, aquifer hydraulic properties, water quality
Alberta Environment	1-15-62-5-4	Observation Well	"	Test Drilling and observation well installation
Alberta Environment	4-26-65-4-4	Observation Well	"	Aquifer hydraulic properties
Alberta Environment	SW28-65-5-4	Observation Well	"	"

Rights Branch, allowed access to the various files within the study area. Mr. Chris Gold, of the Groundwater Branch, generously provided his detailed information on the bedrock topography and surficial geology.

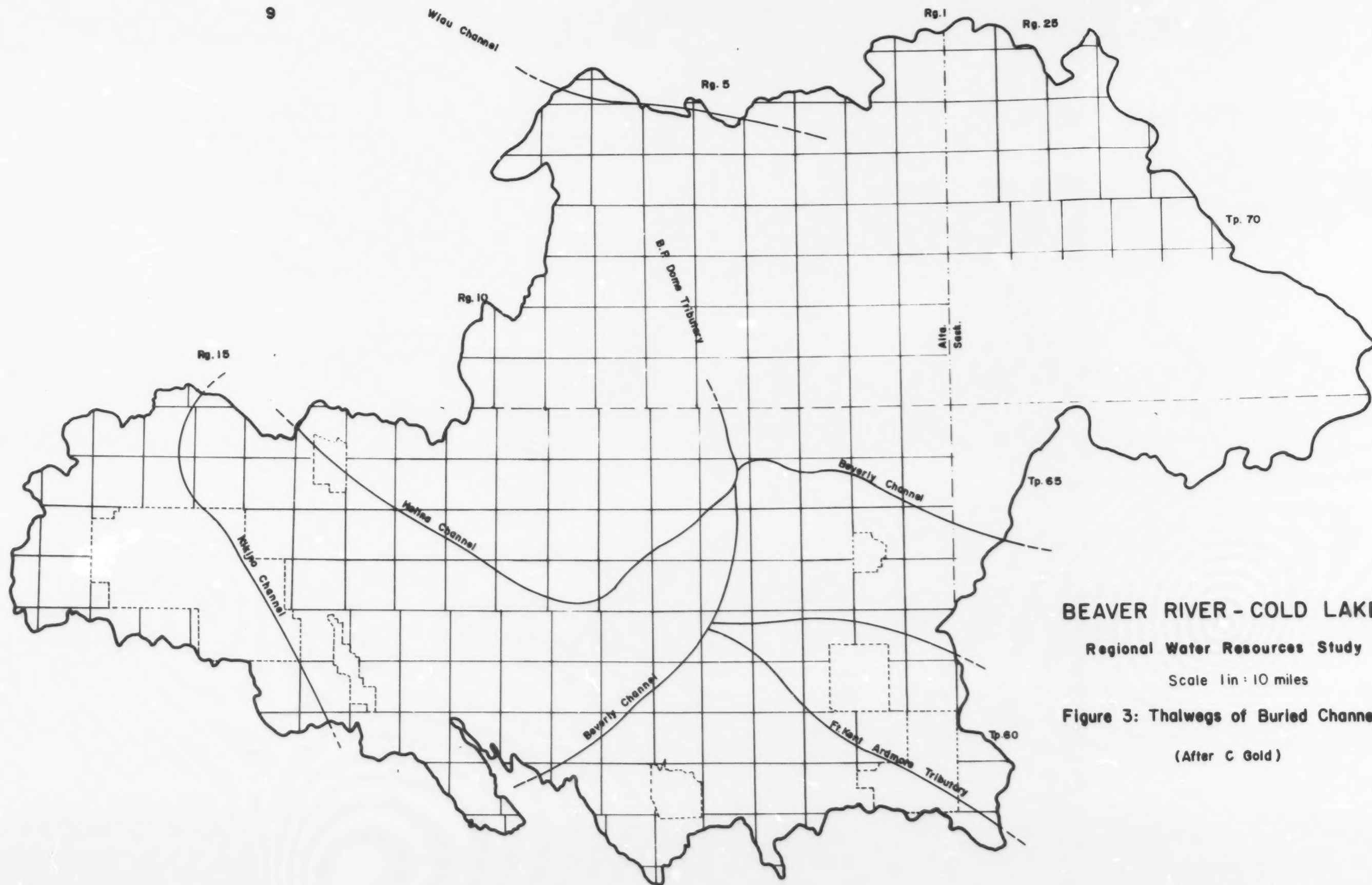
2.0 AQUIFER GEOLOGY

2.1 General Statement

The geology of the aquifers in the Cold Lake - Beaver River area can be best understood in light of the area's glacial and late preglacial history.

During the late Quaternary, the landscape was dissected by a system of rivers which drained from the Cordillera toward the east. Eventually, a mature land surface, characterized by streams running in broad, flat bottomed valleys evolved, the thalwegs of which are shown in Figure 3. These streams ran at or near grade, reworking sands and gravels in the valley bottoms, and bringing about a high degree of sorting in the sediments. The granular materials found in these bedrock valleys, lying directly upon the bedrock, were transported from the Cordillera, and therefore contain a high percentage of siliceous material (chert, quartz, quartzite).

The glaciers which moved over the country during the Pleistocene Epoch brought about a drastic change in the land surface. The preglacial bedrock valleys were filled



BEAVER RIVER - COLD LAKE

Regional Water Resources Study

Scale 1 in : 10 miles

Figure 3: Thalwegs of Buried Channels

(After C Gold)



in with till, especially where the direction of glacial movement was at a large angle to the trend of the valley. During glacial melting, the till was released from its ice matrix and left as an unsorted mass. The valleys served as drainage channels for the meltwaters, which reworked the tills, carrying away the silt and clay fraction, and leaving behind the sand, gravel, and larger fractions. Because of this, the accumulations of sorted granular materials derived from reworked till are thickest in the valleys, and grow thinner toward the valley flanks. Because there were a number of glacial advances and recessions during the Pleistocene, these layers are in places, quite numerous.

2.2 Upper Aquifer

In this report, the name "upper aquifer" is applied to a series of discontinuous, poorly-sorted, intertill sands and gravels found generally within the upper 45 metres or so of overburden. These sand and gravel layers are of glacial origin. They are lensitic, and the thickness of a single layer is seldom more than 3 metres, although their aggregate thickness may be as much as 15 metres. The mineralogy of the upper aquifer reflects the provenance of the tills from which the sands and gravels were derived. Granitic and metamorphic rock fragments from the Precambrian Shield are preponderant. Carbonate minerals are also present, derived from the Paleozoic sedimentary rocks to the northeast.

The upper aquifer has been exploited extensively throughout the study area and northeastern Alberta in general; for domestic and agricultural water supply. It has been particularly useful where buried valleys do not exist, and no ground water is available from the bed-rock. In places, these intertill layers are the only

source of ground water. Because of this, and despite a generally low observed individual well yield, the regional importance of the upper aquifer cannot be denied. However, in the context of this report, we are concerned with high-yield industrial and municipal ground-water supplies. For this reason, the upper aquifer is not discussed in as much detail as are the two deeper, major aquifers in the region.

2.3 Middle Aquifer

The uppermost of the two major aquifers occupying the buried preglacial channels of the study area, is an intertill unit termed the "Middle Aquifer" in this report. An isopach map of the aquifer was prepared from the cross-sections of the Alberta Environment Study, and is shown in Figure 4 (pocket).

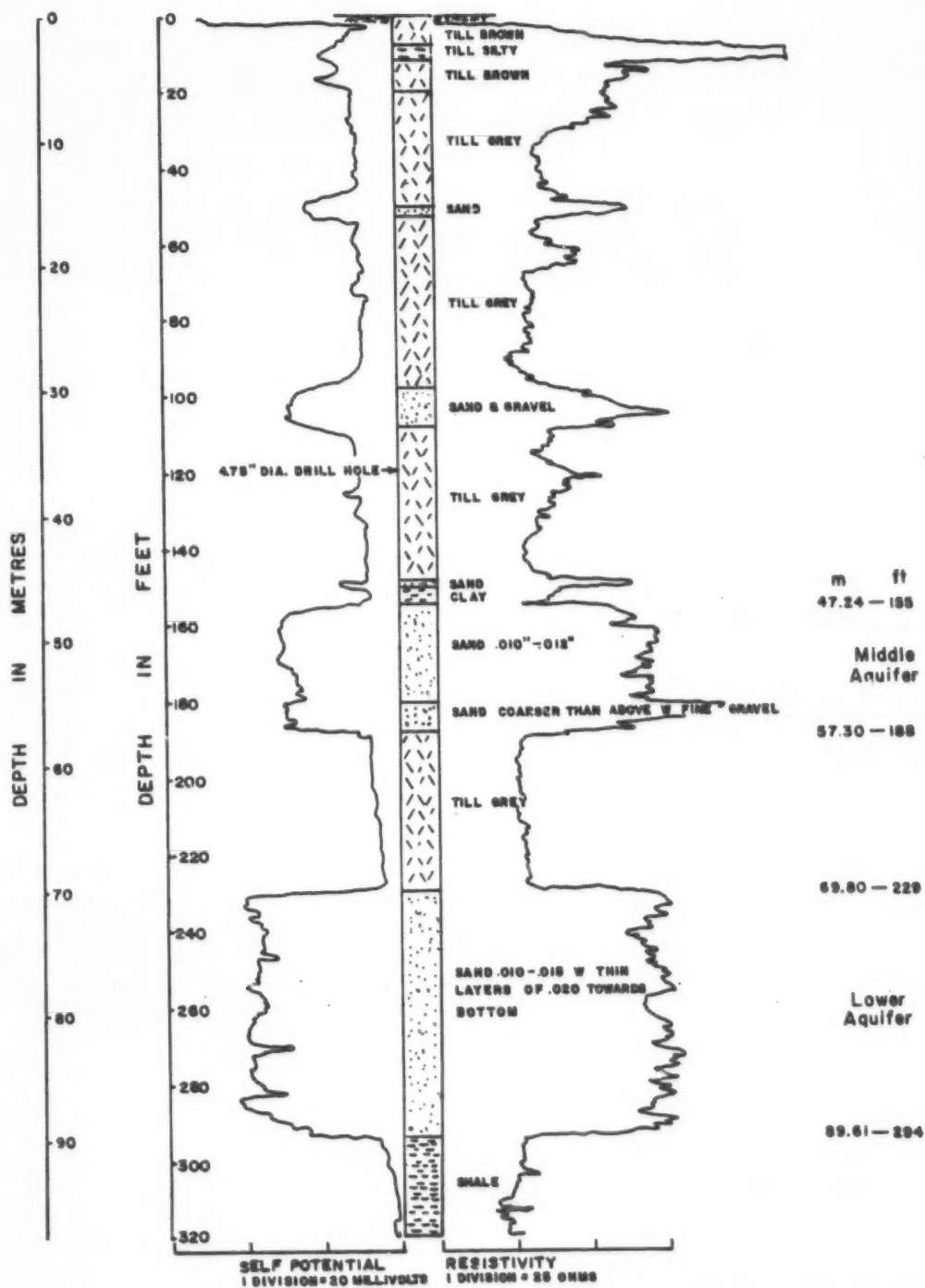
Beverly Channel

In a water supply investigation for the Village of Glendon (NW $\frac{1}{4}$ -Sec4-Tp61-Rge8-W4M), Baydack and Render (1981) drilled a total of seven test holes into the Beverly Channel.

The best definition of the middle aquifer in that area is seen in SE $\frac{1}{4}$ -Sec26-Tp6-Rge8-W4M. A typical stratigraphic section in this area is shown in Figure 5.

In the Beverly Channel here, the Middle Aquifer reaches a maximum thickness of up to 12.2 metres. The sands consist of white silica and black chert fragments; a mineralogy which indicates a Cordilleran source. In the Glendon area, the Middle Aquifer extends approximately 3 km outward from the centre of the Beverly Channel, and consequently, is not encountered beneath the Village of Glendon (NW $\frac{1}{4}$ -Sec.4-Tp.61-Rge.8-W4M).

GEOLOGIC DESCRIPTION



JOB NO B-171-12

TYPICAL STRATIGRAPHIC SECTION

DATE MARCH/82

SCALE

GLENDON (SE 26-60-8-4)

FIGURE 5

MLM Ground-Water Engineering

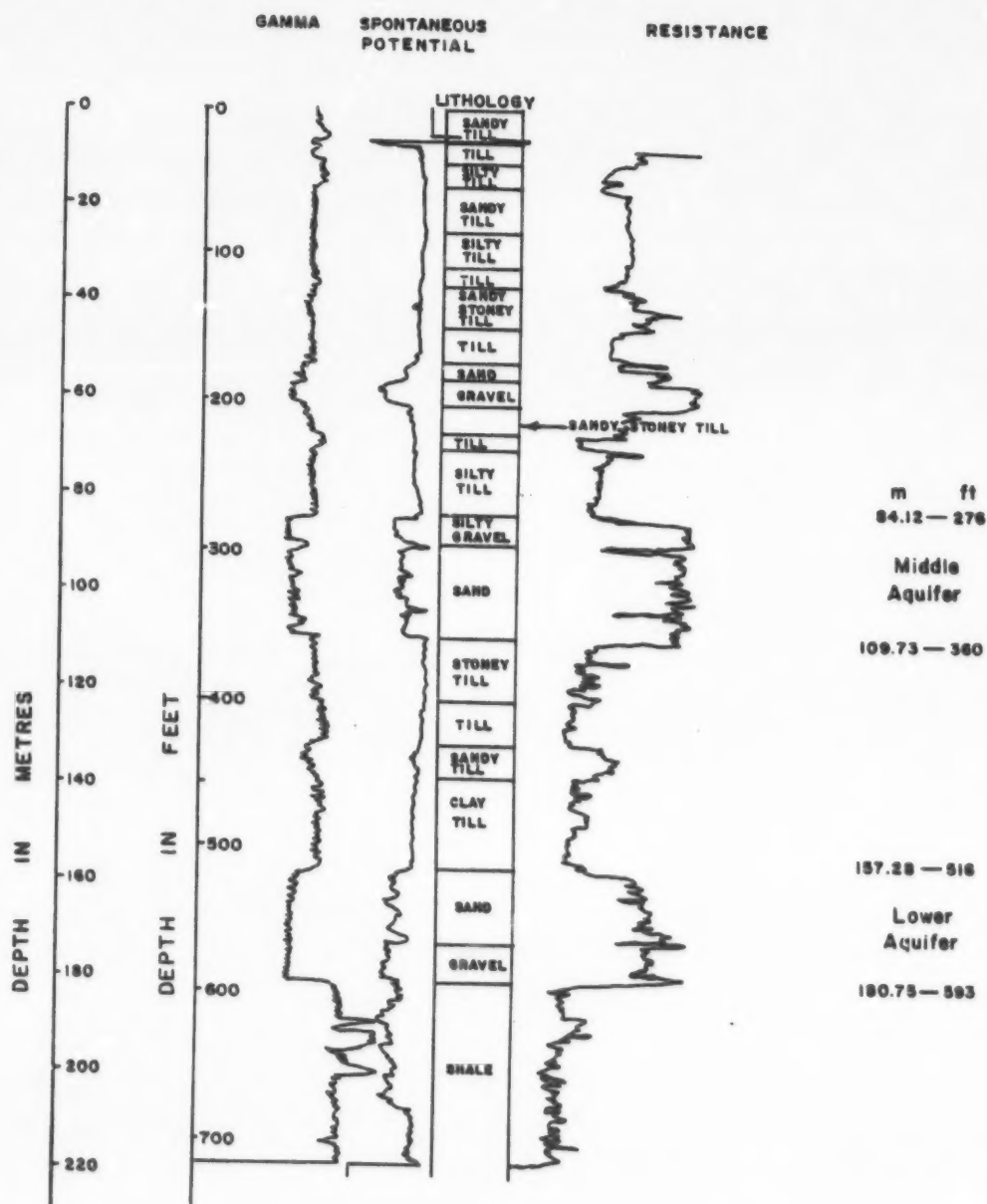
An observation well, drilled in the Bourque Lake area (4-26-65-5-4) by Alberta Environment (1978) encountered the Middle Aquifer over the depth interval 84 to 109 metres (510 to 536 m a.m.s.l.). The aquifer consists of interlayered sandy silt, overlying fine to coarse-grained sand. The log of this well (Figure 6) shows a very good definition of the Middle Aquifer.

Further east, on LSD12-Sec24-Tp65-Rge2-W4M, the Middle Aquifer is very well represented as a 13.7 metre thick layer of brown sand and gravel of an unspecified mineralogic content, in a well drilled and tested for Norcen Ltd. (Hydrogeological Consultants Ltd., 1976). A second Norcen well, located on SW9-Tp65-Rge2-W4M, and tested by Alberta Environment, encountered the Middle Aquifer over the depth interval 65.5 to 75.3 metres. The Middle Aquifer pinches out to the north, and was not encountered in a test hole drilled for Norcen Ltd. on Sec6-Tp77-Rge2-W4M (Hydrogeological Consultants Ltd., 1976).

B.P. - Dome Tributary

British Petroleum's Cold Lake Pilot plant, situated on LSD9-Sec7-Tp66-Rge5-W4M utilizes ground water from the flank of a north-trending tributary of the Beverly Channel. This tributary has been termed the B.P. - Dome Tributary for the purposes of this report. The Middle Aquifer is only about 7.6 metres thick in the vicinity of LSD9-Sec7, and is located directly on the bedrock surface.

The report by Hydrogeological Consultants Ltd. (1976) indicates that as one goes eastward from the B.P. plant site toward the centre of the tributary channel, the Middle Aquifer grows progressively thicker, better-sorted, and more well-defined. A similar trend is seen going in a southerly direction, toward the Beverly Channel. On the west side of the tributary, in NE½-Sec5½-Tp67-Rge4-W4M, a well completed for Dome Petroleum Ltd. by Stanley



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ELECTRIC LOG — BOURQUE LAKE
OBSERVATION WELL

DATE MARCH/82

SCALE

FIGURE 6

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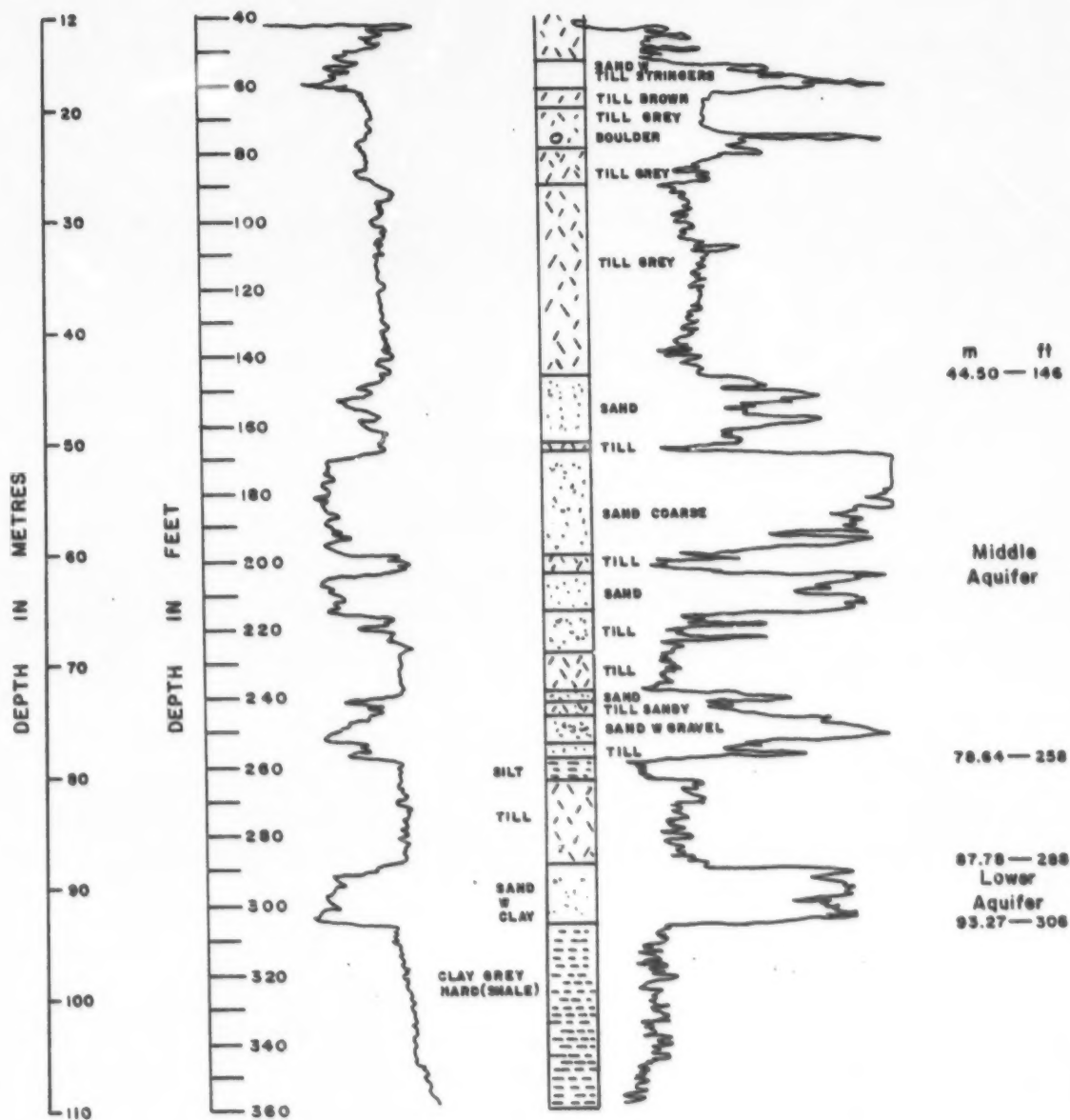
and Associates (1981) utilizes approximately 13.7 metres of interlayered coarse-grained sand and fine gravels present over the depth interval 73 to 87 metres. The sands are made up of rounded, well sorted quartz, quartzite, and chert fragments. It is separated from the bedrock by approximately 36.5 metres of glacial till. The exact relationship between the aquifer here and that utilized by B.P. is not known.

Further south, near the confluence of the B.P. - Dome Tributary, and the Beverly Channel, an observation well, drilled and tested by Alberta Environment, on SW $\frac{1}{4}$ -Sec28-Tp65-Rge5-W4M encountered the Middle Aquifer over the depth interval 114.6 to 130.8 metres. The thickness and the well-developed aspect of the Middle Aquifer at this location can be attributed to the proximity of the test hole to the centre of the Beverly Channel.

Helina Channel

On LSD7-Sec4-Tp63-Rge8-W4M, a well drilled by Amoco Canada Ltd. encountered a series of sand and gravel layers in the Helina Channel. A log of this hole (Figure 7) shows that the Middle Aquifer is less definable as a single unit than in the Beverly Channel. The aquifer consists of two distinct sand and gravel layers. The lower-most is 5.5 metres thick and appears to be a poorly sorted interlayering of sand and till. The uppermost part is much thicker (approximately 28 metres) and is separated from the lower by a 6-metre layer of till. The upper layer also consists of interlayered sand and till, although it appears to be very well sorted throughout one 9-metre interval. The lithology is not specified in the Amoco Report (Elk Point Drilling Corp., 1979).

The bimodal nature of the Middle Aquifer in the Helina Channel is also seen in a well drilled for Canada Cities Service on LSD6-Sec13-Tp63-Rge8-W4M (UMA Group,



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ELECTRIC LOG OF AMOCO WELL

DATE MARCH/82

SCALE

(7-4-63-8-4)

FIGURE 7

MLM Ground-Water Engineering

1981). Here, the log shows less deflection than in the Amoco well, but the stratigraphic succession is very similar.

In a well completed by Alberta Environment in March, 1982, on SW $\frac{1}{4}$ -Sec27-Tp63-Rge7-W4M, the Middle Aquifer is represented by a 61-metre zone of sand found over the interval 53.6 to 59.7 metres, and a 5.8-metre zone in the interval 77.4 to 83.2 metres (R. Hicks, pers. comm.). At this location, the Middle Aquifer appears to be more clearly demarcated from the contiguous till units than is observed in the Amoco well (LSD7-Sec4-Tp63-Rge8-W4M) and the bimodality of the aquifer is well demonstrated.

The obvious differences between the Middle Aquifer in the Helina Channel, and the better-defined examples seen in the Beverly Channel (Bourque Lake observation well; Town of Glendon) must be taken as an indication that while we may find it expedient to simplify the geology of the area for the purposes of presentation, the presentation may not be totally accurate. The series of geological events, which resulted in the observed distribution of aquifer materials at different locations, were not the same throughout the entire region. Such factors as ice front position, the trend of the bedrock channel, and the local quality and type of material dropped from the ice mass, varied from place to place, and over time. Therefore, the stratigraphic sequence from place to place may commonly reflect large, regional depositional events, but the smaller events will be reflected only locally.

Ft. Kent - Ardmore Tributary Channel

The Ft. Kent - Ardmore Tributary shown in Figure 3 was originally introduced in a report by Geoscience Consultants Ltd. (1978) for World Wide Energy Co. Ltd. (WECO), who had commissioned a test-drilling program on sections 28 and 29, Tp61-Rge4-W4M.

On SW $\frac{1}{4}$ -Sec28-Tp61-Rge4-W4M, the Middle Aquifer is encountered within the depth interval 46 to 67 metres (Figure 8). A test hole drilled on LSD9-Sec29-Tp61-Rge4-W4M shows that the aquifer thins from 21 to 18 metres, and becomes more clayey to the northwest of section 28. A well belonging to the M.D. of Bonnyville, situated on LSD8-Sec32-Tp61-Rge4-W4M is completed in the Middle Aquifer, which is 15 metres thick at this location.

Within the Ft. Kent - Ardmore Tributary, the Middle Aquifer is well-defined, and the stratigraphy is similar to that of the Beverly Channel.

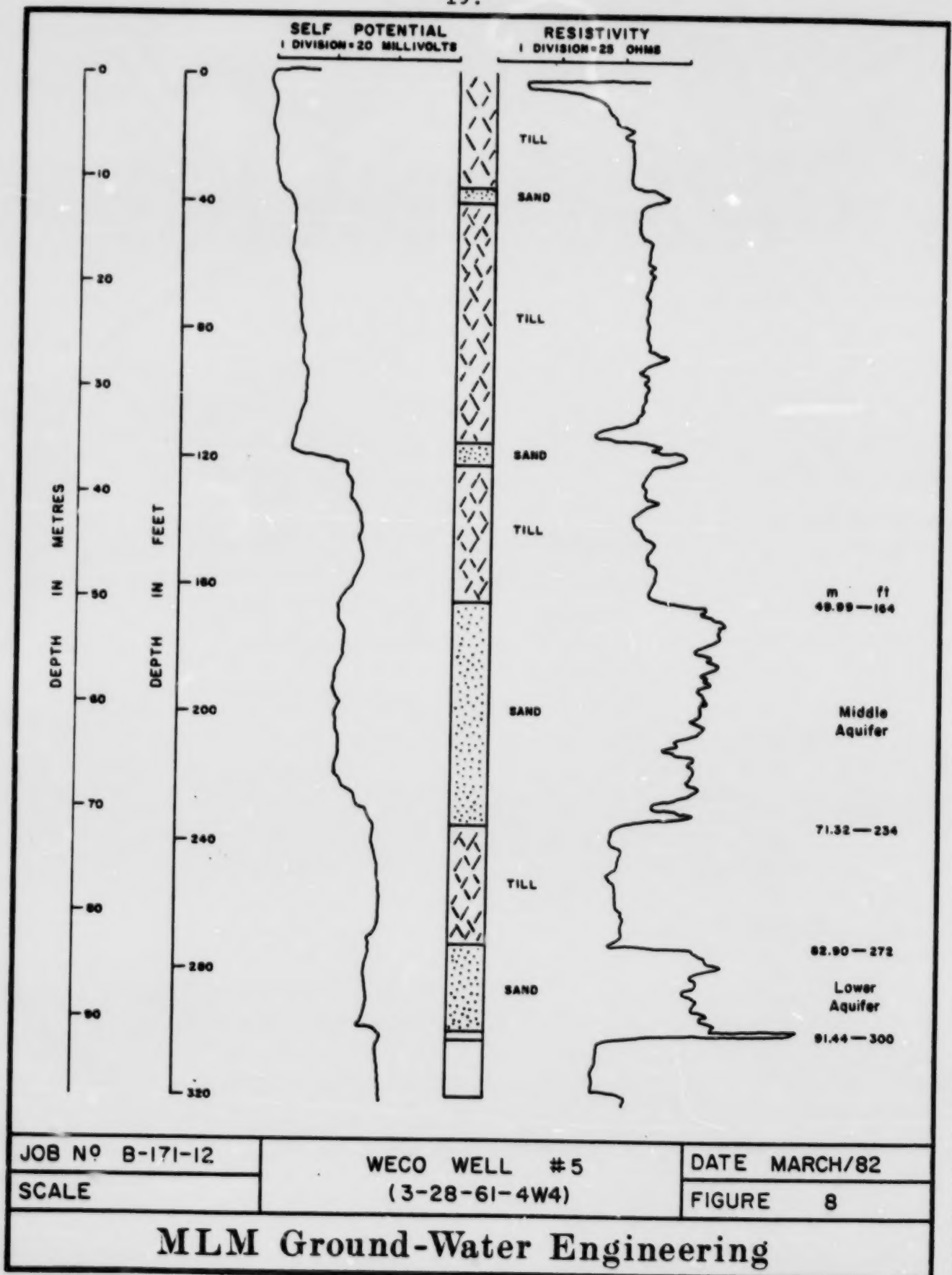
2.4 Lower Aquifer

The Lower Aquifer, which consists of preglacial sand and gravel is often found either in direct contact with the bedrock, or separated from the bedrock by a thin layer of clay. It attains its maximum thickness near the thalweg of the preglacial valleys and tends to pinch out abruptly against the valley walls as one moves away from the centre. An isopach map is shown in Figure 9 (pocket).

The thickness, homogeneity, and degree of sorting in the preglacial alluvium varies somewhat within the confines of the buried channels themselves. These variations however, do not appear to be as pronounced as in the Middle Aquifer. It can be assumed that the deposition of the preglacial alluvium occurred under considerably more stable conditions than that of the glaciofluvial sediments of the Middle and (especially) the Upper Aquifer.

Beverly Channel

Baydack and Render (1981), during their water supply investigation for the Village of Glendon, drilled a number of test holes on Sections 24, 25, 26 and 30, Tp60-



Rge8-W4M and Section 4-Tp61-Rge8-W4M. The electrolog from the main production well is shown in Figure 5. The most favourable development of the lower aquifer was found in SE $\frac{1}{4}$ -Sec26-Tp60-Rge8-W4M. At this location, the aquifer is approximately 16.7 metres thick, and lies directly upon the Lea Park shales.

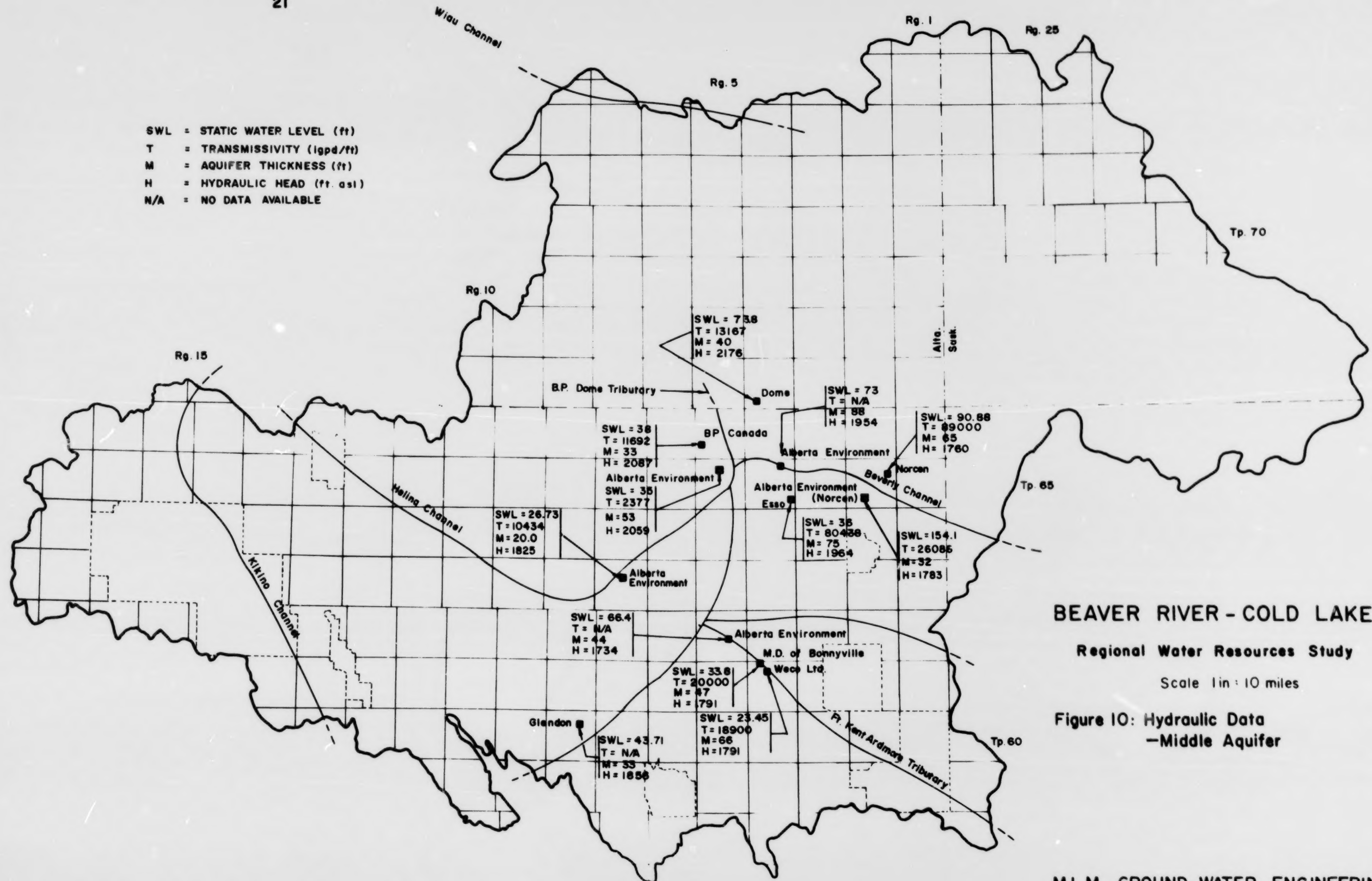
At the Bourque Lake observation well (LSD4-Sec26-Tp65-Rge4-W4M), the Lower Aquifer is well developed. It is approximately 23 metres thick at this location, and is located directly on the bedrock surface (Figure 6).

In the Cold Lake area, the well drilled by Norcen Ltd. on LSD12-Sec4-Tp65-Rge2-W4M shows that the Lower Aquifer is well-developed and appears cleaner and coarser-grained than the Middle Aquifer at that location.

B.P. - Dome Tributary

No evidence of the Lower Aquifer is found in the B.P. and Dome wells (LSD9-Sec7-Tp66-Rge5-W4M, and NE5-TP67-Rge4-W4M, respectively). These two wells are thought to be situated far enough from the centre of the B.P. - Dome Tributary for the Lower Aquifer to have pinched out. The Lower Aquifer is thought however, to reach a thickness of greater than 15 metres toward the centre of the tributary, as indicated on the isopach map (Figure 9). The Alberta Environment well (Figures 2 and 10), located on SW $\frac{1}{4}$ -Sec28-Tp65-Rge5-W4M is situated approximately three km west of the suspected confluence of the B.P. - Dome Tributary and the Beverly Channel. This well encountered the Lower Aquifer over the depth interval 147 to 154 metres. According to the field geologist's report (J. Marciniuk, pers. comm.), the aquifer at this point is relatively poorly sorted. No hydrogeologic data on the Lower Aquifer were collected at this location.

SWL = STATIC WATER LEVEL (ft)
 T = TRANSMISSIVITY (lgpd/ft)
 M = AQUIFER THICKNESS (ft)
 H = HYDRAULIC HEAD (ft. a.s.l.)
 N/A = NO DATA AVAILABLE



BEAVER RIVER - COLD LAKE

Regional Water Resources Study

Scale 1 in = 10 miles

Figure 10: Hydraulic Data
—Middle Aquifer



Helina Channel

In the Helina Channel, the investigations conducted by Amoco Ltd. and Canada Cities Service Ltd. indicate that the Lower Aquifer is considerably thinner than in the Beverly Channel. In Amoco's test hole (SW $\frac{1}{4}$ -Sec4-Tp63-Rge8-W4M), the thickness of the Lower Aquifer is only about 4.87 metres (Figure 7); in the Canada Cities Service well (LSD6-Sec13-Tp63-Rge8-W4M), the thickness is 6 metres.

In the Alberta Environment well completed on SW $\frac{1}{4}$ -Sec27-Tp63-Rge7-W4M, the Lower Aquifer is encountered over the depth interval 93 to 106.1 metres. Its thickness here (6.1 metres) is slightly greater than that observed in either the Canada Cities Service, or the Amoco wells. Although the increased thickness is not great, this could be an indication of a general thickening of the Lower Aquifer in the Helina Channel, toward the northeast (Figure 9).

Ft. Kent - Ardmore Tributary

In the Ft. Kent - Ardmore Tributary, the Lower Aquifer appears to be as well defined as in the Beverly Channel. The report by Geoscience Consulting Ltd. (1978), shows electrologs from several test holes drilled in and around SW $\frac{1}{4}$ -Sec28-Tp61-Rge4-W4M. In this area, the Lower Aquifer is approximately 8.5 metres thick (Figure 8). On LSD9-Sec29-Tp61-Rge4-W4M it is 4.3 metres thick.

2.5 Wiau Channel

At present, very little is known about the aquifer geology of the Wiau Channel (Figure 3).

Alberta Energy Co. Ltd. is presently conducting a test drilling program, and Alberta Environment has completed a test well.

The Alberta Environment well was drilled on LSD10-Sec29-Tp73-Rge7-W4M to a depth of 308 metres. The well is completed in a fine sand, encountered over the depth interval 270 to 308 metres (C. Gold, pers. comm.).

The overburden is much thicker in the northern part of the study area than in the south. The sand unit encountered by the Alberta Environment well is probably "Lower Aquifer", by virtue of its depth. This is not known for sure however, and more data are required.

3.0 HYDROGEOLOGY

3.1 Aquifer Hydraulic Properties

From limited pumping test data on nine wells, the average value of the hydraulic conductivity of the Middle Aquifer is calculated to be 23.1 m/day. It appears to be hydrologically confined throughout the study area. Since most pumping tests were conducted without observation wells, only one value of the storage coefficient has been calculated for the Middle Aquifer. It is 2.6×10^{-3} .

From eight pumping tests, the average value of the hydraulic conductivity of the Lower Aquifer is calculated to be 20.1 m/day. This aquifer also appears to be hydraulically confined throughout the study area. From work done for the Village of Glendon and Belvedere Developments, the storativity has been calculated to be 2.5×10^{-4} .

The results of pumping tests conducted throughout the region indicate that for the duration of a given test, no hydraulic connection between the Middle and Lower Aquifer is seen. This is not to say however, that on the longer term, an interconnection of the aquifers would not be observed.

In a pumping test conducted for Alberta Environment on their well located on LSD10-Sec29-Tp73-Rge7-W4M, the hydraulic conductivity of the lowermost sand aquifer

in the Wiau Channel, was calculated to be 5.1 m/day, with a storage coefficient of 2×10^{-2} .

The results of pumping tests conducted on wells completed in the Middle and Lower Aquifer are presented in Table 2. The location of the data points and the respective hydraulic parameters for the Middle and Lower Aquifers are shown in Figures 10 and 11, respectively.

3.2 Natural Ground-Water Flow

Values of hydraulic head have been calculated from static water level measurements taken during pumping tests and from observation well records. These are presented in Table 3. Where estimates had to be made, it was assumed that the top of the casing was 0.6 metres (two feet) above ground level and that ground level elevations could be determined from 1:50,000 topographic maps. The hydraulic head data has also been summarized in Figures 10 and 11.

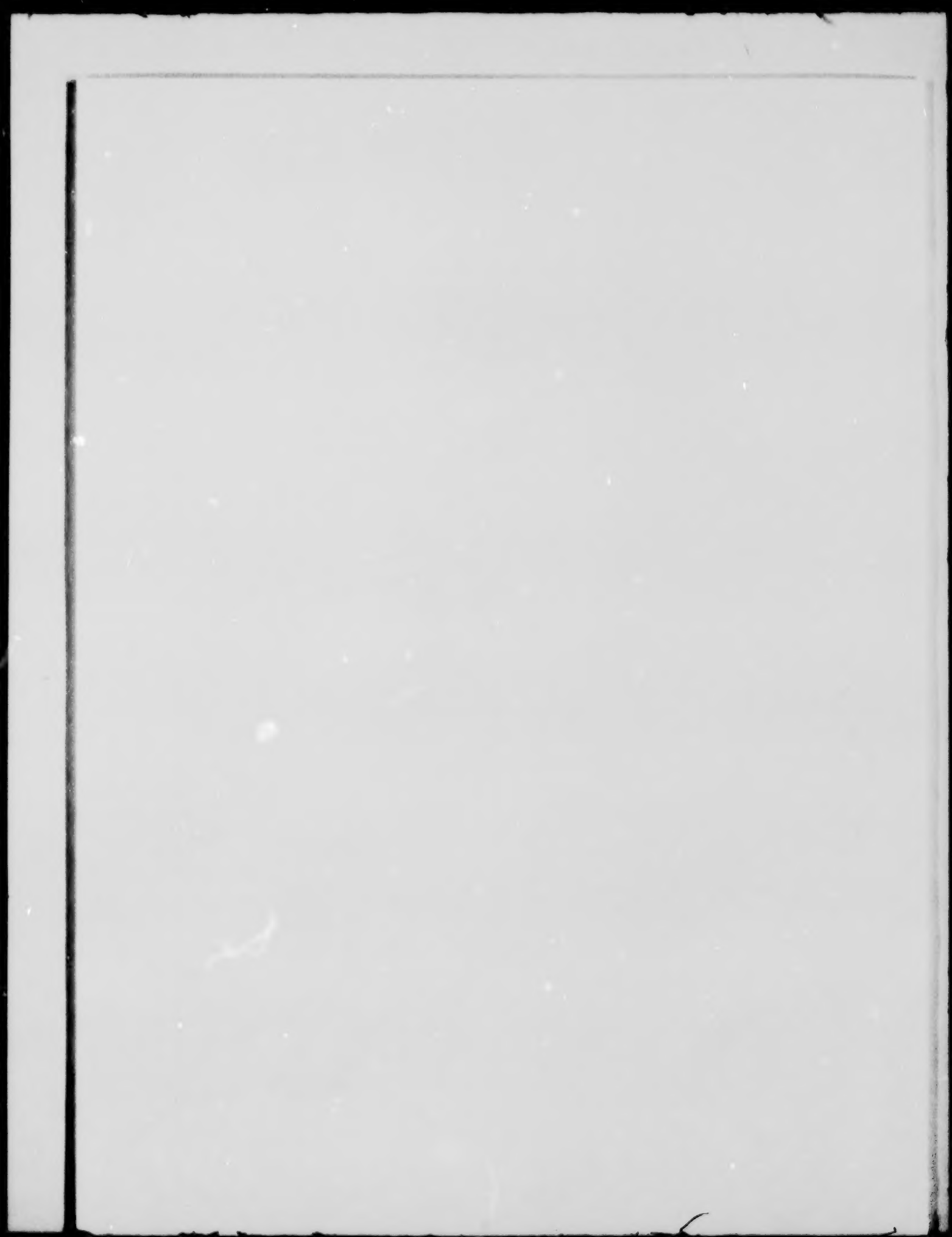
Examination of this data indicates that the direction of natural ground-water movement through both the Middle and Lower Aquifers is not well understood. In the Middle Aquifer (Figure 10), flow through the B.P. - Dome Tributary and the Beverly Channel is toward Cold Lake. Hydraulic head data in the Ft. Kent - Ardmore Tributary indicates ground-water flow is toward the Beverly Channel.

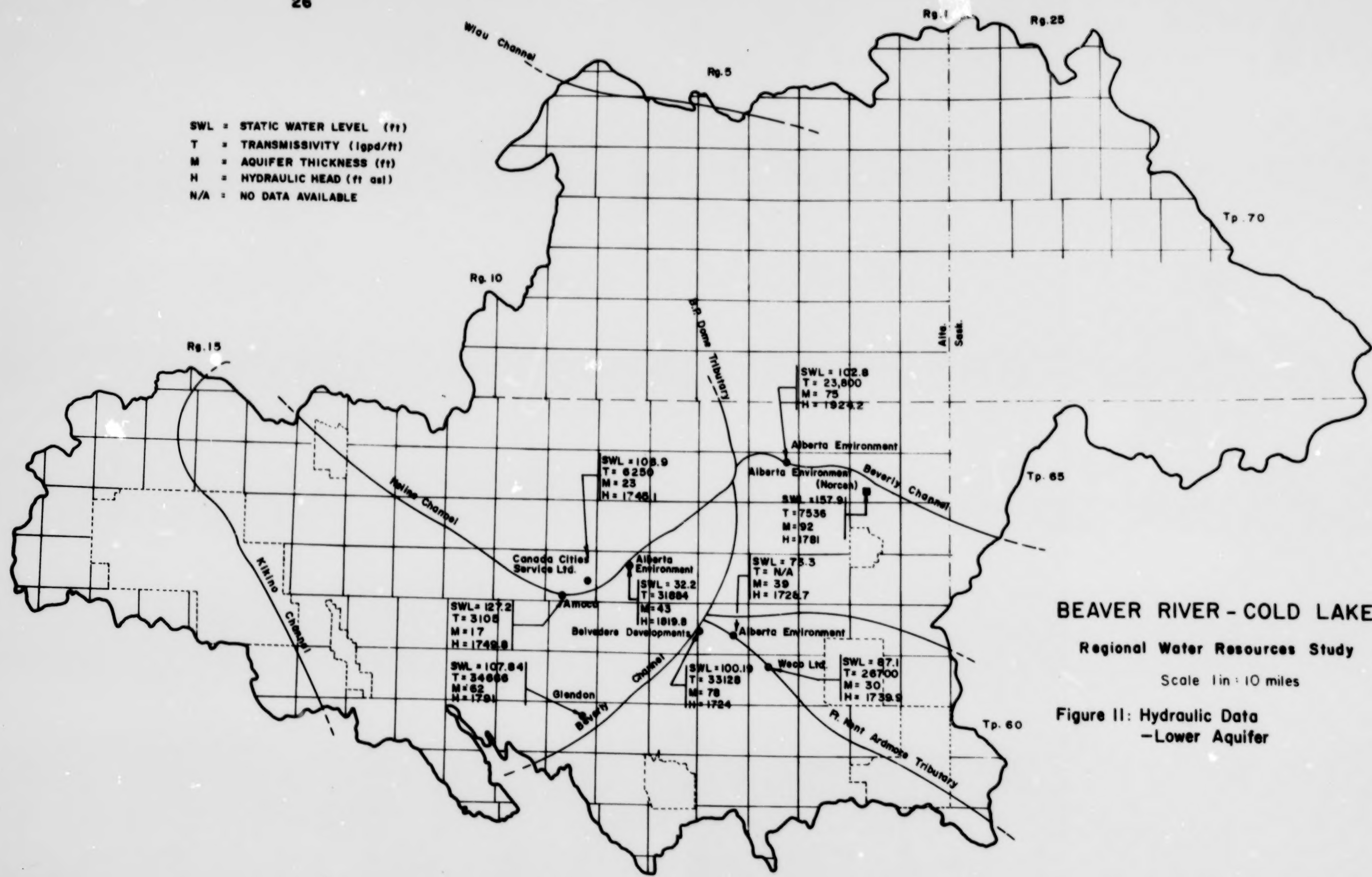
However, the direction of flow within the Beverly Channel is not known. The hydraulic head at the Alberta Environment Bonnyville observation well is approximately 528.5 metres (1734 feet), while at the Bourque Lake site, the hydraulic head is 595.5 metres (1954 feet). It is obvious that additional hydraulic head data for the Middle Aquifer in the Beverly Channel is required before the flow system can be defined.

TABLE 2

Results of Pumping Tests

<u>Well Owner</u>	<u>Aquifer Tested</u>	<u>Transmissivity (m²/day)</u>	<u>Aquifer Thickness (m)</u>	<u>Hydraulic Conductivity (m/day)</u>
B.P.	Middle	174.26	10.06	17.3
Esso	Middle	155.55	22.86	6.8
Norcen	Middle	1,326.46	19.81	67.0
Alta. Env. (Norcen)	Middle	388.79	9.75	39.9
M.D. Bonnyville	Middle	298.08	14.33	20.8
Weco	Middle	245.92	20.12	12.2
Alta. Env. (Helina Chan.)	Middle	155.51	16.15	25.5
Dome	Middle	196.24	12.19	16.1
Alta. Env. (B.P. - Dome Tributary)	Middle	35.43	16.15	2.19
Amoco	Lower	46.28	5.18	8.9
Canada Cities Service	Lower	93.15	7.01	13.3
Weco	Lower	411.35	9.14	45.0
Belvedere Developments	Lower	493.74	23.77	20.8
Village of Glendon	Lower	516.66 (av)	18.90 (av)	27.3
Alta. Env. (Bourque Lake)	Lower	354.20	23.47	15.09
Alta. Env. (Norcen)	Lower	112.32	28.04	4.0
Alta. Env. (Helina Chan.)	Lower	475.20	16.15	36.3
Petro Canada	Lower Wiau Chan.	190.12	38.10	5.1





BEAVER RIVER - COLD LAKE
 Regional Water Resources Study
 Scale 1 in : 10 miles

**Figure II: Hydraulic Data
 -Lower Aquifer**



TABLE 3

Hydraulic Heads

<u>Well Owner</u>	<u>Aquifer Screened</u>	<u>Hydraulic Head in metres above mean sea level</u>
B.P.	Middle	636.12
Esso	Middle	598.63
Norcen	Middle	536.45
Alberta Envir. (Norcen)	Middle	543.46
M.D. Bonnyville	Middle	545.90
Weco	Middle	545.90
Dome	Middle	663.24
Alberta Envir. (Helina Channel)	Middle	556.26
Alberta Envir. (Bourque Lake)	Middle	592.84
Alberta Envir. (Ft. Kent)	Middle	528.52
Alberta Envir. (B.P. - Dome)	Middle	627.58
Amoco	Lower	533.40
Canada Cities Service	Lower	531.88
Weco	Lower	530.35
Belvedere Dev.	Lower	525.48
Village of Glendon	Lower	545.90
Alberta Envir. (Bourque Lake)	Lower	586.44

Table 3 ...

<u>Well Owner</u>	<u>Aquifer Screened</u>	<u>Hydraulic Head in metres above mean sea level</u>
Alberta Envir. (Ft. Kent)	Lower	527.00
Alberta Envir. (Norcen)	Lower	542.85
Alberta Envir. (Helina Channel)	Lower	554.74
Petro Canada	Wiau Channel	657.45

In the Lower Aquifer (Figure 11), the direction of flow is generally away from the area of Bourque Lake and the confluence of the B.P. - Dome Tributary with the Beverly Channel. At Bourque Lake, the hydraulic head in the Lower Aquifer is very high (586.5 metres or 1924.2 feet). The explanation for this is not readily apparent, although it may indicate that infiltration in this area is comparatively rapid, and that high hydraulic heads exist here as a result of interconnection between the Upper, Middle and Lower Aquifers. Such a model would serve to explain the observed potential for southwesterly flow of ground water in the Helina and Beverly Channels, away from Bourque Lake, without contradicting the observed potential for flow from Bourque Lake to the east, toward Cold Lake.

In addition to the lateral movement, there is the potential for a general downward movement of water from the Middle Aquifer to the Lower Aquifer over the entire study area. In fact, the discharge zones for both of the major aquifers have not yet been located.

Local ground-water flow systems are superimposed on the regional systems associated with the Middle and Lower Aquifers. These shallow local flow systems give rise to relatively fresh springs and flowing wells along river valleys and near lakes. Shallow aquifers modify these local flow systems.

3.3 Hydrochemistry

The typical mineralogic composition of tills in the Beaver River - Cold Lake area includes silicates (quartz, plagioclase, minor hornblend and pyroxene, the clay minerals illite, kaolinite, and montmorillonite), carbonates (calcite, dolomite), sulphates (gypsum, anhydrite) and

chlorides (halite, mirabilite). Because ground waters, in order to replenish the aquifers in the region, must percolate through the thick layers of glacial drift, their hydrochemistry may be expected to reflect the mineralogic composition of the drift.

Throughout much of the study area, carbonate and bicarbonate constitute over 60 per cent of the total ions on an equivalents-per-mil basis in all three aquifers (Ozoray, Wallick, and Lytviak, 1980). Sulphate constitutes more than 60 per cent of all anions over only a few very small areas throughout the region.

Of the cations, sodium plus potassium concentrations of greater than 60 per cent of total cations are found in small, isolated areas in the shallower aquifers, but become more common as one inspects the Middle and Lower Aquifers.

In all three aquifers: Upper, Middle, and Lower; the total dissolved solids concentrations are generally quite low, with concentrations below, or only slightly above 1000 mg/l. Exceptions to this are found in the southwest portion of the study area, in townships 63 through 65, ranges 11 through 13, where, according to Oxoray, Wallick, and Lytviak (1979), tds values greater than 2000 mg/l are found in the Upper Aquifer.

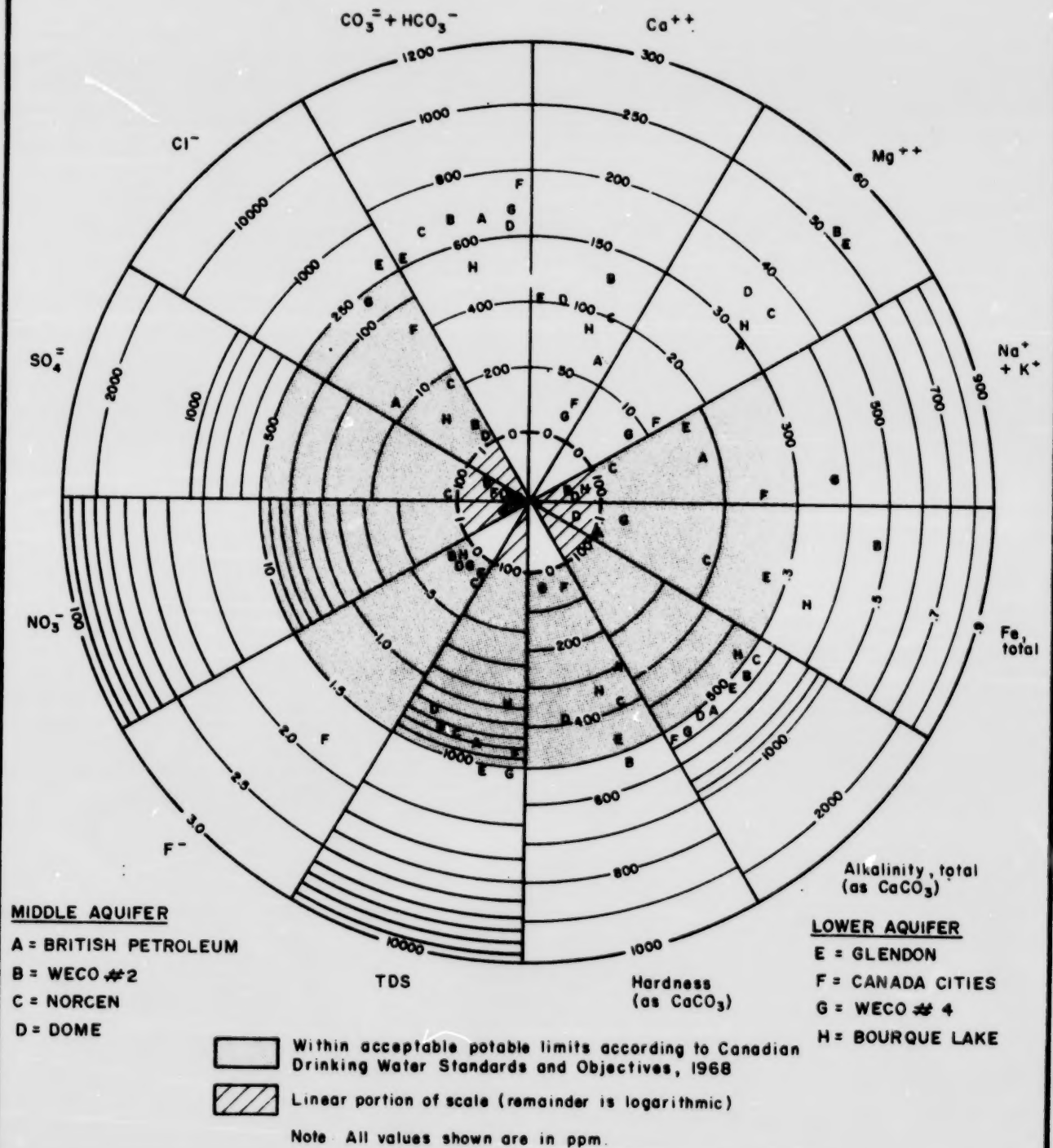
The ground waters in the Upper Aquifer are generally slightly saturated with respect to calcite and dolomite and undersaturated with respect to gypsum, illite, kaolinite, microcline, albite, and anorthite. The saturation index data suggest that calcite and clay minerals in the till, particularly calcium montmorillonite, are the controlling solid phases in the development of the shallow ground water chemistry (Ozoray, Wallick and Lytviak, 1979).

The chemical quality of ground waters from the Middle Aquifer is similar to that of the Upper. The waters are generally of good quality, and are suitable for municipal and steam injection purposes, with some treatment (Figure 12). Appendix A contains a summary of water analyses from both the Middle and Lower Aquifers. The ground waters from the Middle Aquifer are frequently high in iron, and are generally harder than those of the Lower Aquifer. The total dissolved solids concentration from the Middle and Lower Aquifers seldom exceeds 1000 mg/l. The waters are generally alkaline, with no apparent relationship between alkalinity and aquifer type. The noticeable constancy of the anion combination bicarbonate plus carbonate is due to saturation with respect to calcite.

Very generally, the concentration of the alkaline earth cations calcium and magnesium is higher in the Middle Aquifer than in the Lower. The alkali metal cations, sodium plus potassium, are more concentrated in the Lower Aquifer. This can be attributed to the tendency for ground waters associated with deeper, longer flow systems to evolve toward a sodium-rich chemical content, at the expense of calcium and magnesium. Calcium is taken up by cation exchange with clay particles, while sodium is released.

The Middle Aquifer, being rich in carbonate minerals, yields waters of a fairly constant concentration of carbonate plus bicarbonate. In waters from the Lower Aquifer, these concentrations are less constant from place to place, because the equilibrium has been changed by cation exchange.

Sulphate tends to be found in higher concentrations in the Middle Aquifer (generally less than 100 mg/l) than in the Lower, possibly due to the presence of gypsum and anhydrite in the sediments.



JOB No. B-171-12

SCALE

WATER QUALITY IN SURROUNDING WELLS

DATE MARCH/82

FIGURE 12

MLM Ground-Water Engineering

Iron concentrations in both the Middle and Lower Aquifers are quite high, with only two samples from the Middle Aquifer (Dome Petroleum's well, and Weco well #2) below the accepted potable limit of 0.3 mg/l as set forth by the Canadian Drinking Water Standards (1969).

Alkalinity is quite constant as a result of the constancy in the carbonate and bicarbonate content.

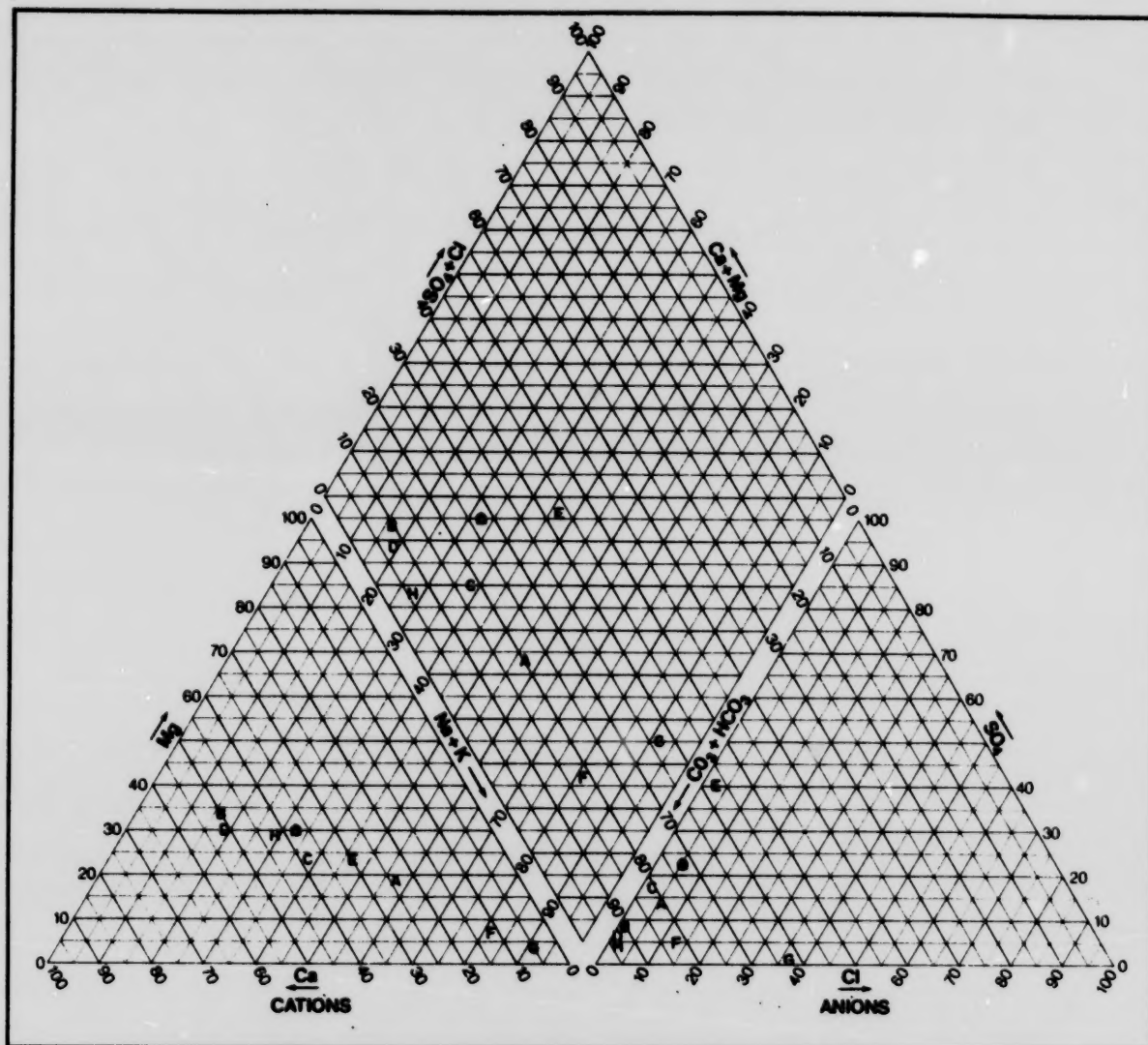
Chloride is variable, but in general, slightly more concentrated in the Lower Aquifer. This can probably be attributed to the release of some chloride into a solution from the Lea Park Formation of the Colorado Group, both of which are marine shales.

Fluoride concentrations are very constant throughout the Lower, Middle, and Upper Aquifers. This suggests that it is input very early in its journey through the ground water system, perhaps as it infiltrates through the upper tills. Because fluoride is a very conservative ion, no further changes in concentration are observed.

Nitrate and Nitrites are at very low concentrations in all locations sampled. This can be attributed to a very low natural nitrate content in the aquifer materials, and an unpolluted ground water regime.

From the accompanying Piper trilinear diagram, (Piper, 1944), on which are plotted several water samples from different locations within the Lower, Middle and Upper Aquifers, the following conclusions can be drawn:

a) There is a general tendency for ground waters moving downward from the level of the Middle Aquifer to that of the Lower Aquifer, to take up sodium and potassium, at the expense of calcium and to a lesser extent, magnesium.



MIDDLE AQUIFER

A = BRITISH PETROLEUM 97-66-S-4
 B = WECO No. 2 5-25-61-4-4
 C = NORCEN
 D = DOME

LOWER AQUIFER

E = GLENDON
 F = CANADA-CITIES
 G = WECO 4
 H = BOURQUE LAKE

UPPER AQUIFER

● = NW 28-61-4-4 (private well)

Piper Trilinear Plot

Figure 13

b) The Lower Aquifer is richer in dissolved chloride, probably because the ground waters come into contact with the marine shales of the bedrock.

c) No well-defined spatial relationship in the epm ratios of bicarbonate and sulphate can be seen.

d) As waters infiltrate deeper, they undergo a very general shift from calcium-magnesium bicarbonate to sodium-bicarbonate type.

4.0 GROUND-WATER SUPPLY POTENTIAL

4.1 Ground-Water Availability

4.1.1 Methodology

In order to determine the existing and potential significance of ground water as a supply source, it was necessary to devise a method to quantify the ground water resources of the region. The use of single well yields, particularly when they are few in number, is not satisfactory. Indeed, most of the aquifer tests conducted were at a rate far less than the calculated safe yield, which casts some doubt on their validity. There are also cases where ground-water withdrawals have had to be restricted as ground-water mining occurred at pumping rates far less than the theoretical safe single well yield.

For this study, we have attempted, where sufficient data is available, to calculate the natural ground-water flow-through for the two major aquifers using the following form of Darcy's Law:

$$Q = 0.365 K I A$$

where: Q = flow rate in dam³/year
 K = hydraulic conductivity in m/day
 I = hydraulic gradient in m/m
 A = crosssectional area of the aquifer in m²

The values of hydraulic conductivity and hydraulic heads were obtained from the aquifer tests conducted in the two aquifers. Crosssections were prepared of the aquifers where hydraulic data was available, and the aquifer areas were calculated from them. The location of the crosssections are shown in Figure 4 (pocket). This flow rate represents the volume of water that moves through the aquifer under natural conditions. Ground water can be removed from an aquifer up to this natural flow without removing water from storage and depleting the aquifer.

Therefore, a comparison of the current and potential ground-water demands to the natural ground-water flow-through will identify areas where supplies may be adequate and areas where ground-water mining may occur. This method has its limitations in that it does not consider leakage from overlying formations. That is, it more or less treats the aquifers as two pipelines. We feel that this method does provide a good first approximation of the regional ground-water resources, and that areas of potential problems can be identified. In this way, future data collection requirements can be determined. However, the flow values calculated should be taken as qualitative due to the limited data.

4.1.2 Middle Aquifer

The calculations of natural ground-water flow in the Middle Aquifer are given in Table 4. The

TABLE 4
Ground-Water Availability
Middle Aquifer

Channel Reach	A (m ²)	I (m/m)	K (m/day)	Flow m ³ /day	Flow dam ³ /yr
Beverly Channel at Fort Kent	7.40 x 10 ⁴	2.28 x 10 ⁻³	20.8	2,770	1,010
Helina Channel	1.18 x 10 ⁵	2.77 x 10 ⁻³	13.8	4,526	1,652
Beverly Tributary at B.P.	1.24 x 10 ⁵	1.52 x 10 ⁻³	7.99	1,505	549
Beverly Channel at Norcen	1.24 x 10 ⁵	2.70 x 10 ⁻³	53.3 (av)	17,800	6,500

value of natural ground water flow at the Norcen site will represent the total flow in the Beverly Channel east of the B.P. - Dome Tributary.

Ground-water flow through the B.P. - Dome Tributary was calculated between B.P. (LSD9-Sec7-Tp66-Rge5-W4M) and the Alberta Environment observation well (SW $\frac{1}{4}$ -Sec28-Tp65-Rge5-W4M). An average value of hydraulic conductivity was used and a flow-through of 549 dam³/year was obtained.

Calculations made between Alberta Environment wells at the B.P. - Dome Tributary and the Helina Channel (SW $\frac{1}{4}$ -Sec27-Tp73-Rge7-W4M) indicated a flow-through of 1652 dam³/year to the south-west along the Helina Channel. This flow may include part of the flow down the B.P. - Dome Tributary.

4.1.3 Lower Aquifer

Calculations of natural ground-water flow through the Lower Aquifer over the various bedrock channel reaches are summarized in Table 5.

Examination of Table 5 shows that in the Beverly Channel, between Glendon and the Belvedere Developments well, near the confluence of the Ft. Kent - Ardmore Tributary, there is a flow of 412 dam³/year, to the north.

Flow in the Fort Kent - Ardmore Tributary is toward the Beverly Channel, and is calculated to be approximately 933 dam³/year.

Flow through the Helina Channel also presents a seeming paradox. Between Amoco (LSD7-Sec4-Tp63-Rge8-W4M) the ground water flow is indicated to be toward Cold Lake, at 87.6 dam³/year. The Lower Aquifer here is thin (approximately 7 metres thick) and the hydraulic conductivity is the lowest for this aquifer than anywhere else in the region.

TABLE 5

Ground-Water Availability
Lower Aquifer

<u>Channel Reach</u>	<u>A</u> <u>(m²)</u>	<u>I</u> <u>(m/m)</u>	<u>K</u> <u>(m/day)</u>	<u>Flow</u>	
				<u>m³/day</u>	<u>dam³/yr</u>
Beverly Channel Near Confluence of Fort Kent Tributary	7.91 x 10 ⁴	7.05 x 10 ⁻⁴	20.8	1,130	412
Beverly Tributary at Fort Kent	1.61 x 10 ⁵	3.50 x 10 ⁻⁴	45.0	2,560	933
Helina Channel (between SW1-Sec27- Tp63-Rge7-W4M and Amoco)	9.90 x 10 ⁴	1.66 x 10 ⁻³	11.1	1,673	611
Beverly Channel Tributary at B.P.	- No Lower Aquifer -				
Beverly Channel at Norcen	3.19 x 10 ⁵	2.49 x 10 ⁻³	4.0	3,191	1,165

Between the Alberta Environment well (SW $\frac{1}{4}$ -Sec27-Tp63-Rge7-W4M) and Amoco, however, the flow is calculated to be to the south-west, at a volume of 611 dam³/year. The latter of these estimates is probably the most reliable, as the head drop between these two wells is large (21.34 metres, or 70 feet), and not likely to be reversed by the error in estimating ground surface elevation from NTS Series maps. The head drop between Amoco and Canada Cities Service is very low (1.3 metres, or 4 feet), and could easily be reversed within the above-noted topographical error.

Calculations made between Alberta Environment's wells at Bourque Lake (LSD4-Sec26-Tp65-Rge5-W4M) and Norcen (SW $\frac{1}{4}$ -Sec9-Tp65-Rge2-W4M) yielded a natural flow-through of approximately 1165 dam³/year. This value represents the total flow through the Lower Aquifer in this portion of the Beverly Channel.

4.1.4 Wiau Channel

To date, very little hydraulic data is available for the two aquifers present in the Wiau Channel. Therefore, it was not possible to calculate natural flow rates through these aquifers. However the results of the test drilling and pump testing conducted by Alberta Environment in the Wiau Channel (10-29-73-7-4) indicate that significant supplies of ground-water can be obtained.

The aquifer testing indicated that the lower sand aquifer had a hydraulic conductivity value of 5.1 m/day, and a single well yield of in the order of 3394 m³/day (519 Igpm).

4.2 Ground-Water Demands

The ground-water demands for the study area have been prepared for the various time horizons by MacLaren Plansearch. This report breaks down the ground-water demands into three major types of users. They are: rural domestic, municipal and industrial users.

The bulk of the rural domestic water supplies in the study area are obtained from individual wells completed in the lensitic sands that make up the Upper Aquifer. As this aquifer was not extensively studied, only the municipal and industrial demands are used to determine the adequacy of ground-water supply. A summary of the municipal and industrial ground-water demands for the various bedrock channel reaches and time horizons is presented in Table 6.

Examination of these data indicates that between 1985 and 2000 there will be an increase in ground-water demands. However, in the year 2000, for the high development scenario, a number of the oil companies plan to switch to surface water. The companies that plan this switch are Union Texas, Dome Petroleum, Norcen, and Husky Oil. The projected ground-water demands for the year 2015 are less than the previous time horizon. This is primarily due to the completion of the Canada Cities plant in the low development scenario and the completion of the Amoco plant in the high development scenario.

4.3 Adequacy of Ground-Water Supply

It is expected that the Middle and Lower Aquifers would be heavily utilized by the municipal and industrial sectors, in any water usage scenario. Ground-water flow calculations have indicated that there is a substantial flow-through in each of these aquifers, as shown in Tables 4 and 5.

TABLE 6

Municipal and Industrial
Ground-Water Demands

<u>Channel Reach</u>	<u>User</u>	<u>Withdrawals (dam³/year)</u>					
		<u>1985</u>		<u>2000</u>		<u>2015</u>	
		<u>High</u>	<u>Low</u>	<u>High</u>	<u>Low</u>	<u>High</u>	<u>Low</u>
Helina Channel	Amoco Canada	1,160	--	2,320	--	--	--
	Canada Cities Services	410	--	750	410	750	--
Beverly Channel Upstream of Sec20-63-5-4	Village of Glendon	80	70	200	120	270	210
	Belvedere Dev.	?	?	?	?	?	?
Ft. Kent - Ardmore Tributary	Fort Kent - Ardmore	60	60	180	100	240	170
	Union Texas	180	130	--	130	--	130
B.P. - Dome Tributary	B.P. Canada	2,190	1,100	2,190	2,190	2,190	2,190
	Dome Petroleum	830	690	--	--	--	--
Beverly Channel	Norcen Energy Resources	140	140	--	460	--	460
	Husky Oil (Alberta)	165	--	--	--	--	--
TOTALS		5,215	2,190	5,640	3,410	3,450	3,160

Based upon the information gathered to date, the following comments are made as to the adequacy of ground-water supplies:

a) It appears the large ground-water demands in the Helina Channel (Amoco and Canada Cities) may not be obtained from the Middle and Lower Aquifers. The maximum demand of 3070 dam³/year (year 2000; high production) is in excess of the combined flow of 2263 dam³/year.

b) In the Beverly Channel south of Sec20-Tp63-Rge5-W4M, the only user of ground-water is the Village of Glendon. The maximum projected demand of 270 dam³/year is much less than the natural flow through the Lower Aquifer of 412 dam³/year in this part of the channel, which lies between Glendon, and the confluence of the Beverly Channel and the Ft. Kent - Ardmore Tributary.

c) Ground-water demands in the Fort Kent - Ardmore Tributary of the Beverly Channel are estimated to be 240 dam³/year in 1985. The users are Fort Kent - Ardmore, and Union Texas. Worldwide Energy plans to use waste water from Bonnyville. As the combined flow through this channel is approximately 1943 dam³/year, no problems are envisioned in meeting these demands.

It may be feasible for Union Texas to obtain their high development water demands of 730 dam³/year from ground-water rather than surface water. This would require a relocation of their well field to an area centrally located in the channel. If this was done, then the maximum ground-water demand in 2015 would be 970 dam³/year, which is still less than the natural flow through the channel.

d) In the B.P. - Dome Tributary of the Beverly Channel, the potential ground-water demand in 1985 is estimated to be between 1790 and 3020 dam³/year. This demand will decrease to 2190 dam³/year in 2000, as current scenarios show Dome switching to surface water. Existing data indicates a natural flow of 549 dam³/year, although it should be stated that the geologic relationships in the channel are not well understood.

e) In the remainder of the Beverly Channel, two oil sands plants are located. These are Norcen and Huskey Oil. In 1985, the ground-water demands are projected to be from 140 to 205 dam³/year. In 2000, the ground-water demand is 460 dam³/year for the low development scenario. The high development scenario has both oil companies utilizing surface water. The ground-water resources are capable of meeting the projected demands. It is likely that Norcen could use ground water for its high development scenario (1740 dam³/year) as the Middle Aquifer at Norcen's location is extremely productive (6500 dam³/year).

5.0 GROUND WATER - SURFACE WATER INTERACTION

One of the terms of reference of this study called for an accounting of ground water - surface water interaction, where the data is adequate and the interaction is critical to the surface water supply balance. In general, all ground water originates as surface water with the principal sources of natural recharge being precipitation and infiltration. Water enters water-bearing formations (aquifers), which then act as conduits for transmission and reservoirs for storage of ground water. This water travels slowly for varying distances until it returns to the surface by the action of natural flow. Ground water emerging into surface stream channels aids in sustaining streamflow when surface water input is low or non-existent.

As previously discussed in the section on natural ground-water flow, there is a shallow surficial flow system associated with the Upper Aquifer. This system is superimposed on the more regional flow systems of the Middle and Lower Aquifers. It is likely this shallow flow system discharges in places into surface water bodies. However, because of the limited data and relative unimportance of the Upper Aquifer, this flow system was not studied extensively.

Hydraulic head data available to date for the Middle and Lower Aquifers indicate that the vertical component of ground-water flow is downward. In fact, discharge areas for these aquifers have not yet been identified. This information, combined with the fact that the piezometric surface for these aquifers is below the level of most surface water bodies, indicates that these aquifers do not interact with surface water bodies. One exception is that the Middle Aquifer, and possibly the Lower Aquifer, may discharge into Cold Lake.

If increased utilization of ground water from the Middle and Lower Aquifers occurs in excess of the natural flow rates calculated in Section 4.2, an increase in the vertical hydraulic gradient may occur. This would result in increased leakage from the Upper Aquifer zones to the major aquifers below, which over the long term, could alter the shallow ground-water flow system. The increase in rural domestic use as predicted in the MacLaren Plansearch Report (3790 - 4300 dam³/year in 2015) would also alter existing shallow flow systems.

Recharge areas would likely expand, causing a reduction in surface water runoff. The size of discharge areas and the volume of ground-water discharge from the shallow system would also be reduced, which would in turn, alter the base flow components of streams and rivers.

It is impossible at this time to quantify this interaction because of insufficient data.

6.0 CONCLUSIONS

Based upon the findings of this study, the following conclusions are warranted:

a) The major aquifers within the study area that are capable of meeting municipal and industrial demands are the sand, sand and gravel, and gravel units associated with the preglacial bedrock channels. Two of these units have been found to be continuous over the study area, and have been called the Lower and Middle Aquifers in this report. The Lower Aquifer, in general, directly overlies the bedrock surface and is restricted to the bedrock channels. The Middle Aquifer is an intertill unit that tends to overlap the bedrock valley sides.

b) The recently identified Wiau Channel, the bulk of which is outside the study area, contains two major sand units of large areal extent. This valley has not yet been extensively studied.

c) The natural ground-water flow in the Lower and Middle Aquifer is not totally understood due to the limited data. The direction of flow in the Beverly Channel between the Fort Kent - Ardmore Tributary and Bourque Lake is not known. Flow in the Helina Channel is suspected to be in a southwesterly direction.

d) The water quality in both the Lower and Middle Aquifers is good, meeting most of the objectives set forth in the Canadian Drinking Water Standards. The one water sample from the Wiau Channel also exhibits good quality.

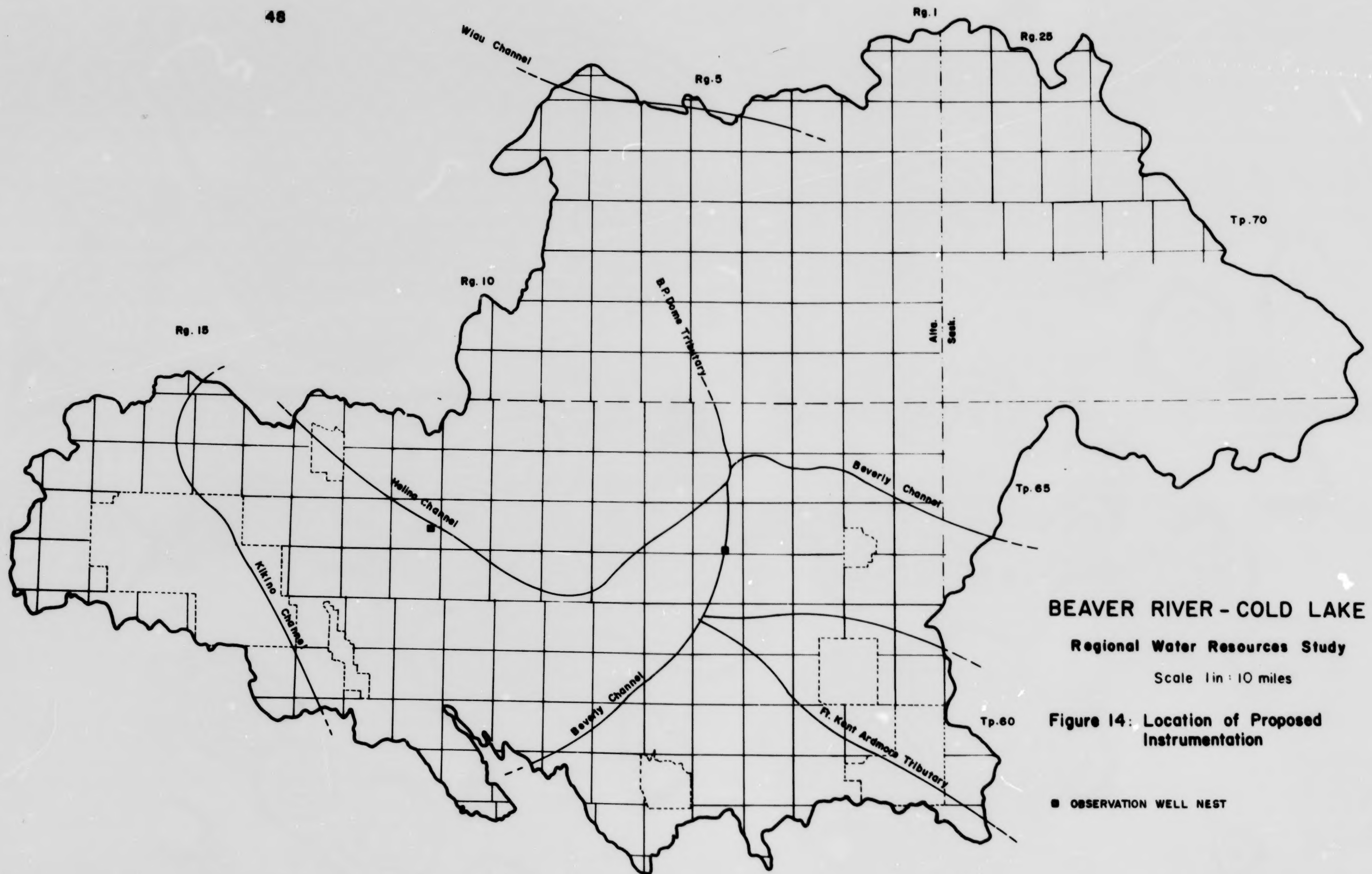
e) Review of the water demand scenarios indicates that regionally there are sufficient ground-water resources to meet the projected demands without causing depletion. However, locally there may be problems in meeting the demands. The most notable of these are: the Helina Channel at Amoco and Canada Cities, and the B.P. - Dome Tributary.

f) The exact interaction between ground water and surface water cannot be quantified at this time. It is felt the shallow ground water flow system associated with the Upper Aquifer, discharges into surface water bodies in the study area. The more regional flow systems associated with the Lower and Middle Aquifers does not appear to directly interact with surface water bodies, although the very high hydraulic heads at Bourque Lake, and within the B.P. - Dome Tributary may be an indication of ground-water recharge. Utilization of the Lower and Middle Aquifers above the natural flow rates could alter the shallow flow system. Also, increased utilization of the Upper Aquifer for rural domestic purposes as predicted in the MacLaren Plansearch Report, could further alter this system.

7.0 RECOMMENDATIONS

This study has identified problems in determining the regional flow system. It is important to have a good understanding of the flow system to determine the ground-water resources of the study. Because of this it is recommended that the following work be undertaken (Figure 14):

a) An observation well nest be placed in the Beverly Channel between the confluence of the Fort Kent - Ardmore Tributary and the confluence of the Helina Channel. This nest will provide data as to the direction of ground-water flow in this part of the channel.



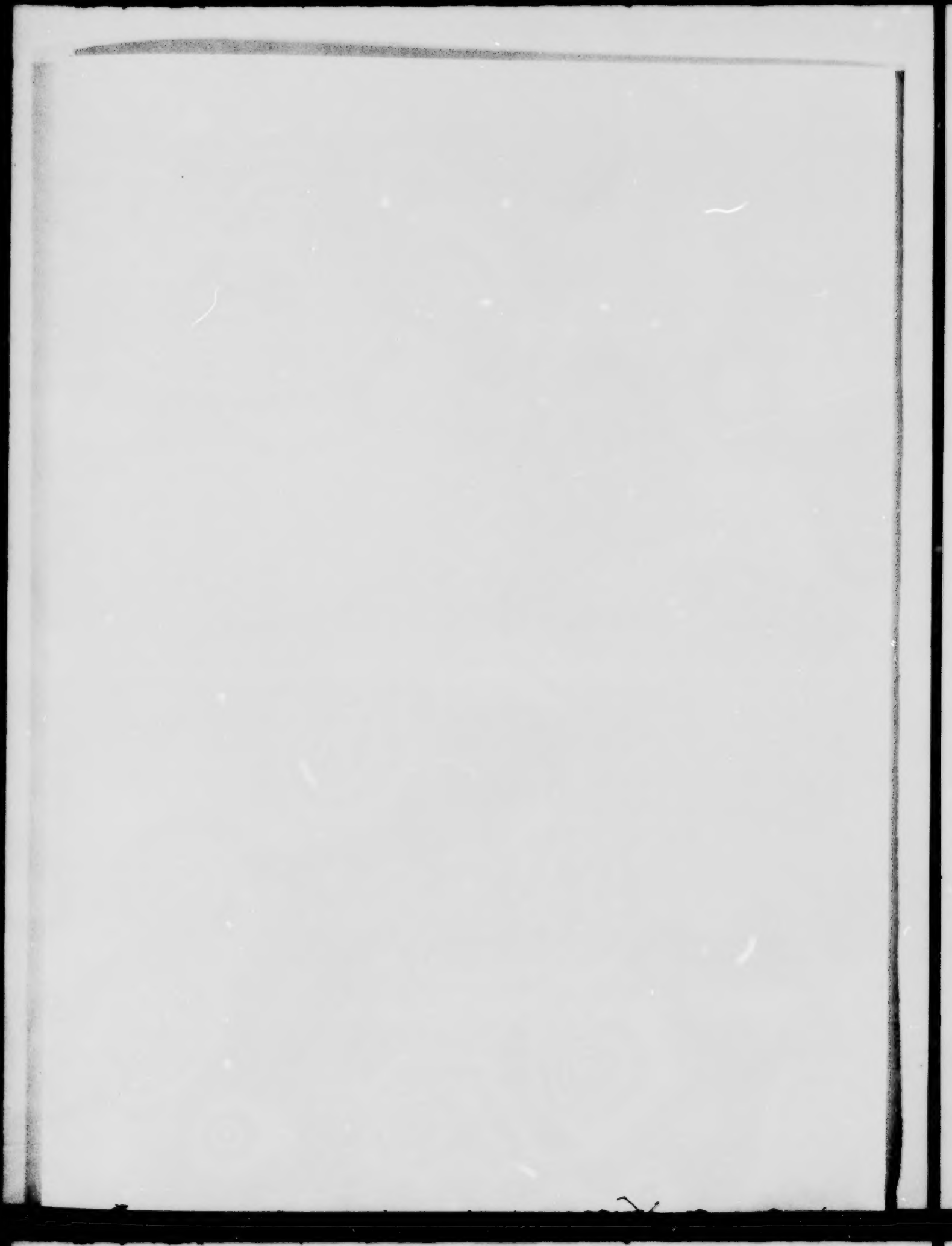
BEAVER RIVER - COLD LAKE

Regional Water Resources Study

Scale 1 in : 10 miles

Figure 14: Location of Proposed Instrumentation

■ OBSERVATION WELL NEST



b) An observation well nest be placed in the Helina Channel northwest of the Amoco site. This nest is necessary to verify the direction of natural flow in the Helina Channel. Because of the potential for ground-water utilization in the Helina Channel, this information is necessary.

As previously stated, the use of Darcy's Law to determine the regional ground-water resources is a simplification. Add to this the problems in ascertaining the natural flow direction in some area and one realizes the calculated values can only be used qualitatively. It is recommended that future data aquisition be geared to ultimately establish a three-dimensional computer model of the study area. Because of the complexity of the aquifer distribution and boundary conditions, this is the only method to quantify the impact of the various water demand scenarios.

The model can take into account the aquifer's areal distribution, variation in hydraulic parameters, leakage between aquifers, and recharge. Simulations of the various water demand scenarios will determine if the ground-water mining will occur and if so, if the resulting lowering of water levels is acceptable.

Once the model is established, then various scenarios can be run. Because of this, the model is an invaluable management tool for the quantifying and allocating of ground-water resources.

8.0

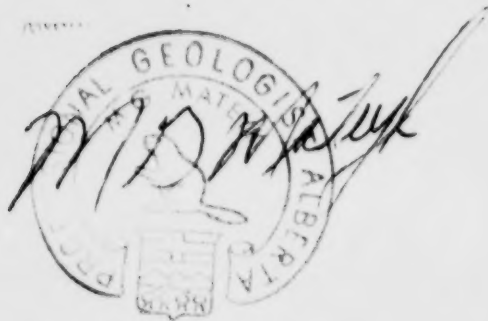
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Respectfully submitted,

MLM GROUND-WATER ENGINEERING



M. G. Mateyk, P. Geol.,
Project Hydrogeologist

APPENDIX A

Summary of
Water Quality Analyses

Determination	Canada Cities 6-13-63-8-4	B.P. 9-7-66-5-4	Weco #2 5-25-61-4-4	Weco #4 3-28-61-4-4	Weco 5-28-61-4-4	M.D. Bonnyville SE32-61-4-4	Norcen 12-24-65-2-4
pH	8.2	7.5	7.1	8.38	7.10	--	7.8
TDS	896	796	726	1006	696	914	735
Sodium	245	} 180	} 46	376	} 49	204	112
Potassium	4.1			8		--	--
Calcium	30	68	127	18	120	--	100
Magnesium	12	29	51	8	50	--	36
Carbonate	Nil	Nil	Nil	20	Nil	--	Nil
Bicarbonate	684	670	690	680	662	--	636
Alkalinity	561	550	566	592	543	--	522
Sulphate	44	88	55	4	60	--	103
Chloride	47	28	3	225	5	30	9
Hardness	125	287	526	78	506	370	398
Iron	1.78	0.10	4.71	1.18	3.65	2.0	1.95
Fluoride	--	--	0.21	0.15	0.24	--	0.19
Nitrate	<0.02	< 1	< 0.5	< 1	< 0.5	--	< 0.10
Spec. Cond.							
Aquifer Type	Lower Aquifer	Middle Aquifer	Middle Aquifer				Middle Aquifer

Determination	Bourque Lake Observ. Well 4-26-65-4-4	Private Well NW28-61-4-4	Dome (PW1-81) NE5-67-4W4	Village of Glendon (PW-1)
<u>Determination</u>				
pH	7.9	7.9	7.73	7.8
TDS	481	947	617	1005
Sodium	63	123	43	173
Potassium	4.2	9	3.4	5.7
Calcium	82	140	101	102
Magnesium	32	62	35.7	50
Carbonate	Nil	Nil	Nil	Nil
Bicarbonate	548	780	614	636
Alkalinity	449	639	503	522
Sulphate	23	194	30.5	341
Chloride	6	31	1.2	21
Hardness	338	600	399.5	460
Iron	3.48	0.4	0.06	2.72
Fluoride	0.14	0.18	0.19	0.21
Nitrate	< 0.10	< 0.45	0.003	< 0.05
Spec. Cond.		1500	913	1569
Aquifer Type	Lower Aquifer	Upper Aquifer	Middle Aquifer	Lower Aquifer

APPENDIX C

GROUND WATER MODELLING

A Report Prepared By
C. GOLD AND T. CHAU
EARTH SCIENCES DIVISION
ALBERTA ENVIRONMENT



MEMORANDUM

ENVIRONMENT

EARTH SCIENCES DIVISION

FROM L.D.M. Sadler
Director
Earth Sciences Division

OUR FILE REFERENCE

YOUR FILE REFERENCE

TO Carl Primus
Director
Planning Division

DATE 1983 08 24

TELEPHONE

SUBJECT Groundwater Study: Cold Lake - Beaver River Basin

We are pleased to submit the final report which provides the groundwater resources evaluation for the Cold Lake - Beaver River Basin. The report, entitled "A Modelling Study of Groundwater Allocation in the Cold Lake - Beaver River Basin", will be included as an appendix in the water management report prepared by your division.

Please contact our division with any questions pertaining to this study.

L.D.M. Sadler
Director
Earth Sciences Division

tsc/kc

Encl.

ABSTRACT

A mathematical modelling study was conducted in the Cold Lake-Beaver River Basin to evaluate the groundwater supply potential of two major aquifers in the Empress and Durlingville Formations. Although there is a relatively large amount of geological information available for the study area (about 14,200 km²), there is a limited hydrogeological data base. Calibration of a two dimensional, finite element aquifer flow model was carried out on each aquifer by comparing simulated piezometric heads with observed water levels for 1980. Aquifer recharge was estimated and areas of data deficiency were identified in this phase of model calibration. Recharge determined for the Durlingville Aquifer was substantially higher than the Empress Aquifer. A sensitivity study was also performed to assess the effect of parameter variations on the hydraulic head distribution in the Empress Aquifer. The model was then used to predict the hydrological impact on each aquifer with several water demand schemes. A long term feasible groundwater withdrawal scheme for potential users was developed for each formation. The Durlingville Aquifer was shown to have better groundwater allocation potential than the Empress Aquifer.

ACKNOWLEDGEMENTS

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Chapter 1

INTRODUCTION

The Planning Division of Alberta Environment, in view of the rapid growth in water demands in the Cold Lake-Beaver River Basin, has undertaken a water management study for the region. Short and long term plans are being formulated to manage the basin's surface water and groundwater resources. The main objective of the proposed management plan is to ensure adequacy of water supply to meet future demands of users in the region. To attain this objective, regional studies of the basin's surface water and groundwater supply potential are required. In recent years, the Earth Sciences Division of Alberta Environment started a program to obtain the surficial and bedrock stratigraphy of buried channels in East-Central Alberta (Yoon and Vander Pluym, 1974; Gold, 1979). The purpose of this program is to delineate a regional hydrogeologic framework on which an assessment of the basin's groundwater supply potential can be made using a mathematical model. The model, with an adequate data base for input and verification, is an useful tool to analyze the sensitivity of the aquifer system to various proposed water management alternatives. The specific management scheme which maximizes the use of the total available groundwater resources of the aquifer system will be incorporated by the river basin planners into a long term water management plan.

A finite element aquifer flow model was used to obtain quantitative assessments of the groundwater supply potential of two major aquifers (Empress and Durlingville Aquifers) in the basin. The Empress Aquifer, found in the Empress Formation, is the lowermost of the multilayered, surficial aquifer systems in the basin. Underlain by the bedrock, it is separated from the overlying Durlingville Formation by the Bronson Lake till. Previous groundwater resource evaluations using analytical or numerical models were conducted on a local scale (e.g. Baydack and Render, 1981; Hydrogeological Consultants Ltd., 1976). This report presents the first regional groundwater modelling study for the basin.

1.1 Purpose of Study

The purposes of this study are:

- (1) To synthesize and interpret available hydrogeologic data and construct the data base as necessary for model input,
- (2) To calibrate the model by comparing computed responses with observations and examining the adequacy of the data base along with calibration results,
- (3) To perform a sensitivity study of aquifer responses to model parameter variations,
- (4) To use the predictive capability of the model to identify a feasible groundwater withdrawal scheme for potential water users in each aquifer system,
- (5) To make recommendations for future study based on modelling results obtained from this investigation.

1.2 Location of Study Area

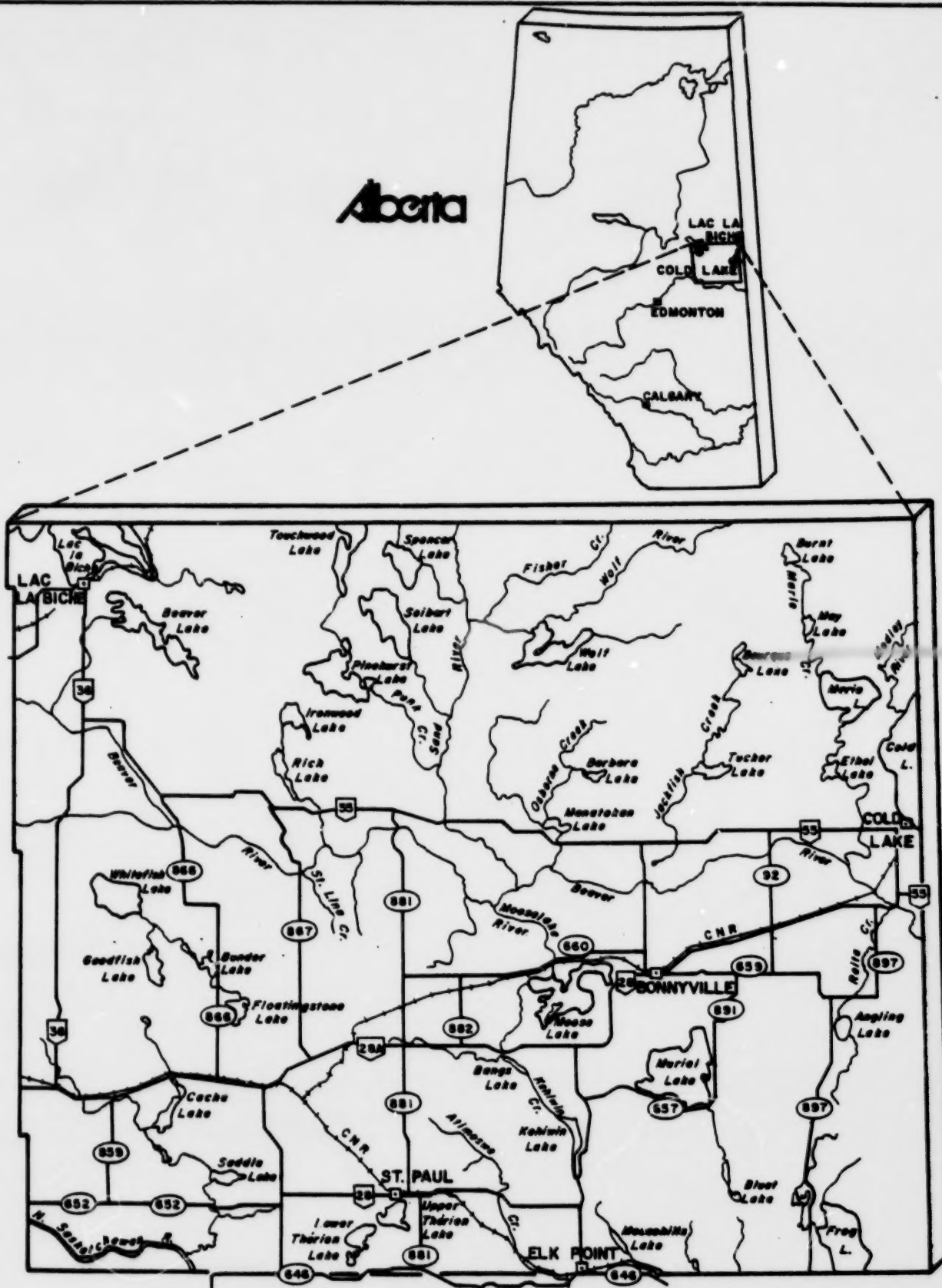
The location plan of the study area is given in figure 1.1. The study area, about 14,200 square kilometres, lies between longitudes 110° and 112° west and latitudes 54° and 55° north. It covers parts of townships 58 to 69, within ranges 1 to 14, west of the fourth meridian. The area was chosen to include present and anticipated future groundwater users in the basin.

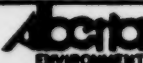
1.3 Investigational Procedure

The study was conducted in several phases:

- (1) Geological data were collected and interpreted to produce maps and cross-sections in the study area. The maps show locations of wells containing pertinent information on the surficial geology in the area - an estimated 2000 data points selected from a variety of water well industry, oil and gas industry, and other government sources. Also indicated on the maps are depths to the Cretaceous

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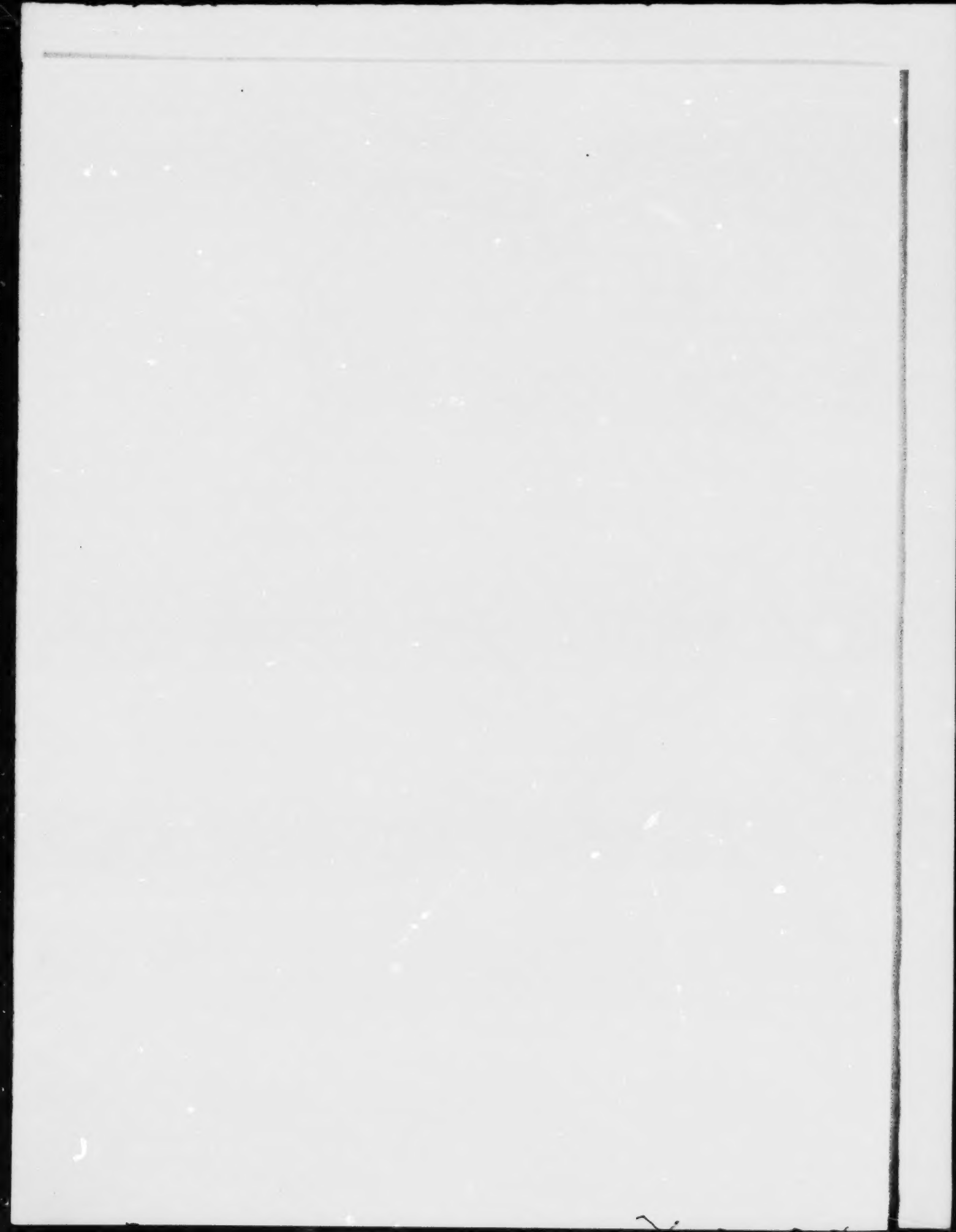
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bedrock surface and interpreted contours of the preglacial topography indicating the main channels of the preglacial or interglacial drainage system. Cross-sections were constructed along each township line and range line and appropriate well logs selected to indicate the surficial and bedrock stratigraphy. These maps and cross-sections covered much of the area from Tp 55 to Tp 74, R 1 to R 18. In addition, numerous cross-sections were constructed along and across the thalwegs of the channels in order to delineate aquifer boundaries. The maps and cross-sections provided a general picture of the areal extent of the two major aquifers and the stratigraphy of the multiaquifer systems. A separate geological report, currently under preparation by Gold, will contain the maps and cross-sections for the study area.

- (2) Through the Groundwater Resources Information Service of the Hydrogeology Branch, a computer search was conducted to locate wells within the study area. Wells tapping the Empress and Durlingville Aquifers were identified and the reliability of static water levels at these wells was assessed. More detailed and site specific hydrogeologic data for pumping centers were obtained from the Ground Water Rights Branch. Information on hydraulic properties, water level history at pumping and observation wells, and magnitude of pumpage at these locations was collected and interpreted. Additional observation wells were installed by the Earth Sciences Division at key locations in the basin in an attempt to clarify general flow direction in the aquifer system. Aquifer tests were also conducted with these wells to determine hydraulic parameters.
- (3) Having completed the data identification phase, the data base for model input was constructed from available hydrogeologic data. This required specification of aquifer properties, aquifer stresses and boundary conditions over the finite element mesh which represents the aquifer domain. Model application started with calibration of the flow model, a process by which different model

input parameters were verified or determined. It involved choosing a period in the past (1980) for which data were available on the excitation (pumpage and recharge) and response (water level) behaviour of the aquifer. With this excitation as input, the model response for this period was compared with water levels recorded at an observation well.

- (4) Having established confidence in model input parameters during the calibration phase, the model was used to predict the steady state response of the aquifer subject to various groundwater withdrawal schemes. A feasible withdrawal scheme was identified for each individual aquifer.



Chapter 2

GEOLOGICAL SETTING OF THE STUDY AREA

2.1 Bedrock Geology

The Western Canada sedimentary basin is a gently dipping trough between the Cambrian Shield in the northeast of Alberta and mountain ranges to the southwest. This basin has been a shallow marine sea in two geological periods: the Devonian and the Cretaceous. At the start of Cretaceous time, a shallow sea existed between the Canadian Shield and the central Cordilleran mountains. The general history of Cretaceous deposition is the history of advancing and retreating marine episodes. These are preserved in the existing rock types. A shale formation is the result of a marine advance and the subsequent deposition of fine grained clastic materials (mostly clays) at some distance from the sediment source. The various sandstone wedges which separate these shales throughout much of the province are the result of marine retreat and the development of extensive deltaic deposits along the shorelines of the time. These sand wedges extend outward into the centre of the basin, their extent depending on the extent of marine regression and the amount of available sand size sediment from the high ground to the southwest and, to a lesser extent, to the northeast. The main geological map units of the Cretaceous period found within the study area are:

- (1) The Lower Mannville, which is primarily a continental sandstone immediately overlying Devonian rocks that had been eroded extensively during a long time period.
- (2) The Upper Mannville, which represents a time of marine transgression over the non-marine beds below. It is somewhat mixed in composition and not simple in structure as it consists of both marine shales and non-marine sandstones. In northern and central Alberta, it becomes more shaly in composition.

- (3) The Lower Colorado, which represents a time of complete marine inundation and is almost entirely composed of shale. Within the Lower Colorado unit the Joli Fou Formation and the Viking Formation are found. The Joli Fou is a thin marine shale. The Viking is a thin fine grained sandstone.
- (4) The Upper Colorado Group, also consisting of relatively uniform dark gray shale. Within it are found the Fish Scales marker and also the First and Second White Specks marker horizons.
- (5) The post-Colorado formations in Alberta, which can be simplified to consist of the Lea Park shale followed by the Belly River sandstone and then the Bearpaw shale. Overlying the Bearpaw shale is the Horseshoe Canyon Formation, again primarily sandstone.

Within the study area, the Cretaceous bedrock immediately underlying the surficial deposits is primarily the Lea Park shale. In the extreme southwest, however, the Belly River sandstone may also occur, overlying the Lea Park shale. All Cretaceous formations in this area dip gently to the southwest.

Following the retreat of the last Cretaceous sea in Alberta, the Bearpaw sea, deposition of the continental sandstones of the Cretaceous Horseshoe Canyon and the Tertiary Paskapoo, all of which are to the southwest of the study area, was followed by a long period of continental erosion in which many of the uppermost Cretaceous sediments were removed. In northern Alberta, the combined Belly River sandstone and Horseshoe Canyon Formation are termed the Wapiti Formation as the intermediate Bearpaw shale is absent. It is probable that the Belly River or Wapiti Formation at one time existed throughout the study area, but they have subsequently been removed by erosion, exposing the underlying Lea Park shale.

2.2 Bedrock Topography and Buried Valleys

Gold et al. (1983) prepared a bedrock topography map for the study area. A similar map (in pocket) indicates several channels incised into the uppermost Cretaceous Lea Park shale. The largest of these, the Helina Valley, extends westward from Saskatchewan through Cold Lake and Marie Lake, swings southwest near Marguerite Lake, passes beneath Manatokan Lake and then swings to the northwest towards Lac La Biche. It is interpreted that this valley formed part of a southeast collector for drainage emanating from the Rockies to the southwest. The second major channel, the Beverly, extends from Marguerite Lake south to Moose Lake and southwest and west through Vincent Lake and Cache Lake in the southwest of the study area. This valley once formed the channel of the preglacial North Saskatchewan River. It is thought that several rivers of this type, including the Athabasca and others, drained radially out from the relatively newly formed Front Ranges of the Rocky Mountains and were funnelled into the southeast-trending system represented in this area by the Helina Channel. Two tributaries extend northwards from the Helina Channel within the study area. One of these, the Sinclair Valley extends north from Marguerite Lake into the Cold Lake Air Weapons Range. In immediate preglacial time it appears to have been a tributary draining south into the Helina system. In the northwest of the study area, the Imperial Mills Valley appears to have at some preglacial time formed a link with the Helina Valley system and with the Wiau Valley in the northern part of the Air Weapons Range. This Wiau valley is several times larger than the Helina Valley and appears to be approximately 25 kilometres wide, 45 metres in depth and primarily filled with very well sorted fine sand. It is overlain by 150 to 250 metres of glacial deposits. It is a major groundwater resource but does not appear to be hydraulically connected with the preglacial channels present within the study area.

In addition to the channels previously mentioned in the southwest of the area, the Kikino Valley joins the Beverly Valley from the northwest. It is also thought to be of preglacial origin and

connects with the Helena Valley to the west of the study area. The Vermilion Valley, running north along the Alberta/Saskatchewan border, is also thought to be preglacial. Several valleys, presumed to be of glacial origin, also exist within the study area. These are the Holyoke Valley, the Bronson Lake Valley, the Big Meadow Lake Valley, the Vegreville Valley and the St. Paul Valley. The only one of these valleys of major importance for groundwater resources is the Bronson Lake Valley which extends south-southeast from the Beverly Valley to the north of Moose Lake.

2.3 Surficial Stratigraphy and Hydrostratigraphy

The preglacial erosion represented by the valley systems previously described was terminated by the onset of glaciation in the Pleistocene Era. Several ice sheets advanced and retreated through the area, eroding underlying deposits and depositing new till layers. Between glaciations, drainage attempted to resume its interrupted course. On the high ground between the channels only the most recent till deposits seem to be readily identifiable. Within the valleys themselves, several till layers have been identified. In general terms, therefore, surficial aquifers should be expected between or below major till sequences and not within them. While it is certainly true that local sand lenses may well be found within particular till units, it is only when sub-aerial river drainage and erosion take place that the well sorted sands and gravels required for groundwater aquifers are of any regional importance. The till and inter-till stratigraphic units were first described in Gold (1979). These descriptions have been refined and given formal formation names by Andriashek and Fenton (in preparation). The lowermost surficial aquifer is found in the Empress Formation. It consists primarily of preglacial sands and gravels within the confines of the preglacial valley floors. In the eastern part of the study area (as far west as Range 5) this formation may be divided into the Upper and Lower Empress, based on the presence of a thin clay

layer separating the two. Further west this clay layer is absent and the Empress Formation is considered to be a single entity. The Empress Formation is found more or less continuously throughout the Beverly Valley, the Helina Valley and the northwest portion of the Bronson Lake Valley. It also appears to exist in the southern part of the Sinclair Valley and probably also in the Imperial Mills Valley, although these two tributaries are less well defined because of the absence of drilling information in their northern parts.

Overlying the Empress Formation is the Bronson Lake till. On the basis of drill hole records, this till has not been identified continuously throughout the study area. Above this is the next interglacial deposit, the Durlingville. This again is confined to the Beverly and Helina Valleys as well as parts of the Imperial Mills, Sinclair and Bronson Lake Valleys. In particular, it appears to have larger groundwater potential than the Empress Formation since by this time the preglacial valleys had been partially filled and the resulting valley floor was shallower and wider. The Durlingville Formation is therefore the most important groundwater resource within the study area. In general confined to the valleys, it has however, extended in some places across some interfluves (for example, between the Helina and Beverly valleys in the Moose Lake area), although in these cases it is relatively thin and may be insignificant from a hydrogeological point of view.

Overlying the Durlingville is the Bonnyville Formation. Above this are found, in some parts of the study area, the Ethel Lake clays and sands, the overlying Medley till, then the Beaver River sands and finally the till of the Grand Centre Formation. Inter-till aquifers of either Ethel Lake or Beaver River origin are present in various parts of the study area but are not usually of great lateral extent. They are however of particular interest in the Sinclair Valley area partly because of their thickness and partly because of their industrial potential. It is unfortunate that little is known about the lateral

extent of these sands within the Sinclair Valley since the bulk of the valley falls within the Cold Lake Air Weapons Range and very little information is available.

In summary, the main groundwater aquifers are found within predominantly preglacial valleys incised into the underlying Cretaceous shales. While four preglacial or interglacial sand and gravel aquifers are of some interest, only two, the Empress and the Durlingville Aquifers are of major regional importance. On the basis of work done by the Earth Sciences Division, as well as by the Alberta Geological Survey, the greatest emphasis was therefore placed on the delineation of the Empress and Durlingville Formations and their evaluation for groundwater supply potential.

Chapter 3

DESCRIPTION OF THE NUMERICAL MODEL

The geological setting of the study area indicates complex aquifer geometry and hydrogeologic processes to be modeled. A finite element aquifer flow model, which has flexibility in representing the complicated aquifer domain and in handling various boundary conditions acting on the aquifer system, was chosen for this study. This aquifer computer code, AQUIFEM-1 (AQUIfer Finite Element Model-1-layer), was developed by Townley and Wilson (1980) at the Ralph M. Parsons Laboratory for Water Resources and Hydrodynamics, Massachusetts Institute of Technology. The model solves the equation describing two-dimensional, horizontal groundwater flow in a non-homogenous anisotropic aquifer with leakage. AQUIFEM-1 is formulated using the Galerkin finite element technique and uses linear interpolation functions with triangular elements. Steady state or transient solutions for confined, phreatic and changing status (confined to phreatic or vice versa) aquifers can be computed. Numerous boundary conditions such as recharge and pumping wells, lateral inflows and outflows, leakage from an adjacent aquifer and the exchange of water between the aquifer and fully or partially penetrating surface water bodies can be included. This practically oriented computer code was verified with analytical solutions and has been applied by the original developers to solve numerous flow problems in the subsurface environment. The mathematical formulation of the model is outlined in Appendix A which contains the following information: governing equation and boundary conditions, Galerkin finite element formulation and solution techniques for solving the system of matrix equations. Both steady state and transient responses of the Empress and Durlingville Aquifers were studied with this numerical model.

At the beginning of this study, consideration was given to modelling the surficial aquifers in the study area with either a three dimensional (3-D) flow model or a multilayered flow model. The two dimensional flow model described previously was used instead because of economic and time constraints on this study and the inadequate hydrogeologic data base to define the 3-D system and to calibrate the model. A more detailed discussion on the feasibility of 3-D modelling is presented in Appendix B.

Chapter 4

AVAILABLE DATA

4.1 Aquifer Stresses

Stresses acting on an aquifer result primarily from groundwater withdrawals and recharge. To simulate the impact of pumpage during the simulation period, groundwater users in each aquifer system have to be identified for that period. Suncor Inc. Resources Group (formerly known as Worldwide Energy Company Ltd.) and Amoco Canada Petroleum Energy Ltd. were users of the Empress Aquifer. Union Texas of Canada Ltd., Municipal District of Bonnyville and Norcen Energy Resources Ltd. obtained groundwater from the Durlingville Aquifer. Pumping records submitted by these users to the Ground Water Rights Branch of Alberta Environment were provided as weekly or monthly production volumes. Based on these records, average monthly pumping rates were calculated for 1980 at each groundwater withdrawal center. When there were several pumping wells in a pumping center, their withdrawals were represented as a cumulative nodal withdrawal. Figure 4.1 presents simplified pumping records of water users for each aquifer. Each step function is considered as an estimate of the actual pumpage at the withdrawal center during 1980.

Groundwater recharge to the Empress and Durlingville Aquifers is a difficult parameter to evaluate in the hydrogeologic system. This parameter, which is a function of many physical and climatological factors, can vary in time and space. For shallow aquifers, a mass balance analysis may provide an estimate of the net recharge component. For deep confined aquifers, like the Empress and Durlingville which are several hundred feet below the ground surface, the vertical flux through the till layer into the aquifer can be estimated from Darcy's Law. To compute this flux, the till hydraulic conductivity and the hydraulic

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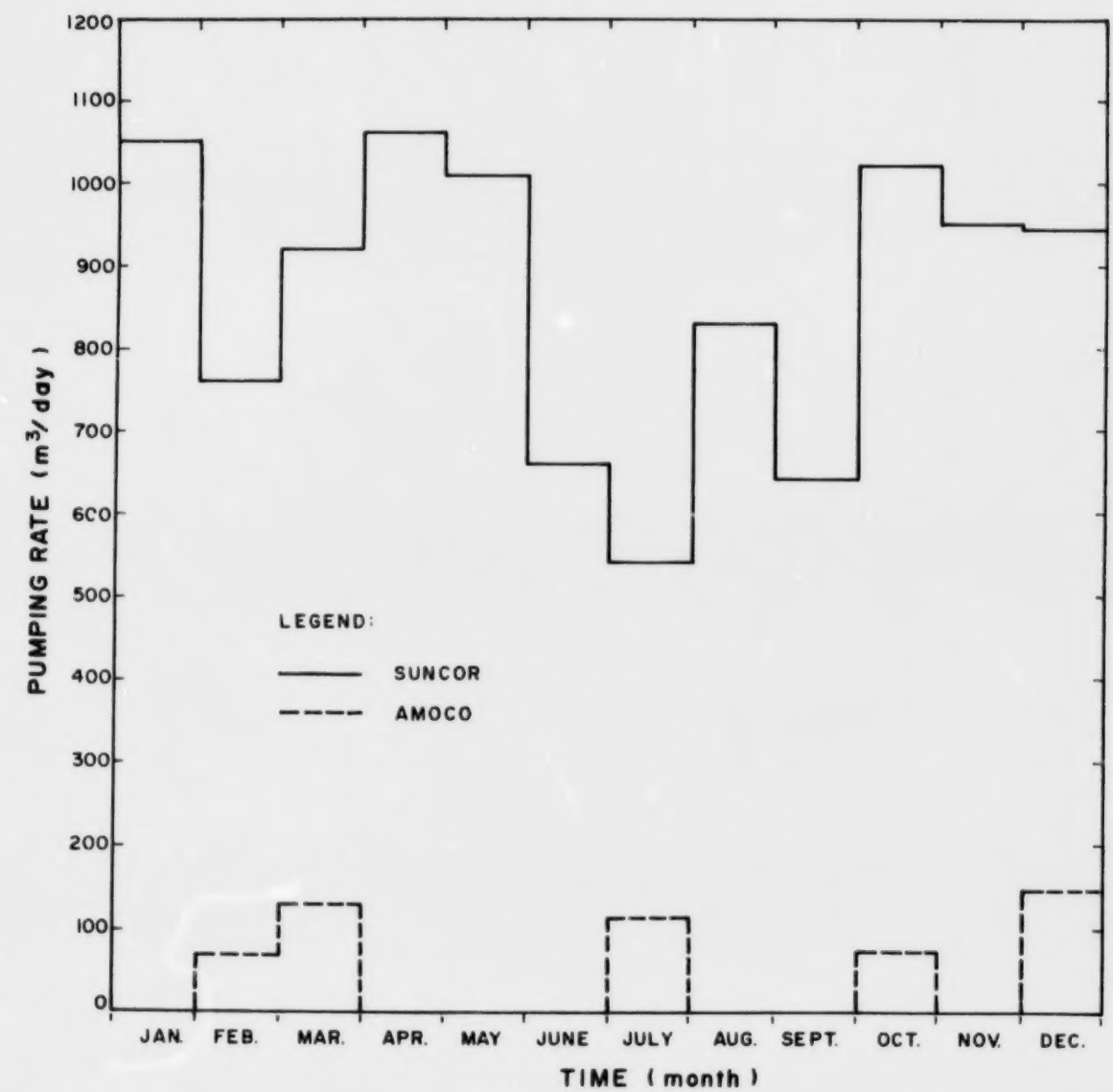
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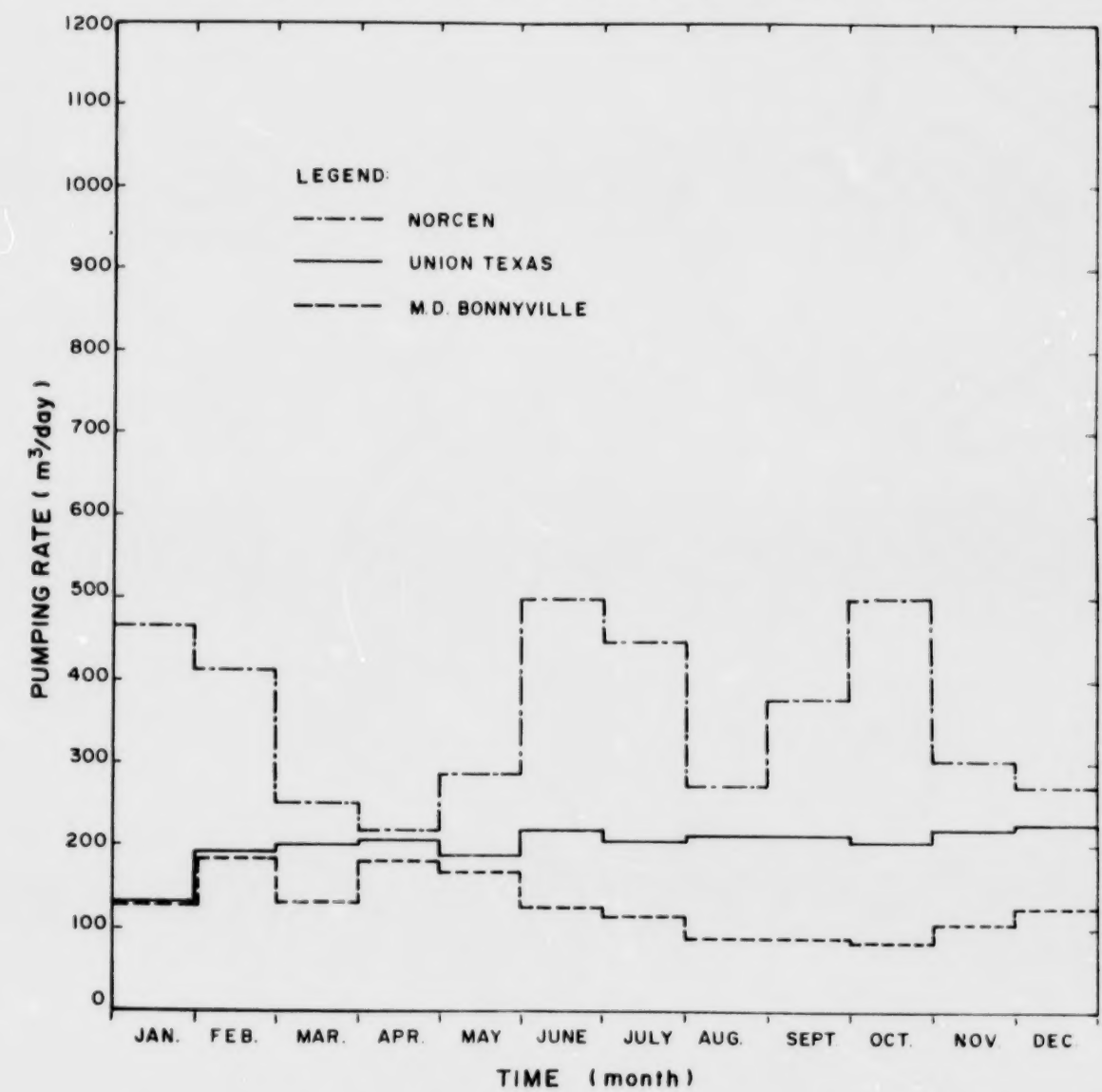
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MONTHLY WITHDRAWALS IN 1980-EMPRESS AQUIFER



MONTHLY WITHDRAWALS IN 1980-DURLINGVILLE AQUIFER



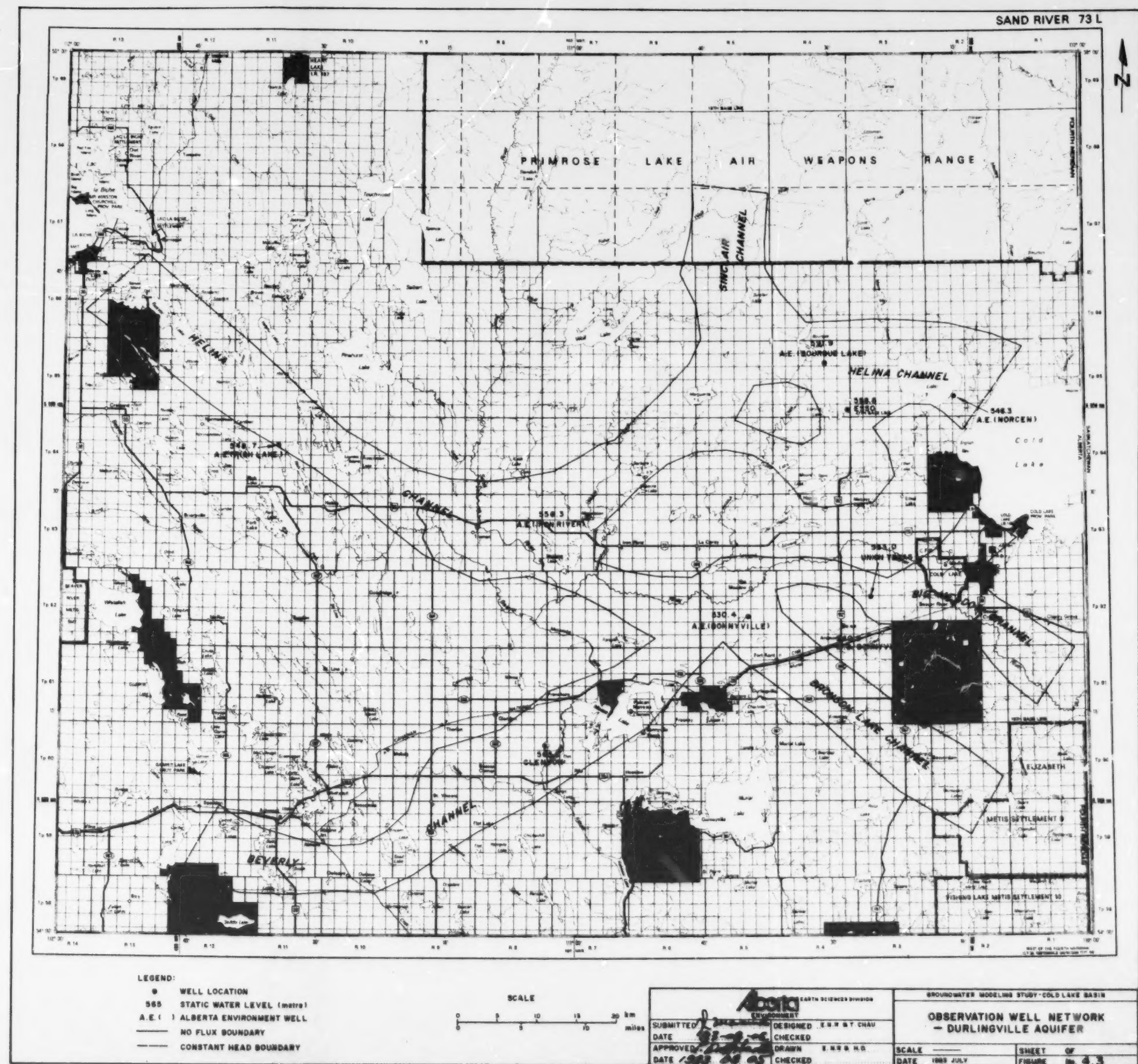
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gradient across the till unit must be known. The hydraulic gradient is difficult to determine on a basin wide scale because of the limited information on piezometric heads. Also, the range in hydraulic conductivity can be quite significant. Analysis performed by the Groundwater Branch of the Alberta Research Council on data in the study area showed that the hydraulic conductivity of the till unit overlying the Empress Formation can vary between 4.3×10^{-8} m/sec and 2.4×10^{-7} m/sec. In this study, the net recharge rate to each aquifer was estimated through a trial-and-error approach during calibration of the model. With several uniformly distributed recharge rates as input, the simulation model was used to compute a steady state hydraulic head distribution using boundary conditions acting at the beginning of the simulation period. The recharge rate that gave the best match between observed and computed piezometric heads was considered to be the best estimate of the recharge component. This recharge rate was used in the subsequent transient simulation and in the prediction of aquifer responses to various groundwater withdrawal scenarios.

4.2 Observation Well Network

A network of observation wells is necessary to define the piezometric head distribution and hence the regional flow direction in an aquifer system. These observed piezometric heads also form the basis of comparison with the hydraulic heads computed by the model. Figures 4.2 and 4.3 summarize, respectively, the static water level (above mean sea level) of observation wells in the Empress and Durlingville Aquifers. In order to obtain accurate water levels, surveying has been completed by the Survey Branch to obtain ground elevations of all Alberta Environment wells and some industrial wells in both aquifers. Table 4.1 provides the location of these wells for both aquifers. These wells will be constantly referred to by their names given in this table for the rest of the report. Continuous water level records are available for the A.E. (Bourque Lake) wells since March, 1980 and for the A.E. (Bonnyville) wells since August, 1977. The rest





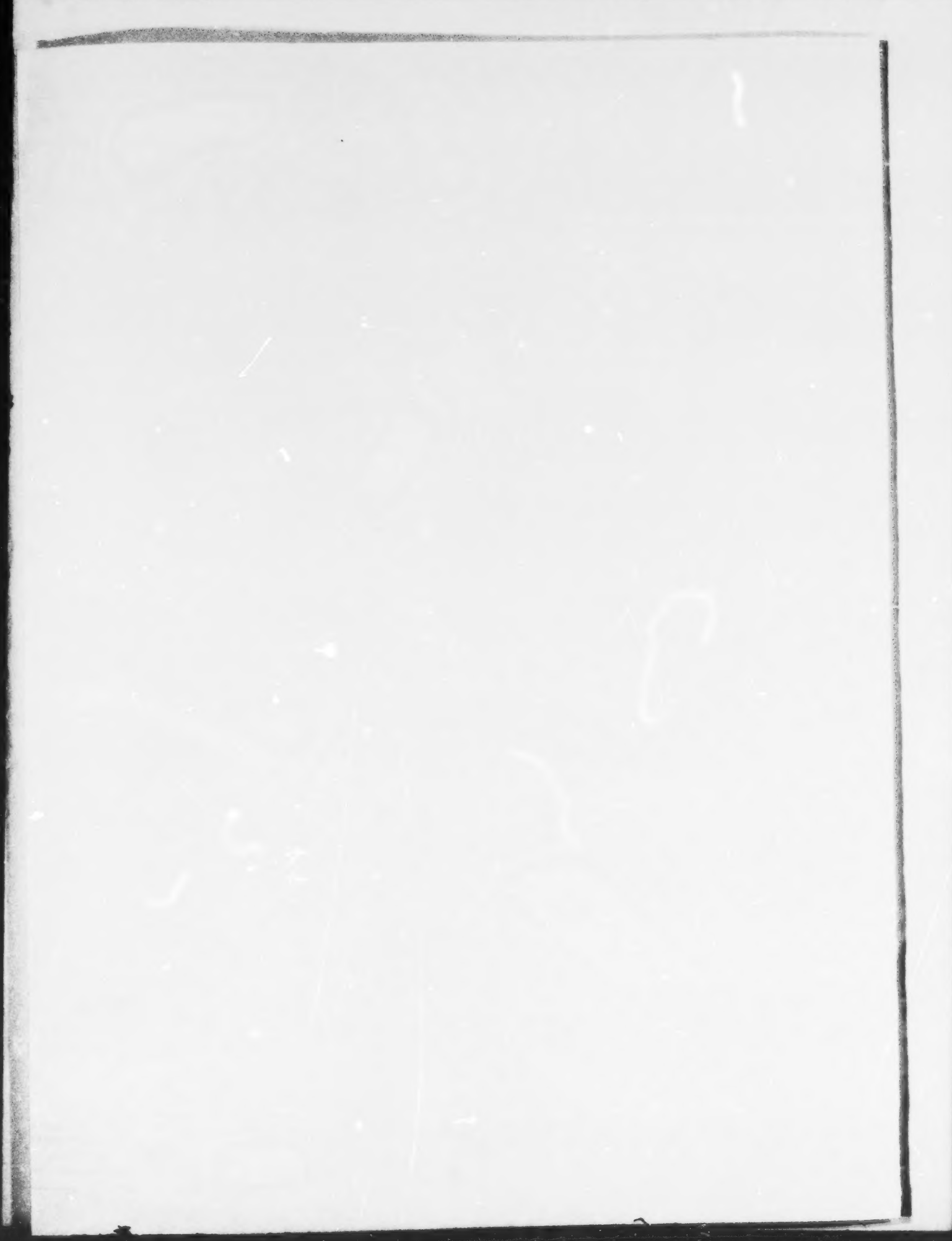


TABLE 4.1

Location of Wells in Empress and Durlingville Aquifers*

Well name	Well location	Empress Aquifer	Durlingville Aquifer
A.E. (Bonnyville)	NE-15-62-5-W4	P	P
A.E. (Bourque Lake)	SW-26-65-4-W4	P	P
A.E. (BP)	SW-28-66-5-W4		
A.E. (Iron River)	SW-27-63-7-W4	P	P
A.E. (La Corey)	NW-25-63-5-W4	P	
A.E. (Norcen)	SW-9-65-2-W4	P	P
A.E. (Rich Lake)	SW-27-64-11-W4	P	P
Amoco	SW-4-63-8-W4	P	
Belvedere Development	SW-19-62-5-W4	P	
Canada Cities	SW-13-63-8-W4	P	
Esso	NE-1-65-4-W4		P
Glendon	SE-26-60-8-W4	P	
M.D. Bonnyville	SE-32-61-4-W4		P
Suncor	SW-28-61-4-W4	P	
Union Texas	NW-20-62-3-W4		P

* For all A.E. wells, P implies the completion of well in that particular aquifer. For the rest of the wells, P also means potential groundwater use from the aquifer.

of the Environment wells were constructed during 1981 and 1982, in an attempt to establish the flow pattern and to obtain more data on aquifer properties. No recorders were installed at that time to monitor water level changes. In the near future, recorders will be installed in these wells so that the impact of basin wide groundwater development can be measured on a continuous basis. In this study, the A.E. (Bonnyville) well, which had continuous water level readings during 1980, was used to calibrate the model for both aquifers. In addition to production wells in a pumping center, there were associated observation wells for monitoring of water level changes during the simulation period. The measured responses at these observation wells were not used for model calibration because the finite element grid was primarily designed for regional application. A very fine mesh is required in the proximity of the production wells in order to generate the transient responses for comparison with those observed at these observation wells.

The piezometric heads given in figures 4.2 and 4.3 were constructed from static water levels measured at different times (several years apart between two measurements in some cases). Hence, the piezometric heads supplied in both figures should not be taken as a snapshot of the aquifer response at some point in time. These heads, in effect, could reflect the changes in hydrogeologic events in the basin over a period of many years. These changes can be significant for a basin undergoing intense groundwater development. This time factor should not be overlooked if these maps are used to infer aquifer flow patterns.

Based on the piezometric heads in figure 4.2, several characteristics are apparent for the Empress Aquifer. First, high heads were measured in Alberta Environment wells near Bourque Lake and La Corey. Secondly, observed heads near La Corey and Belvedere Development suggested the existence of a sharp hydraulic gradient between these two locations (about 50 metres head difference over 15 kilometres). Thirdly, along the Helina Channel, heads measured at Canada Cities and Amoco were lower than that recorded by the Alberta Environment Wells

(A.E. (Iron River) and A.E. (Rich Lake)). This appears to indicate a convergence in flow towards the area where Amoco and Canada Cities are located. If that is the case, there has to be a source of outflow for these incoming flows around that area in order for the flow dynamic to be physically reasonable. To sum up, existing piezometric heads do not provide a consistent and clear picture of the flow distribution in the Empress Aquifer system.

Figure 4.3 is used to analyze flow patterns in the Durlingville Aquifer. High heads were observed at the A.E. (Bourque Lake) Well and the Esso Resources Well. This may indicate a dominant flow coming from the north through the Sinclair Channel. This is then divided into two components at the channels confluence, one component flowing along the Helina Channel towards Lac La Biche and another component directed towards Cold Lake. Along the Bronson Lake Channel, static water levels show that flow is from the east towards the junction of three channels (Beverly, Bronson Lake and Big Meadow). Comparing the static water level of Union Texas Well to the slightly higher lake level of Cold Lake seems to indicate outflow from the lake through the tributary connected to the Big Meadow Channel. The flow in this tributary is divided into an east and west component along the Big Meadow Channel. Based on piezometric heads recorded at wells near Glendon and another well (6-27-59-12-W4), flow along Beverly Channel is from the west towards the east. Previous discussions illustrate flow towards the junction of the three channels. This may suggest possible hydraulic connection between the aquifer and the Beaver River which might serve as a discharge zone for the aquifer.

4.3 Aquifer Properties

Aquifer properties (transmissivity and storage coefficient) have to be determined for the aquifer modelling domain. The transmissivity is defined as the rate of flow per unit width through the entire depth of the aquifer under a unit hydraulic gradient. The

storage coefficient provides the relationship between changes in the amount of water stored in the aquifer and changes in head. Under the steady state situation ($\frac{\partial h}{\partial t} = 0$), the storage coefficient is not required in the computation of the steady head distribution. It is needed only in transient analysis. Pumping tests, a technique for the identification of aquifer properties, were conducted at different locations in the river basin. A summary of aquifer parameters obtained by pumping tests was contained in the report prepared by MLM Groundwater Engineering (1982). Based on their results, logs and lithology data, transmissivities were estimated for the aquifer domain. Since some of the pumping tests were performed with only a source well, the storage coefficient could not be estimated by conventional pumping test techniques. Hence, the data base for the storage coefficient over the modelling region was very limited. Using a range of values for the storage coefficient in the transient simulation, the storage coefficients that gave the best match between the model and the observed response were assigned to the aquifer system.

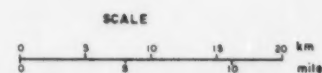
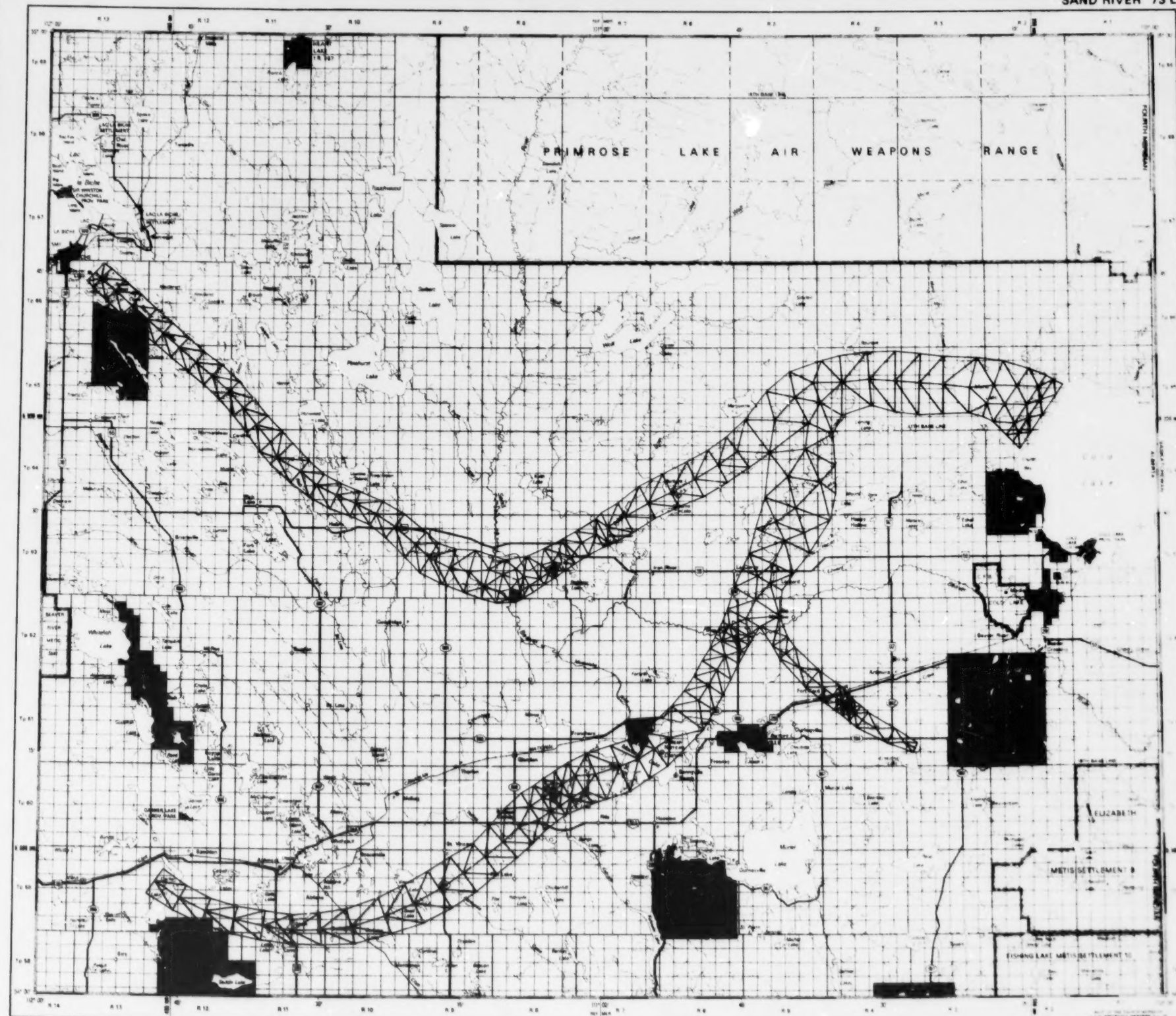
Chapter 5

APPLICATION OF MODEL TO EMPRESS AQUIFER

5.1 Aquifer Description and Specification of Boundary Conditions

The triangular grid which represents the modelling region is presented in figure 5.1. The grid was discretized into 545 elements (triangles) with 380 nodes. Closely spaced nodes were placed around potential groundwater withdrawal centers because of the rapid change in piezometric heads around them. These pumping centers include Amoco Canada Petroleum Energy Ltd., Canada Cities Services Ltd., Belvedere Development Ltd., Suncor Inc. Resources Group and Village of Glendon. Despite the large number of elements used, the finite element mesh was only designed for regional application. For the finest portion of the mesh, distance between adjacent nodes is about 300 metres. Most pumping centers have several pumping wells and associated observation wells. Because of the regional scale considered in this study, the cluster of wells in a pumping center could be adequately represented by a single node. When localized modelling is required in the future and there is insufficient data to define the fine mesh boundary conditions, the boundary nodal points of the fine mesh can be designed to merge with existing nodes of the regional mesh. In this manner, the prescribed boundary heads for the fine mesh are given by the computed heads of the regional mesh.

No flux or constant head boundary conditions were specified along the boundaries of the Empress Aquifer. If the recharge and the underflow into or out of the modelling boundary were sufficient to maintain a nearly constant piezometric head, the constant head assumption was introduced along that boundary. The no flux boundary condition was applied when the flow direction was essentially parallel to that boundary, implying that the flux through the channel wall was negligible. In figure 4.2, dotted lines indicate constant head



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SCALE DATE 1988 JULY		SHEET OF FIGURE No. 5.1	



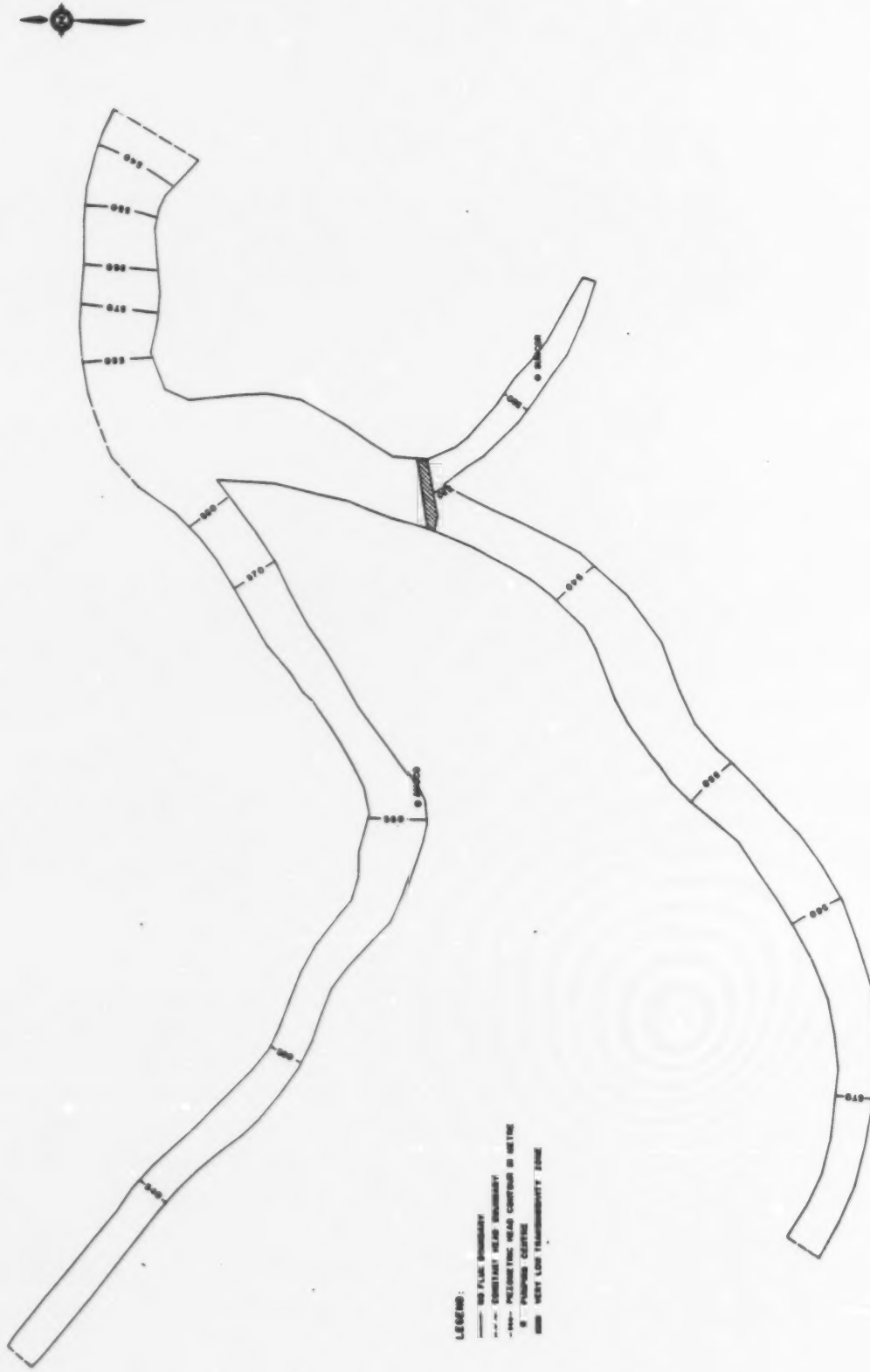
boundaries and no flow boundaries are represented by solid lines. Surficial stratigraphy along the Bronson Lake Channel indicated that the aquifer gradually pinched out. A no flux boundary condition was prescribed at the end of this channel. The water level of Cold Lake and static water level of a well near Vincent Lake provided, respectively, the constant head boundary conditions for the east end of the Helina Channel and west end of the Beverly Channel. Along the confluence of three channels (Sinclair, Helina and Beverly), a constant head boundary was used. There was an insufficient data base to understand the geology of the confluence region in detail. The continuity of the Empress Aquifer into the Sinclair Channel was of limited extent based on the available geological data. The high piezometric heads recorded by the Alberta Environment wells near Bourque Lake and La Corey seem to suggest a dominant flow arriving from the north through the Sinclair Channel. This main stream is divided at the channel junction into several flow components. One component flows east through the Helina Channel into Cold Lake. Other components flow south through the Beverly Channel towards La Corey and west through the Helina Channel towards Lac La Biche. The static water levels observed in wells near Vincent Lake, Glendon and Belvedere suggest that flow along this reach of the Beverly Channel is from Vincent Lake towards Belvedere. To explain the observed sharp drop in heads between La Corey and Belvedere, the geological setting of this area was subsequently re-interpreted. This area suggests a discontinuity which is similar to the presence of a geologic fault in the Beverly Channel. Horizontal flow across this fault is prohibited. To simulate this geologic discontinuity with the model, very low transmissivity values were assigned to the elements covering this discontinuous zone. The presence of this discontinuity implies two independent flow regimes within the Empress Aquifer system.

The northern limit of the modelling boundary was limited by the southern boundary of the Cold Lake Air Weapons Range, an establishment

of the Department of National Defense. The continuity of the Sinclair Channel could not be established north of Township 66 because hydrogeologic information was not available for the area. Within the Sinclair reach considered for this study, observation wells owned by Alberta Environment and BP Exploration Canada Ltd. were available to define piezometric heads only in the surficial aquifer overlying the Durlingville Aquifer. To define the unknown constant head boundary condition along the channels confluence, several boundary heads were used for model input. By this trial and error procedure, the boundary head selected was the one that best simulated the observed response of the aquifer system.

5.2 Calibration of Model

A spatial hydraulic head distribution at the start of the simulation period was required to carry out the transient analysis. It was originally intended to construct this initial head distribution from the observation well network in the Empress Aquifer. The limited observation wells were insufficient to provide a general picture of the flow direction. It would be even more difficult to generate the initial hydraulic heads at all nodal points of the aquifer mesh. This initial condition was obtained by assuming a steady state condition prevailed at the start of the simulation period. Using equation A.14 given in Appendix A, a steady state solution was computed with the boundary conditions and stresses acting at the beginning of the simulation period. The computed steady state hydraulic head distribution, serving as an initial condition for the transient simulation, is presented in figure 5.2. The observed static water levels in figure 4.2 form the basis of comparison with the computed steady state piezometric distribution at the start of 1980. Since the observed piezometric heads were based on static water levels measured at different times, they might not necessarily represent actual water levels at the beginning of 1980. This factor should be considered when making the comparison.



LEGEND:

- AQUIFER BOUNDARY
- - - CONTACT HEAD BOUNDARY
- - - - - PIEZOMETRIC HEAD CONTOUR IN METRE
- PUMPING CENTRE
- VERY LOW TRANSMISSIVITY ZONE



Abana WATER RESOURCES DIVISION SUBMITTED: <i>[Signature]</i> DATE: 21-10-80 APPROVED: <i>[Signature]</i> DATE: 19-10-80		DESIGNER: T CHAU CHECKED: <i>[Signature]</i> DRAWN: <i>[Signature]</i> CHECKED: <i>[Signature]</i>	
ORGANIZED MODELING STUDY, COLD LAKE BASIN PIEZOMETRIC MAP OF EMPRESS AQUIFER AT THE BEGINNING OF 1980			
SCALE AS SHOWN DATE: JULY 1983		1:10,000 FIGURE No. 9.3	

Table 5.1 provides a comparison between the observed and computed heads at specific locations. There are large discrepancies in heads (about 30 metres) at the Amoco and Canada Cities locations. Model simulation gave rise to unidirectional flow in that reach of the Helina Channel. This contradicts the inference drawn from the observed heads which indicate a convergence in flow towards the Amoco and Canada Cities locations. During 1980, Amoco was the only groundwater withdrawal location and the production rates were by no means high enough to create such an observed flow pattern. There had to be another means of outflow in order to induce flow from both ends of the channel. Because of the limited data base for that part of the Helina Channel, the geology of many locations was unknown. This leaves the possible outflow locations open to speculation. Perhaps there is hydraulic connection between the Sand River and the Helina Channel. The bedrock formation for some part of the channel may not be as impervious as assumed for this study. Hence, it can act as a discharge zone for the Empress Aquifer. The same applies to possible lateral outflow from the valley wall. Refined geological interpretation for this reach is necessary as more data becomes available in the future. The model will then be modified to include this new boundary condition. On the whole, there is reasonable agreement between observed and computed heads at most locations in the Beverly and the Bronson Lake Channels. The discrepancy in heads at A.E. (Bourque Lake) and A.E. (Norcen) wells was due to the lack of information to specify boundary condition at the confluence of three channels.

Transient aquifer response for the simulation period was calculated with the steady state computed head distribution as the initial condition, using equations A.15 to A.17 in Appendix A. Water levels observed in the Alberta Environment well near Bonnyville during 1980 were compared with the computed hydraulic heads. These results are shown in figure 5.3. It appears that there is a time lag between the observed and simulated response curves. From May onward, the shape

TABLE 5.1

Observed and Computed Steady State Heads
in the Empress Aquifer

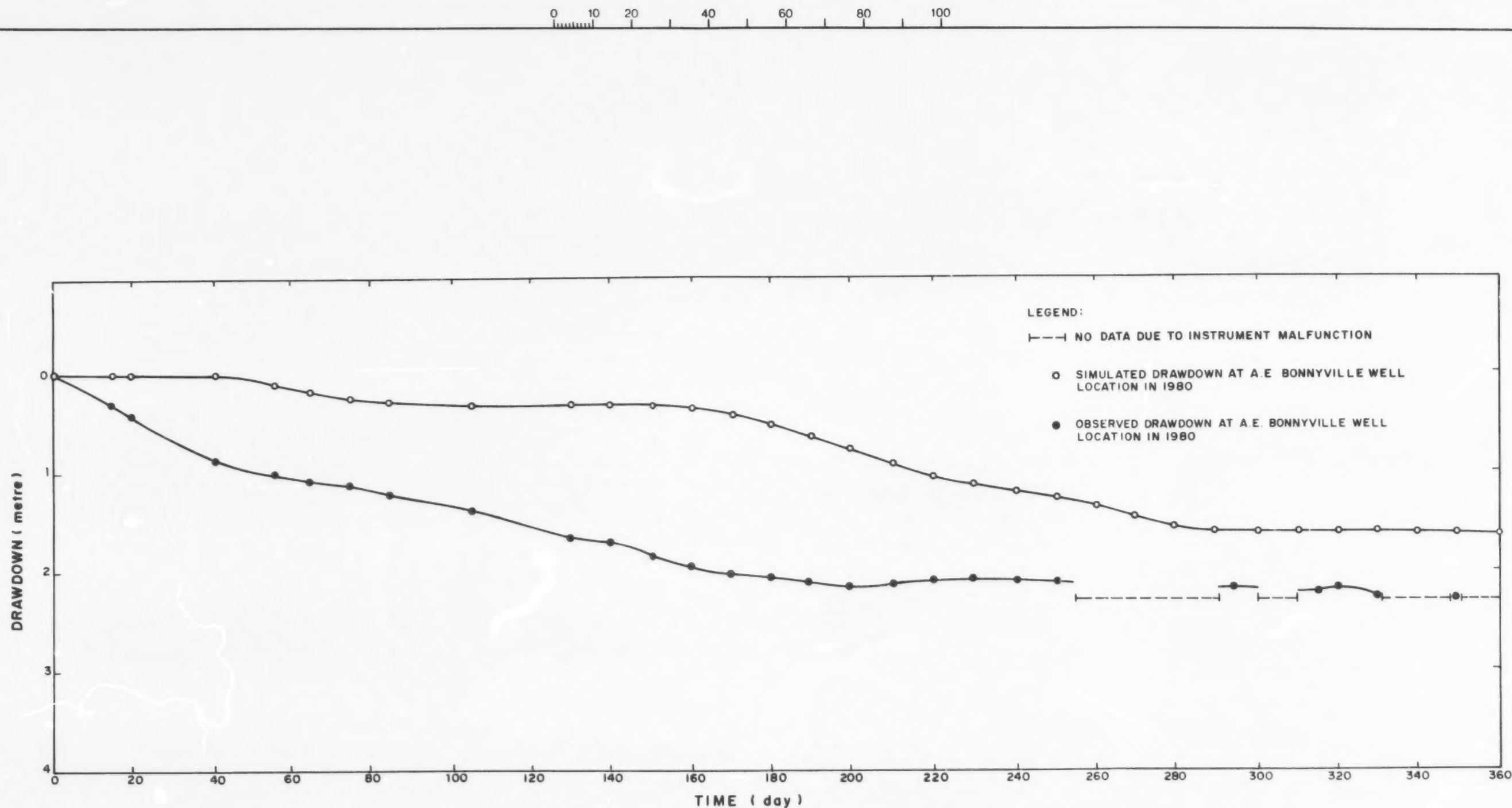
Well location	Observed head (m)	Computed head (m)
A.E. (Bonnyville)	526.0	524.0
A.E. (Bourque Lake)	585.9	577.7
A.E. (Iron River)	554.5	565.9
A.E. (La Corey)	587.0	586.6
A.E. (Norcen)	546.2	538.6
A.E. (Rich Lake)	544.7	546.9
Amoco	533.5	560.7
Belvedere	533.0	531.5
Canada Cities	532.0	562.7
Glendon	543.5	547.6
Suncor	526.2	518.0

MICROFILM DATE

OE

DRAWING FILE No

CENTRAL FILE No



Alberta EARTH SCIENCES DIVISION ENVIRONMENT		GROUNDWATER MODELING STUDY-COLD LAKE BASIN	
SUBMITTED <i>[Signature]</i>		DESIGNED T. CHAU	
DATE 83-08-15		CHECKED	
APPROVED <i>[Signature]</i>		DRAWN H.O.	
DATE 1983 08 05		CHECKED	
SCALE		SHEET OF	
DATE 1983 JUNE		FIGURE No. 5.3	

of the simulated response curve resembles quite closely the observed response curve starting from February. This delayed response might arise from the use of a steady state solution as the initial condition in the simulation. It takes a longer time to start an aquifer system from a steady state as compared to a system that is undergoing dynamic changes. The entire aquifer system at the time might not necessarily be in a steady state, particularly for the Bronson Lake Channel which was having constant groundwater withdrawal by Suncor. In fact, the steady decline in water level as indicated by the Environment and Suncor observation wells prior to 1980 suggested that a steady state condition was not attainable for this channel. However, a steady state representation might be applicable to other parts of the aquifer system. Because there is no better means to define the initial head distribution, the steady state solution still provides a reasonable initial estimate.

Since the aquifer system responds to pumping changes on a smaller time scale, the use of constant monthly pumpages to represent the actual state of stress acting on the aquifer could introduce a significant source of error in the transient computation. However, the available pumping records did not justify using constant pumping rates of duration less than a month.

5.3 Sensitivity Study of Model Parameters

Sensitivity analysis was carried out with the model to assess the effect of parameter variations on the hydraulic head distribution in the Empress Aquifer. With the set of parameters used in the calibration of the model as a basic set, one parameter was varied at a time from its base value. Model parameters such as transmissivity, storage coefficient, recharge and boundary head were examined independently.

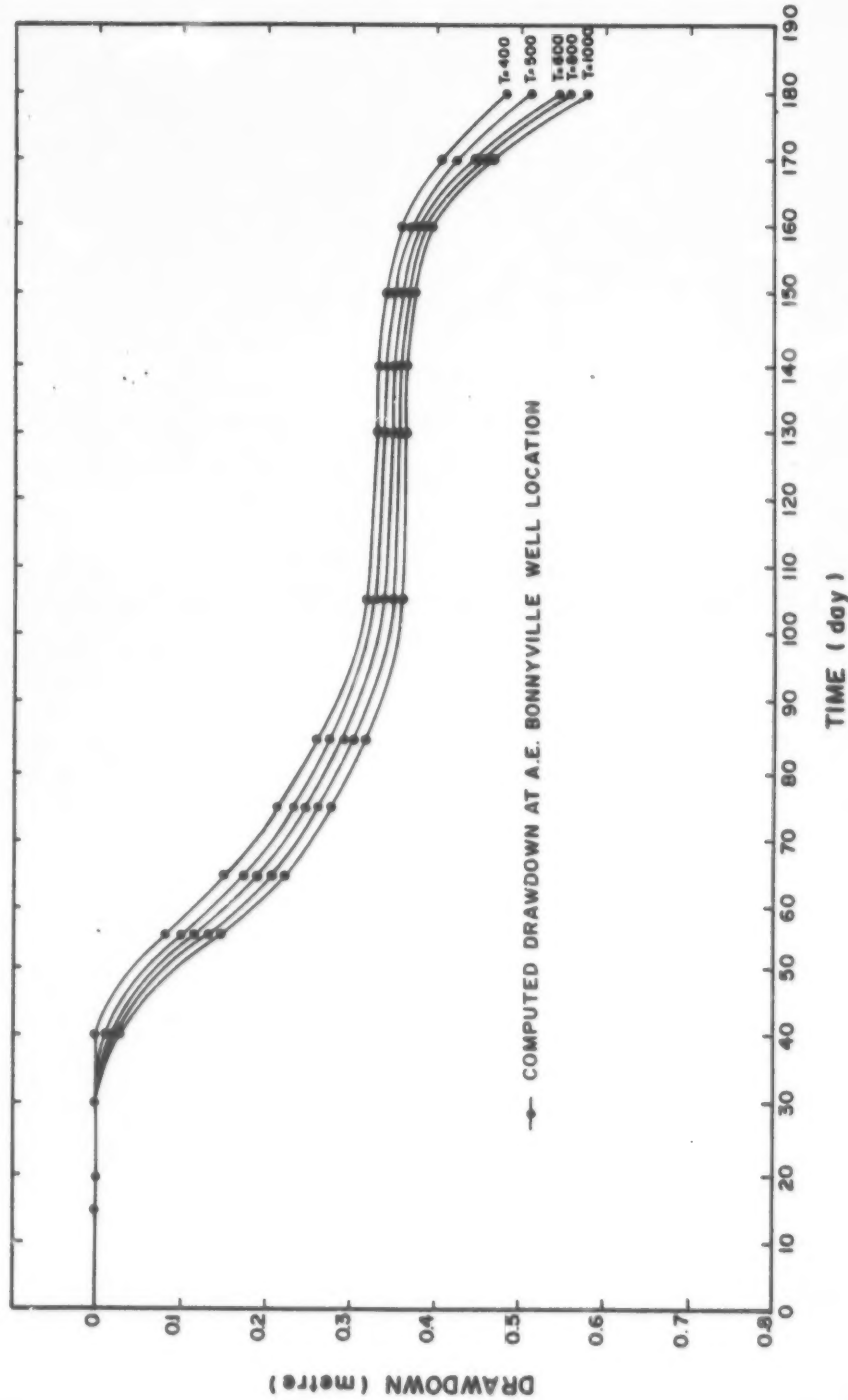
Transmissivity values, ranging between 400 m²/day and 1000 m²/day, were assigned to the Bronson Lake Channel. The corresponding transient response curves at the A.E. (Bonnyville) well

location are presented in figure 5.4. A 50 percent increase of transmissivity from 400 m²/day produced only small changes in water levels. The maximum difference in heads between the two curves was 0.027 metres for the period considered. Raising the transmissivity 2½ times to 1000 m²/day produced a maximum change in head of 0.093 metres.

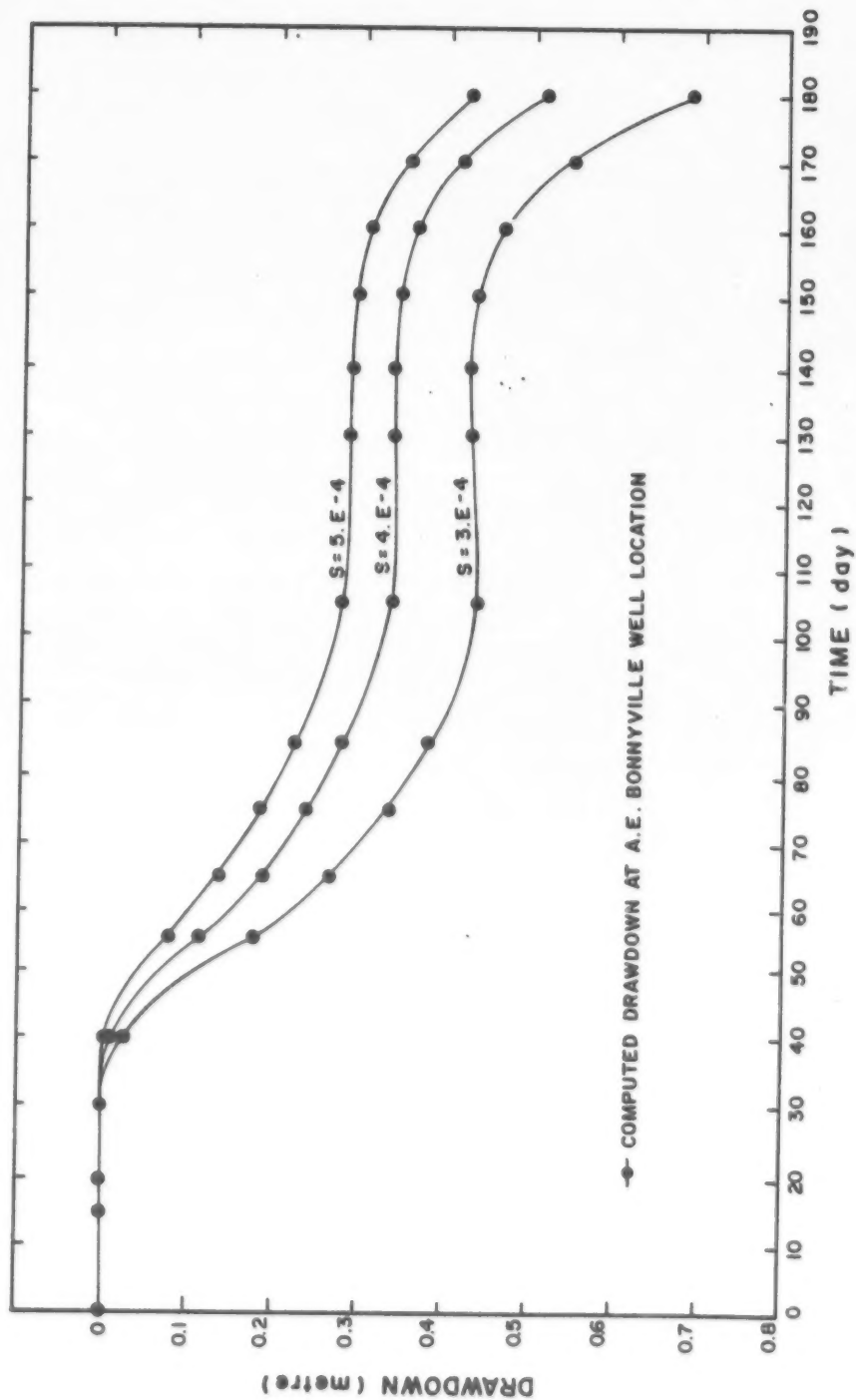
Water level response curves were computed at the location of A.E. (Bonnyville) well with different storage coefficients assigned to the Bronson Lake Channel. These results are shown in figure 5.5. For the six months considered, the aquifer response is more sensitive to changes in storage coefficient than transmissivity.

The recharge (10^{-7} m/day) was determined during model calibration. Table 5.2 shows the computed heads under steady state condition for different recharge rates. Computed heads at A.E. wells (La Corey, Bourque Lake and Norcen) were insensitive to changes in the recharge rates. On the contrary, heads computed for the Helina Channel west of the channel's confluence with the Beverly Channel were most sensitive to recharge changes. The difference in the response behaviour was due to low transmissivities and flow in this portion of the Helina Channel.

Since there was no reliable well to define the constant head boundary condition along the confluence of the channels, the inherent uncertainty of this boundary head was assessed with different boundary values. In calibrating the model, the boundary head used was 590 metres. Based on the sensitivity analysis results provided in table 5.3, water level change at each location was linearly proportional to the change in boundary head. The results also showed that the effect of boundary perturbation, as indicated by the water level changes in the aquifer domain, decreases with distance from that boundary.



GROUNDWATER MODELING STUDY - COLD LAKE BASIN	
SENSITIVITY STUDY WITH VARYING TRANSMISSIVITIES	
SUBMITTED <i>J. Macmillan</i> DATE 83-08-95	DESIGNED T. CHAU CHECKED
APPROVED <i>P. D. Macmillan</i> DATE 1993 08 05	DRAWN H.O. CHECKED
SCALE	SHEET
DATE 1993 JUNE	FIGURE No. 5.4



Abena EARTH SCIENCES DIVISION ENVIRONMENT		GROUNDWATER MODELING STUDY-COLD LAKE BASIN	
SUBMITTED <i>[Signature]</i> DATE 83-08-25		SENSITIVITY STUDY WITH VARYING STORAGE COEFFICIENTS	
APPROVED <i>[Signature]</i> DATE 1983 08 05		SCALE _____ DATE 1983 JUNE _____	SHEET _____ OF _____ FIGURE NO. 5.5
DESIGNED T. CHAU CHECKED _____ DRAWN H.Q. CHECKED _____			

TABLE 5.2
Effect of Recharge on the Steady State Hydraulic Heads
in the Empress Aquifer

Well location	Observed head (m)	Computed head (m)		
		Recharge = 0 m/day	Recharge = 10^{-7} m/day	Recharge = 10^{-6} m/day
A.E. (Bonnyville)	526.0	522.6	524.0	536.0
A.E. (Bourque Lake)	585.9	577.6	577.7	577.8
A.E. (Iron River)	554.5	564.1	565.9	582.0
A.E. (La Corey)	587.0	586.6	586.6	587.4
A.E. (Norcen)	546.2	538.6	538.6	538.6
A.E. (Rich Lake)	544.7	545.5	546.9	560.3
Amoco	533.5	558.8	560.7	578.1
Belvedere	533.0	530.2	531.5	543.1
Canada Cities	532.0	560.8	562.7	579.8
Glendon	543.5	546.5	547.6	557.4
Suncor	526.2	516.7	518.0	530.2

TABLE 5.3
Effect of Boundary Head on the
Steady State Hydraulic Heads in the Empress Aquifer

Well location	Observed head (m)	Computed Head (m)		
		Boundary head = 590 (m)	Boundary head = 595 (m)	Boundary head = 600 (m)
A.E. (Bonnyville)	526.0	524.0	524.0	524.0
A.E. (Bourque Lake)	585.9	577.7	581.5	585.4
A.E. (Iron River)	554.5	565.9	568.7	571.5
A.E. (La Corey)	587.0	586.6	591.3	596.0
A.E. (Norcen)	546.2	538.6	538.9	539.2
A.E. (Rich Lake)	544.7	546.9	548.2	549.5
Amoco	533.5	560.7	563.1	565.5
Belvedere	533.0	531.5	531.5	531.5
Canada Cities	532.0	562.7	565.3	567.8
Glendon	543.5	547.6	547.6	547.6
Suncor	526.2	518.0	518.0	518.1

TABLE 5.4
Projected Groundwater Demand for the Empress Aquifer

User	Low Demand		High Demand	
	(dam ³ /yr)	(m ³ /day)	(dam ³ /yr)	(m ³ /day)
Amoco	1160	3178.4	2320	6356.8
Canada Cities	410	1123.4	750	2055.0
Glendon	120	328.8	270	739.8
Suncor	1160	3178.4		
Belvedere	168	460.8		

5.4 Predictive Capability

Future water withdrawal by industrial, municipal and domestic users in the basin has been projected by the Planning Division. The potential groundwater users of the Empress Aquifer consist of Amoco Canada Petroleum Company Ltd., Canada Cities Services Ltd., Village of Glendon, Suncor Inc. Resources Group and Belvedere Development Ltd. The groundwater demand for these users are summarized in table 5.4.

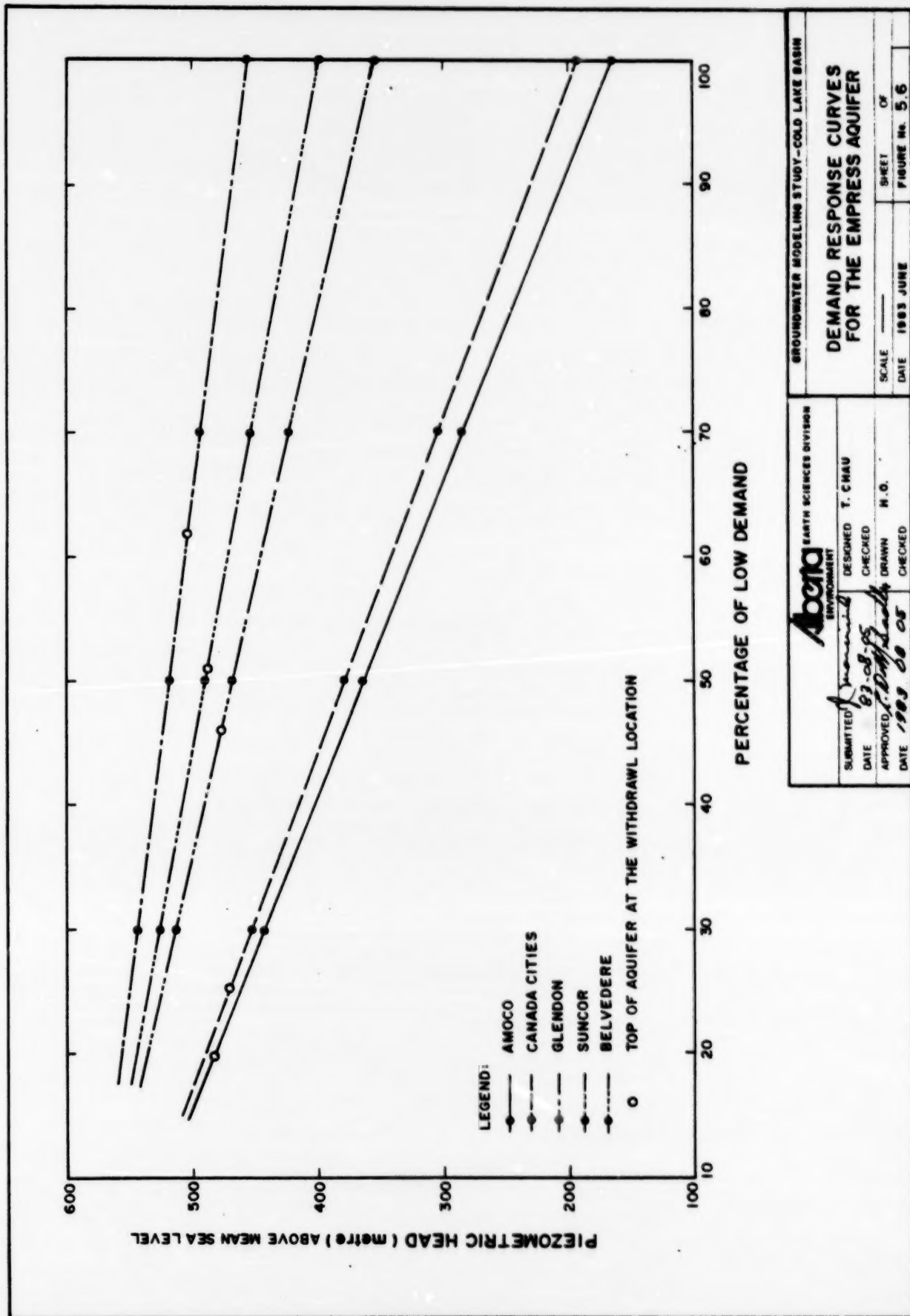
For a regional flow system, under constant pumping in time, a steady state solution would be suitable to describe the ultimate response of the aquifer system. To evaluate the effect of a particular groundwater mining scheme, the main criteria employed for this study was to maintain a confined aquifer status. Hence the piezometric head at the pumping location should always be maintained above the top of the aquifer. Simulation using both high and low demand schemes resulted in excessive drawdown at all pumping centers. To arrive at a long term feasible withdrawal scheme for potential groundwater users in the river basin, the analysis was undertaken in the following manner. Corresponding to different percentages (30%, 50%, 70% and 100%) of low demands, a steady state aquifer response was obtained for each excitation. The calculated piezometric head at each withdrawal location was compared with the top of the aquifer. If this head was above the aquifer top, the drawdown was acceptable. When the computed hydraulic head fell below the top of the aquifer, the calculated head based on the confined aquifer assumption did not necessarily represent the water table under unconfined condition. However, it did demonstrate a change of aquifer status had taken place. Consequently, the specified pumping demand at that location was not feasible. Table 5.5 presents the computed heads at the pumping centers for five different groundwater withdrawal schemes. Scheme 5 is considered a feasible pumping scheme. To identify this particular pumping scheme, results of table 5.5 for schemes one to four were plotted in figure 5.6 with head versus

TABLE 5.5

Steady State Responses To Several Pumping Schemes - Empress Aquifer

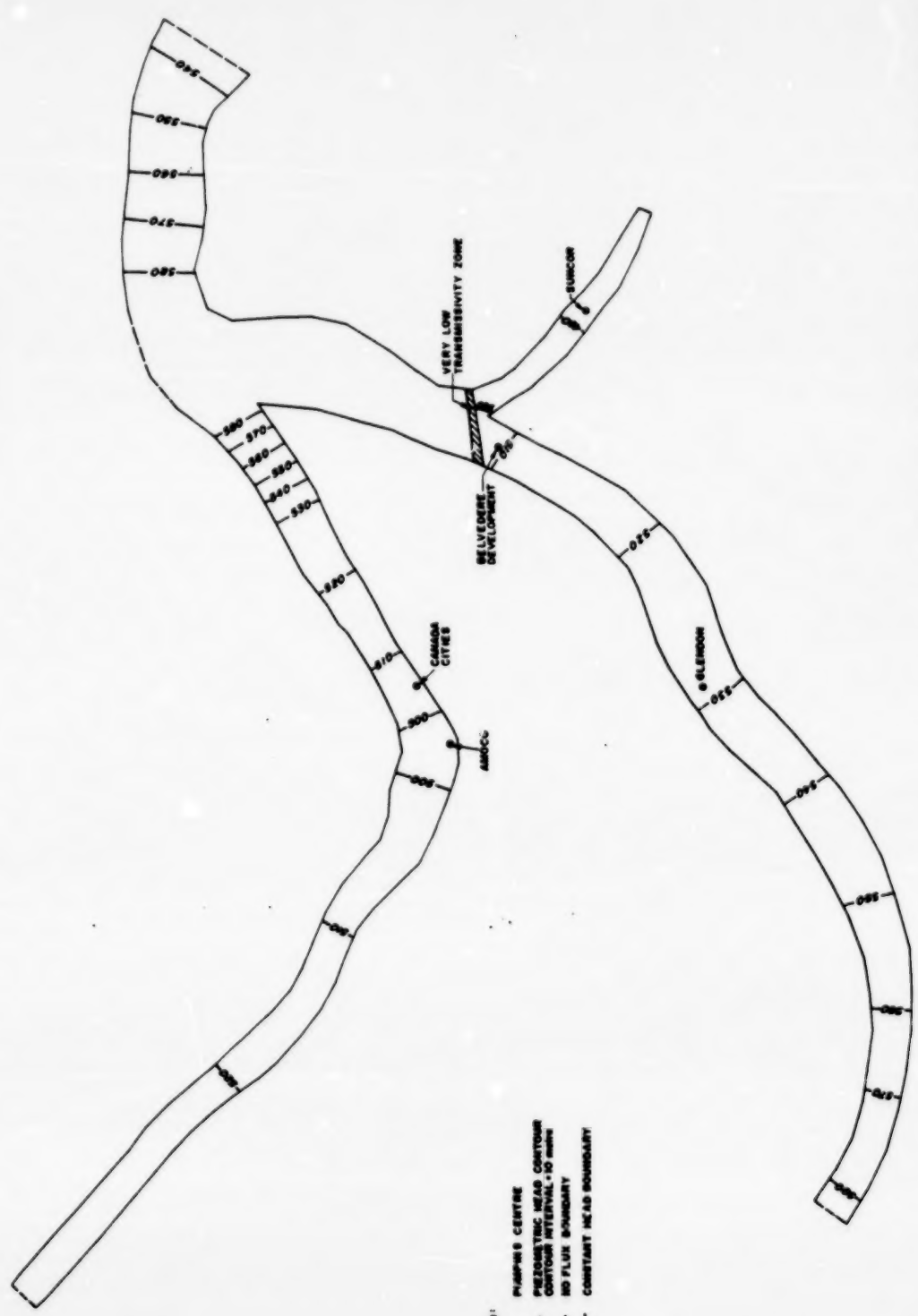
Pumping location	Aquifer top elevation (m)	Scheme 1		Scheme 2		Scheme 3		Scheme 4		Scheme 5	
		Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)
Amoco	484	3180.0	166.8	2225.1	285.0	1589.1	363.8	953.4	442.6	477.0	496.9
Canada Cities	469	1123.5	192.4	786.3	303.6	561.6	377.7	336.9	451.7	224.7	502.0
Glendon	504	328.8	453.4	230.1	491.9	164.4	517.5	98.7	543.2	164.4	528.5
Suncor	477	3180.0	354.1	2225.1	422.5	1589.1	468.0	953.1	513.6	1272.0	488.6
Belvedere	486	460.8	397.2	322.6	452.6	230.4	489.5	138.2	526.5	207.4	505.8

Scheme 1	- 100% of low demand
Scheme 2	- 70% of low demand
Scheme 3	- 50% of low demand
Scheme 4	- 30% of low demand
Scheme 5	- feasible

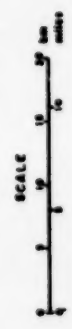


percentage of low demand. The plot shows a linear relationship between pumpage (percentage of demand) and response (hydraulic head) at all withdrawal locations, under a confined aquifer situation.

A feasible pumpage at each withdrawal center can be estimated from its linear response curve by choosing a demand that gives rise to the piezometric head above the top of the aquifer. To be conservative, the computed hydraulic head which was 10 metres above the top of the aquifer was chosen to give the pumpage for the feasible scheme. The pumpages, each expressed in terms of a percentage of the respective low demand, are 20% for Canada Cities, 15% for Amoco, 50% for Glendon, 40% for Suncor and 45% for Belvedere Development. For this scheme, the spatial head distribution is shown in figure 5.7. Comparing this figure with figure 5.2 which provides the computed steady state head distribution at the beginning of 1980, the regional flow direction differs along the Helina Channel. Groundwater withdrawals from Amoco and Canada Cities induce flow through the Helina constant head boundary. In general, a steeper hydraulic gradient exists along the Helina Channel. Detailed piezometric distributions around pumping centers for the two cases (steady state conditions resulting from the feasible withdrawal scheme and at the beginning of 1980) are presented, respectively, in figures 5.8 and 5.9. The deepening in the cones of depression is clearly illustrated for the higher withdrawal case.

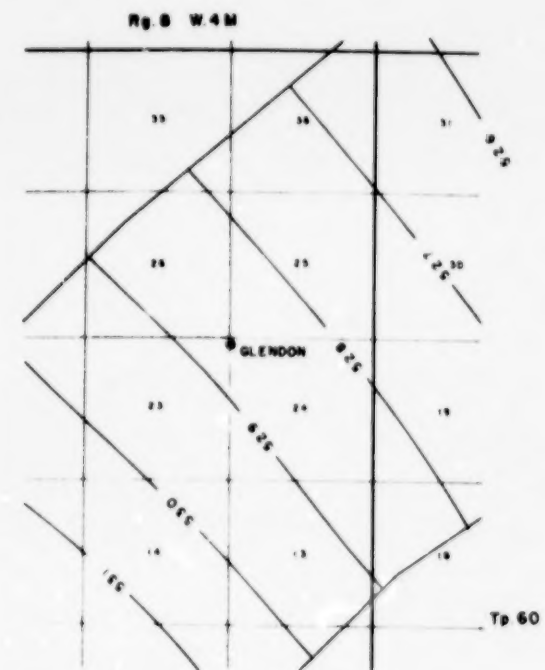
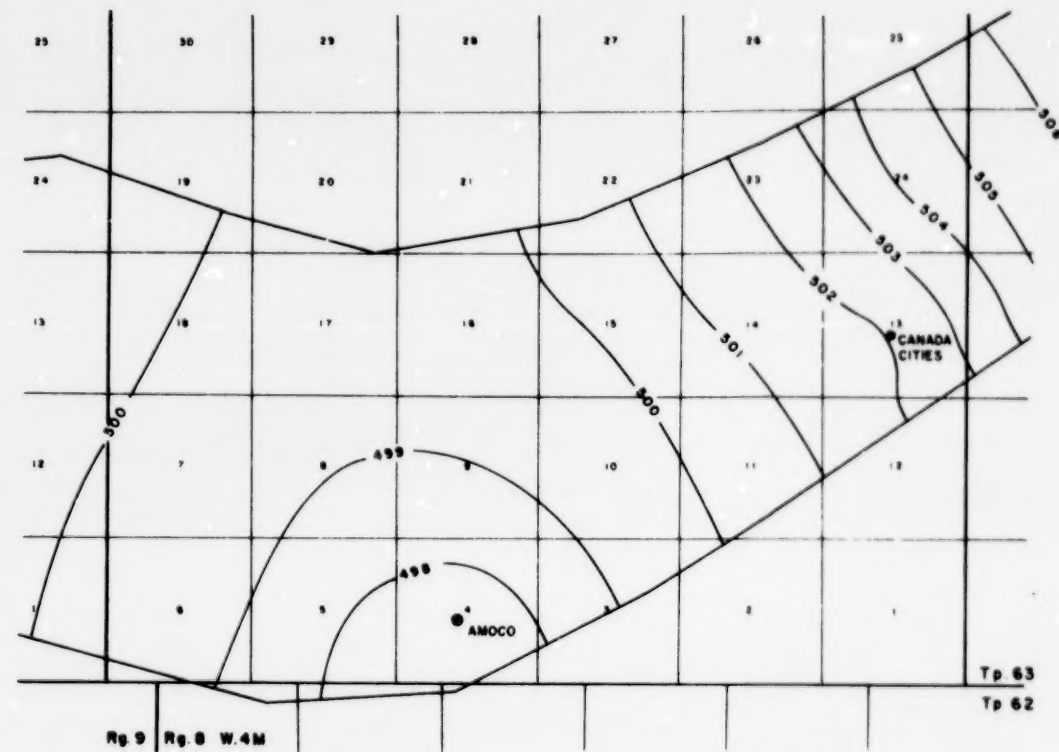


LEGEND:
 • PUMP'S CENTRE
 --- PNEUMATIC HEAD CONTOUR
 --- CONTOUR INTERVAL 10' MIN
 --- NO FLUX BOUNDARY
 --- CONSTANT HEAD BOUNDARY

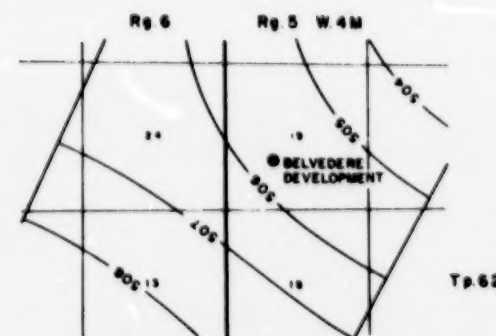
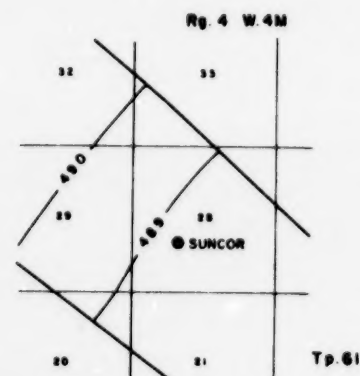


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APPROVED <i>[Signature]</i> DATE <i>01-01-01</i>		CHECKED <i>[Signature]</i> DATE <i>01-01-01</i>	
SUBMITTED <i>[Signature]</i> DATE <i>01-01-01</i>		CHECKED <i>[Signature]</i> DATE <i>01-01-01</i>	
APPROVED <i>[Signature]</i> DATE <i>01-01-01</i>		CHECKED <i>[Signature]</i> DATE <i>01-01-01</i>	

SCALE 1:10,000
 SHEET 1 OF 1
 DATE 1999 JULY
 FIGURE 1.1



SCALE

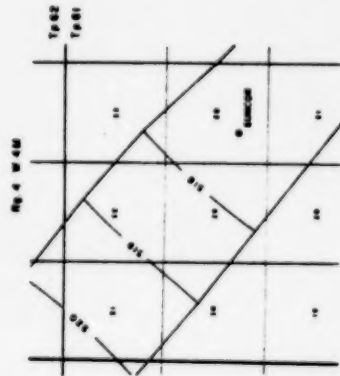
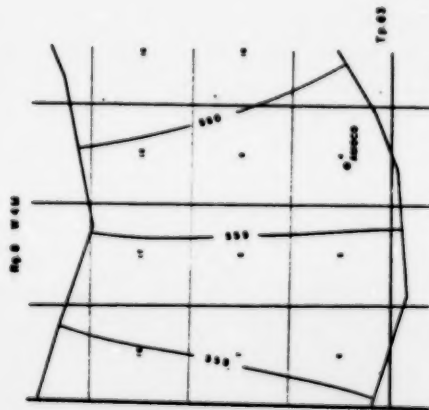


PIEZOMETRIC CONTOURS AROUND PUMPING CENTRES

LEGEND

- PUMPING CENTRE
- 330 - PIEZOMETRIC CONTOUR
- CONTOUR INTERVAL 1 metre
- AQUIFER BOUNDARY

		EARTH SCIENCES DIVISION	
SUBMITTED <i>[Signature]</i>		DESIGNED T. CHAU	
DATE 03-08-85		CHECKED	
APPROVED <i>[Signature]</i>		DRAWN <i>[Signature]</i>	
DATE 1985-08-04		CHECKED	
GROUNDWATER MODELING STUDY - COLD LAKE BASIN			
RESPONSE OF EMPRESS AQUIFER TO FEASIBLE WITHDRAWAL SCHEME			
SCALE	AS SHOWN	SHEET	OF
DATE	JULY 1983	FIGURE	No. 9.0



PIEZOMETRIC CONTOURS AROUND PUMPING CENTRES

LEGEND

- PUMPING CENTRE
- PIERZOMETRIC CONTOUR
- CONTOUR INTERVAL: 1.1 metre
- ADAPTED BOUNDARY



		GEOGRAPHICAL DIVISION SUBMITTED BY: T. CHAU DATE: 21-07-85 APPROVED: [Signature] DATE: 2002-08-05		RESPONSE OF EMPRESS AQUIFER AT THE BEGINNING OF 1980 SCALE AS SHOWN DATE JULY 1983	
		CHECKED: [Signature] DATE: 2002-08-05		SHEET 07 FIGURE 04.5.9	

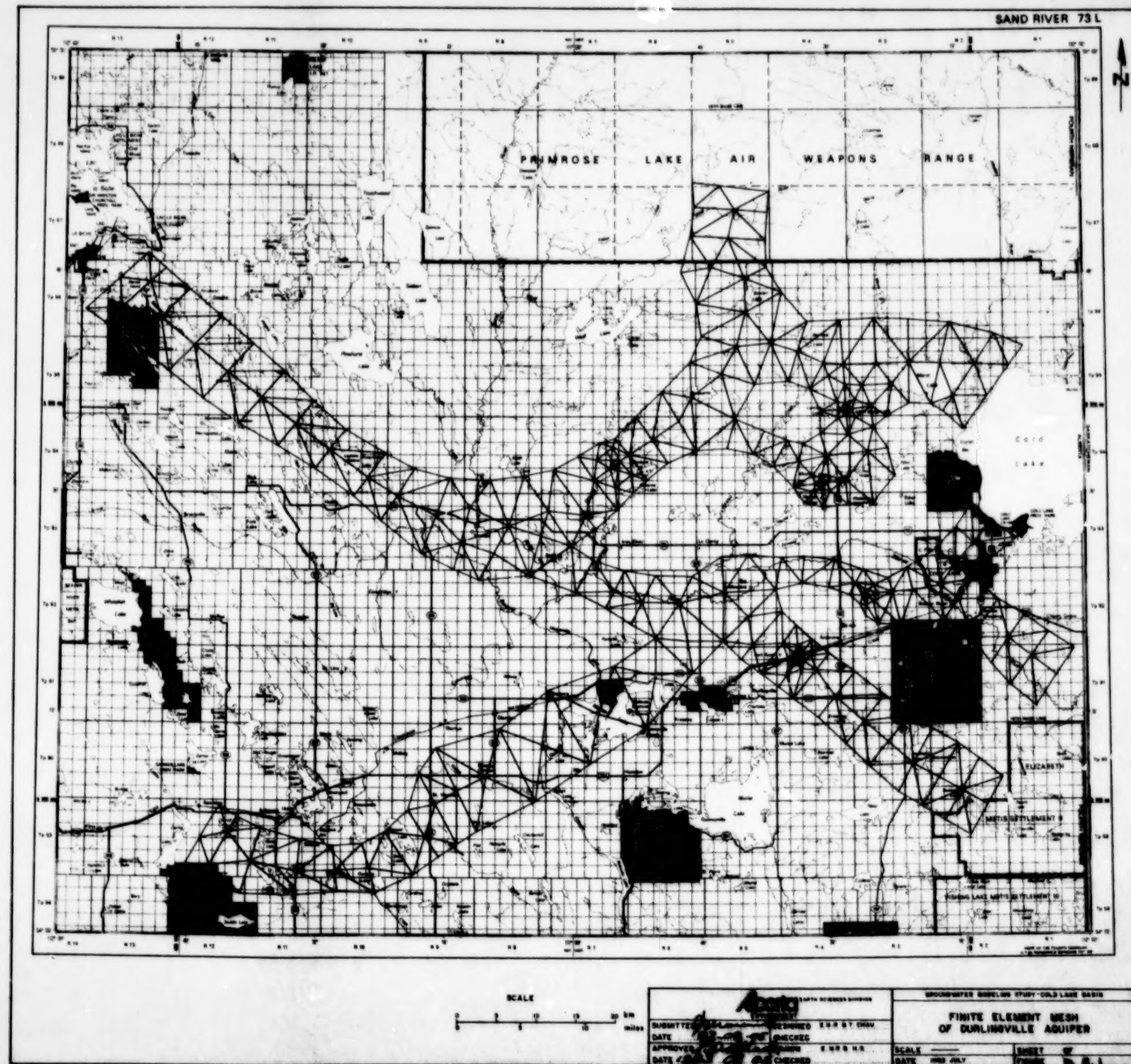
Chapter 6

APPLICATION OF MODEL TO DURLINGVILLE AQUIFER

6.1 Aquifer Description and Specification of Boundary Conditions

Figure 6.1 presents the finite element grid, representing the aquifer modelling domain. The grid consists of 444 elements and 312 nodes. The distance between the two closest nodes is about 800 metres. The potential groundwater users projected by the Planning Division include Husky Oil Operations Ltd., Municipal District of Bonnyville, Union Texas of Canada Ltd. and Mobil Oil Canada Ltd. As Mobil and Husky do not presently have any specific groundwater withdrawal locations, an artificial pumping node was assigned in each lease area. Glendon was also included as another groundwater user in the predictive analysis.

The Durlingville Aquifer has more inflow and outflow boundaries than the Empress Aquifer. The water level of Cold Lake provides the constant head boundary for the Helina Channel and the tributary linking Big Meadow Channel. Static water levels obtained at two wells provided the boundary conditions for the west end of the Beverly Channel and the east end of the Bronson Lake Channel. As for the rest of the constant head boundaries, reliable wells could not be identified. Some boundary heads were initially estimated by linearly extrapolating known piezometric heads within the aquifer domain. In other cases, the uncertain head (e.g., along Sinclair boundary) was initially estimated based on wells tapping a higher aquifer. BP Exploration Canada Ltd. and Alberta Environment had installed observation wells terminating at an aquifer above the Durlingville in the Sinclair Channel. Piezometric heads observed at these wells served as an initial estimate for the boundary head. The values were subsequently adjusted in order to simulate the observed heads in the modelling domain.



The linkage between the Helina and Beverly Channels is characterized by a zone of poorly sorted sand and till. Similarly, a zone of low permeability constitutes a linkage between the Beverly and the Big Meadow Channels. A preliminary geological evaluation indicated that there is a hydraulic connection between the Beaver River and the Durlingville Aquifer. The river partially penetrates the channel and acts as an outflow boundary of the aquifer. The process of river-aquifer interaction was simulated in the model by applying a mixed type boundary condition to the nodes denoting the river. This condition relates the difference in heads between the river and the aquifer to the flux across the river-aquifer boundary. The aquifer outflow locations along the river were not known. That reach of the river crossing the Beverly Channel was assumed to be the aquifer outflow boundary in this study. River levels for the simulation period were required for model input because the quantity of aquifer outflow was governed by the difference in heads between the river and the aquifer. Since there was no gauging station along the river-aquifer interaction reach, the 1980 mean river level (496 m) determined at a gauging station (SE 15-62-2-W4) about 35 kilometres downstream was used for the simulation period. This assumption could introduce an error in computing the quantity of aquifer outflow to the river. At the time this study was performed, a crude estimate of the upstream river level based on the steady, uniform flow assumption could not be obtained because the gradient of the river bed for that reach was unknown.

6.2 Calibration of Model

The same calibration procedure for the Empress Aquifer was applied to the Durlingville Aquifer. The steady state hydraulic head distribution computed at the beginning of 1980 is presented in figure 6.2. Flow direction is in agreement with that described in section 4.2. The estimated recharge was 2.5×10^{-6} m/day. Table 6.1 shows the comparison between observed and computed heads at different locations in

TABLE 6.1
Observed and Computed Steady State Heads
in the Durlingville Aquifer

Well Location	Observed head (m)	Computed Head (m)
A.E. (Bonnyville)	530.4	530.7
A.E. (Bourque Lake)	591.9	587.0
A.E. (Iron River)	556.3	556.1
A.E. (Norcen)	546.3	546.1
A.E. (Rich Lake)	548.7	549.1
Esso	598.8	581.8
Glendon	565.0	564.8
M.D. Bonnyville	540.8	539.8
Union Texas	533.0	532.5

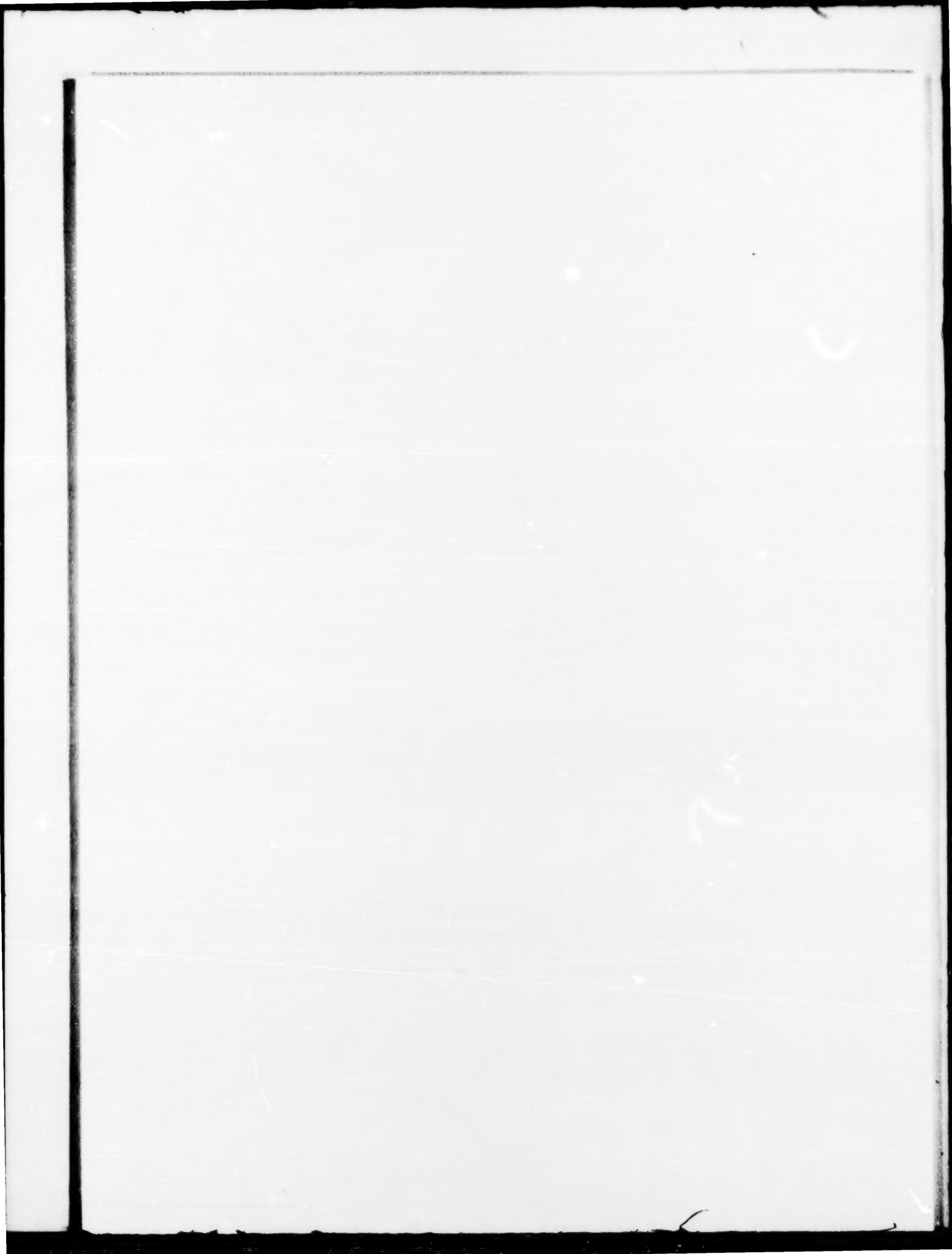
TABLE 6.2
Projected Groundwater Demand for the Durlingville Aquifer

User	Low Demand		High Demand	
	(dam ³ /yr)	(m ³ /day)	(dam ³ /yr)	(m ³ /day)
Husky	160	438.4	1220	3342.8
Mobil	580	1589.2	2180	5973.2
Union Texas	180	493.2	730	2000.2
M.D. Bonnyville	100	274.0	240	657.6
Glendon	120	328.8	270	739.8

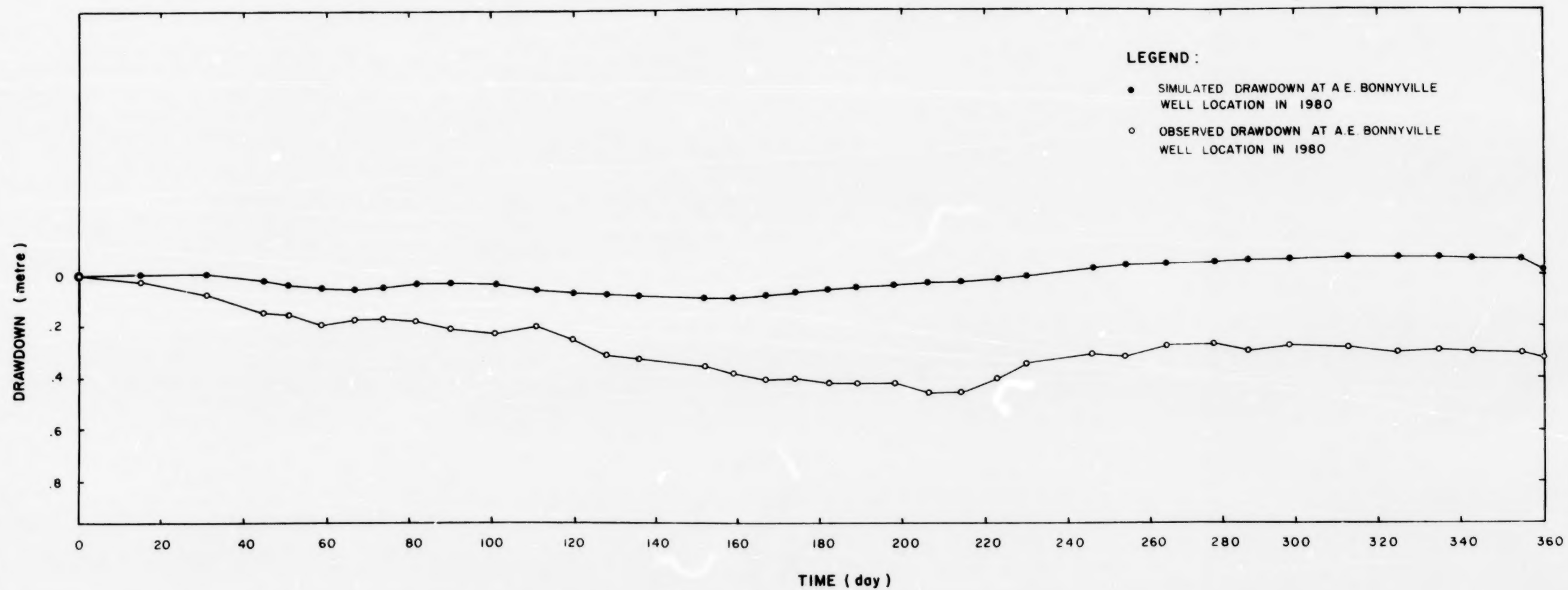
the Durlingville Aquifer. There is good agreement at most locations with the exception of the Alberta Environment well near Bourque Lake and the Esso well. This steady state hydraulic head distribution served as the initial condition for the transient simulation of the aquifer response during 1980. The computed and observed drawdowns at the A.E. (Bonnyville) well are shown in figure 6.3. Drawdowns computed with the model were less than those observed. The simplified pumping schedules for the Durlingville Aquifer, as given in figure 4.1, might not represent the actual pumpages for the simulation period. This could contribute a significant source of error in the transient simulation.

6.3 Predictive Capability

Table 6.2 summarizes high and low groundwater demands projected for the Durlingville Aquifer. The same allowable drawdown criterion used for the Empress was applied to the Durlingville. Model results indicated that the hydraulic head was above the aquifer top at all withdrawal locations for the low demand scheme. However, computed heads were below the top of the formation at Union Texas and Mobil locations for the high demand scheme. Further simulations were then carried out with various withdrawal schemes. Each scheme consists of different percentages of high demand for Mobil and Union Texas while maintaining other users at their respective high demands. Table 6.3 presents steady state aquifer response results for several groundwater withdrawal schemes. From schemes 1 to 4, hydraulic head and percentage of high demand for Mobil and Union Texas were plotted as shown in figure 6.4. A linear relationship exists between the two variables under a confined aquifer status. These demand response curves were used to decide on feasible withdrawals at the two pumping centers. The feasible withdrawal scheme consists of the following percentages of high demand: 55% for Mobil, 45% for Union Texas, 100% for Husky, 100% for M.D. Bonnyville and 100% for Glendon.



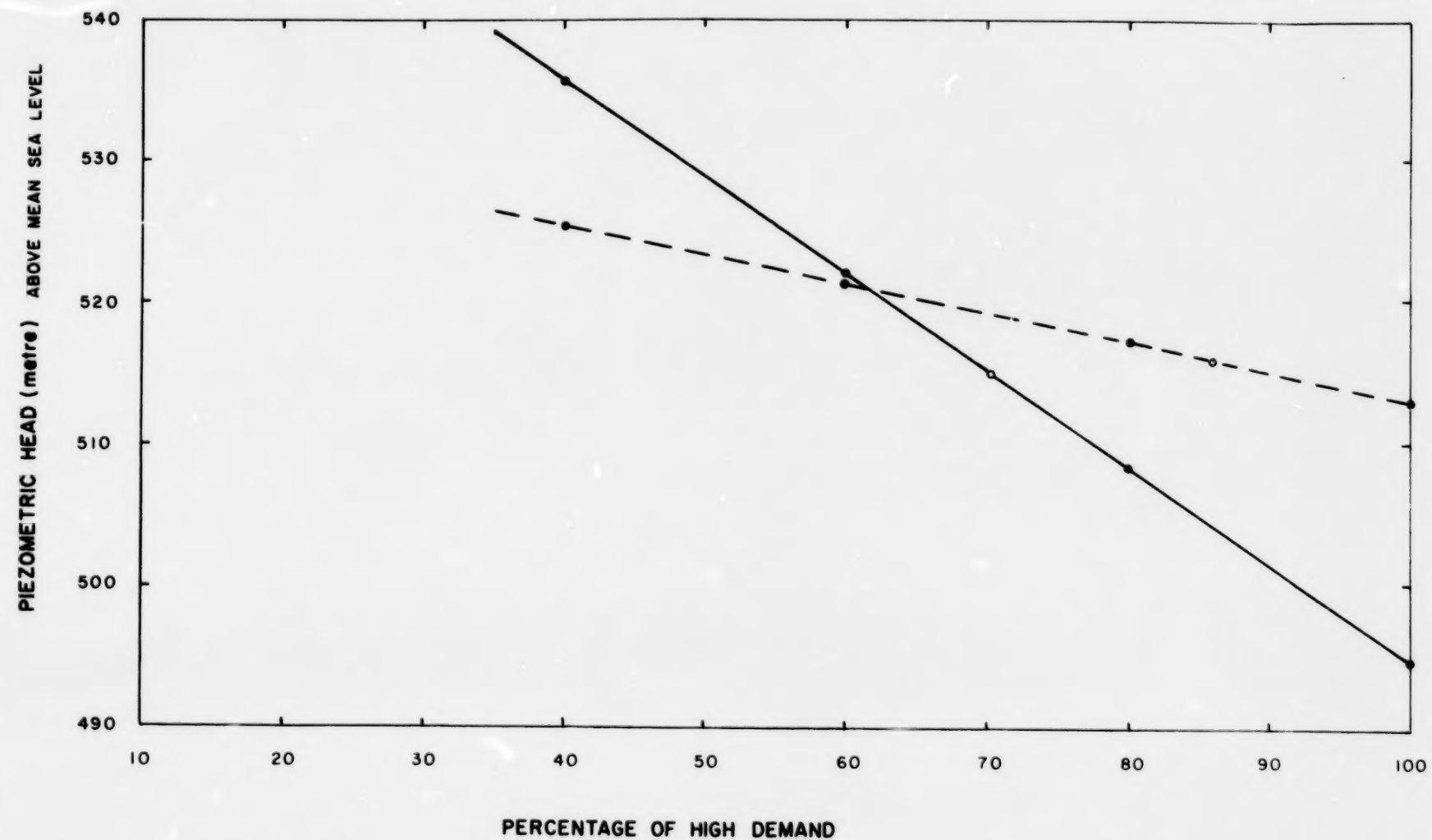
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 EARTH SCIENCES DIVISION ENVIRONMENT		GROUNDWATER MODELING STUDY-COLD LAKE BASIN	
		OBSERVED AND SIMULATED DRAWDOWNS - DURLINGVILLE AQUIFER	
SUBMITTED <i>[Signature]</i>	DESIGNED T. CHAU	SCALE —	SHEET OF
DATE 03-08-05	CHECKED	DATE JULY 1983	FIGURE No. 6.3
APPROVED <i>[Signature]</i>	DRAWN <i>[Signature]</i>		
DATE 1983 08 05	CHECKED		

TABLE 6.3
Steady State Responses To Several Pumping Schemes - Durlingville Aquifer

Pumping location	Aquifer top elevation (m)	Scheme 1		Scheme 2		Scheme 3		Scheme 4		Feasible Scheme	
		Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)	Pumping rate (m ³ /day)	Head (m)
Husky	509	3342.8	544.4	3342.8	547.0	3342.8	549.5	3342.8	552.0	3342.8	550.1
Mobil	515	5973.2	494.8	4778.6	508.5	3584.0	522.1	2389.3	535.7	3285.3	525.5
Union Texas	516	2000.2	513.0	1600.2	517.2	1200.1	521.3	800.1	525.4	900.1	524.4
M.D.											
Bonnyville	503	657.6	536.0	657.6	536.1	657.6	536.2	657.6	536.4	657.6	536.3
Glendon	533	739.8	552.5	739.8	552.5	739.8	552.6	739.8	552.6	739.8	552.6
Scheme 1		- 100% of high demand									
Scheme 2		- same as scheme 1, except 80% withdrawals at Union Texas and Mobil									
Scheme 3		- same as scheme 1, except 60% withdrawals at Union Texas and Mobil									
Scheme 4		- same as scheme 1, except 40% withdrawals at Union Texas and Mobil									
Feasible Scheme		- same as scheme 1, except 55% withdrawal at Mobil and 45% withdrawal at Union Texas									

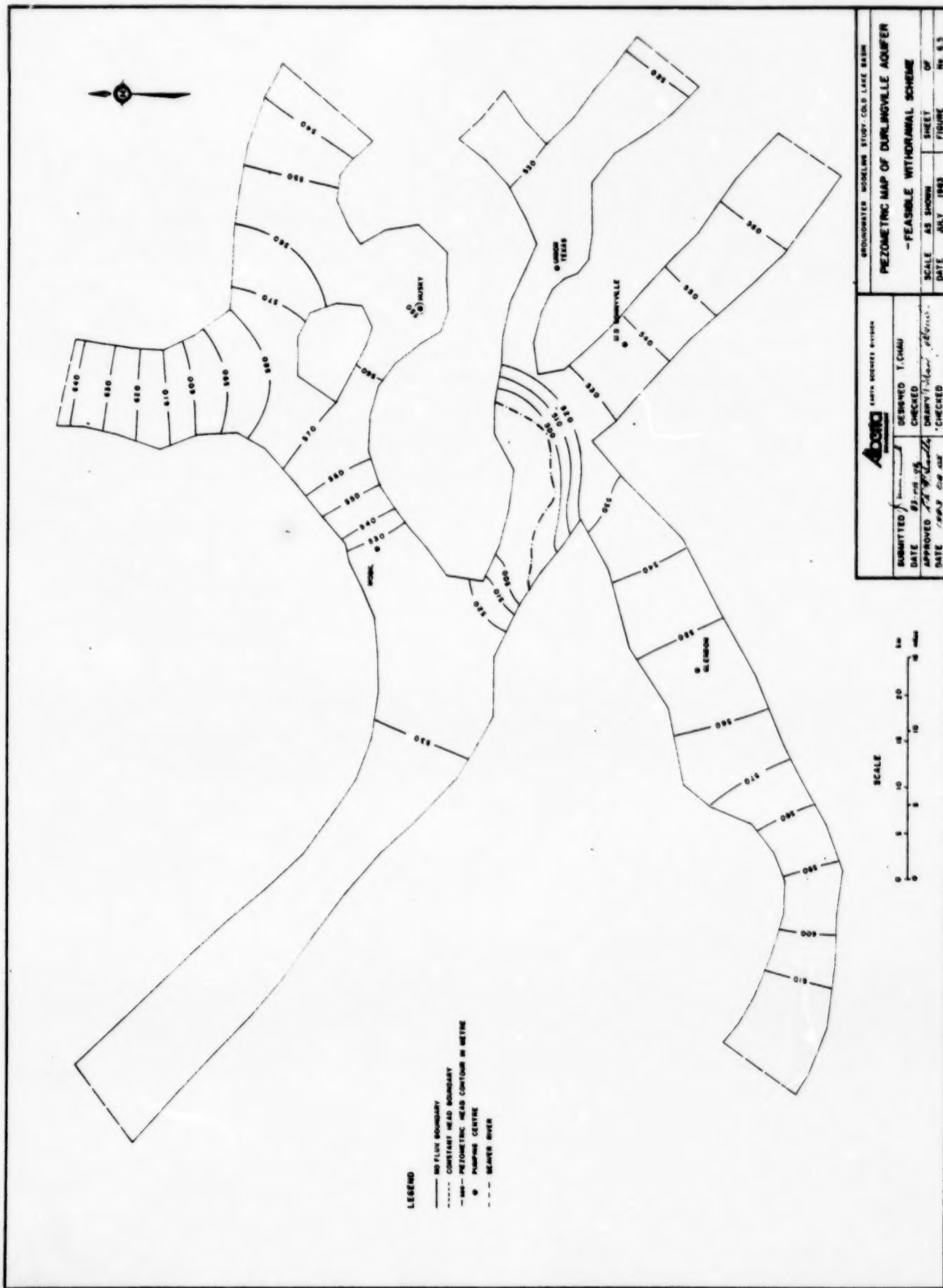


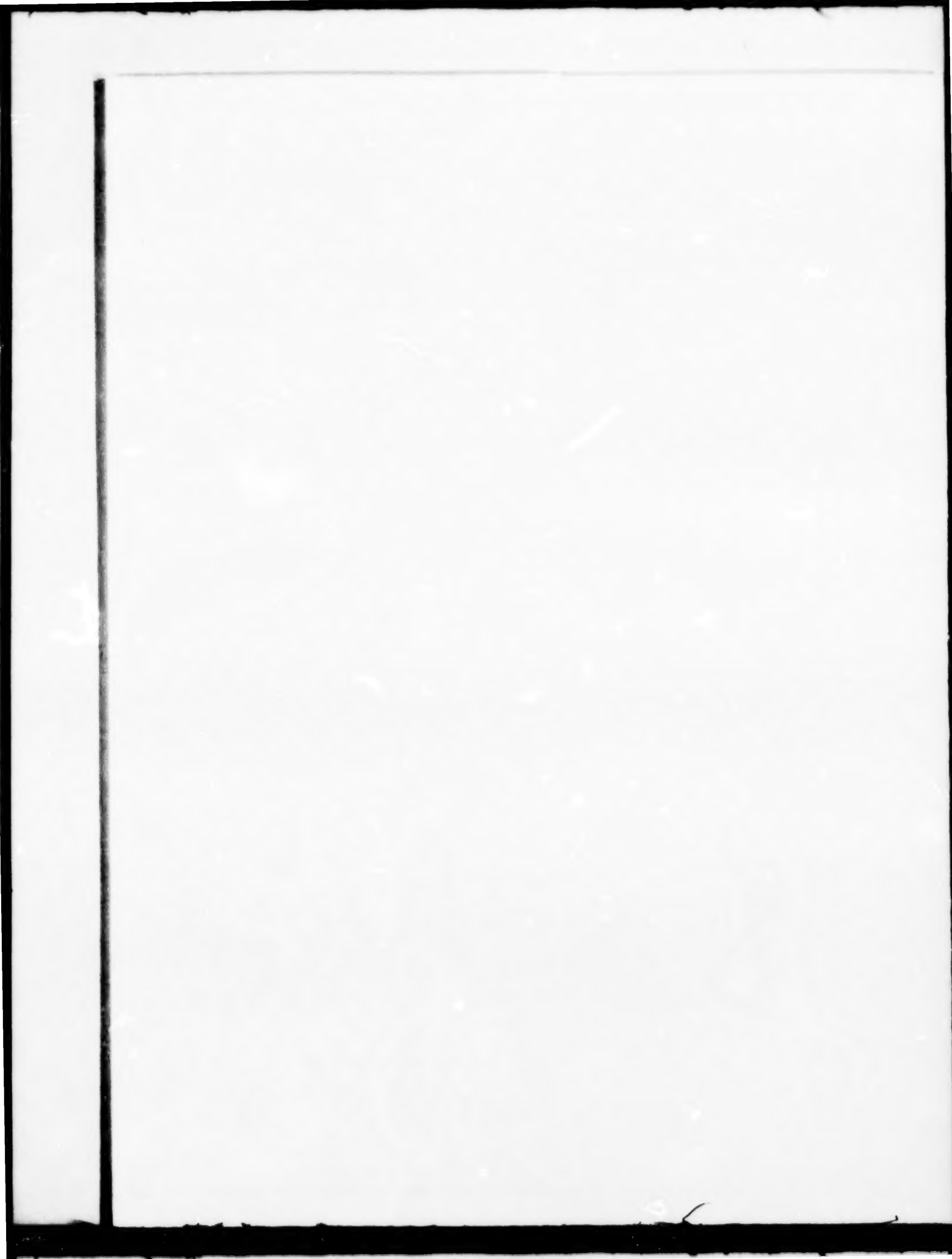
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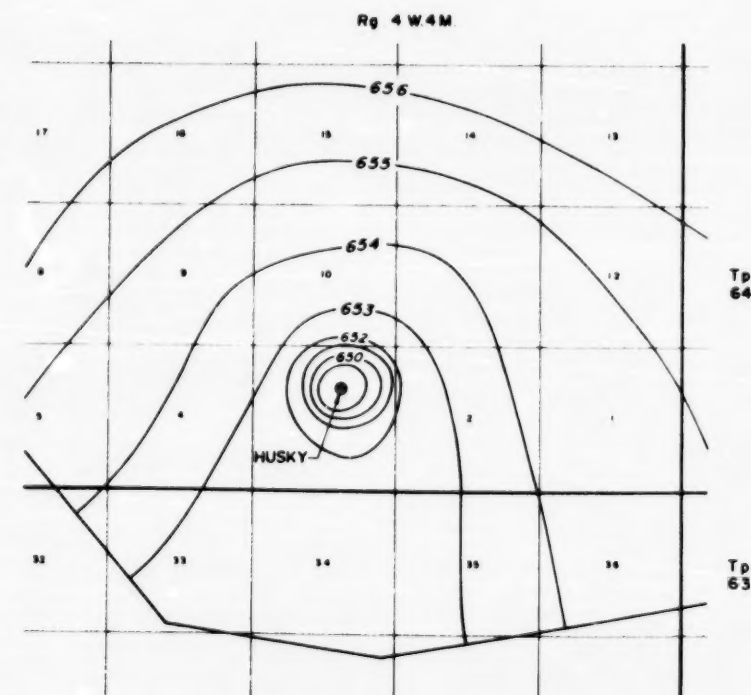
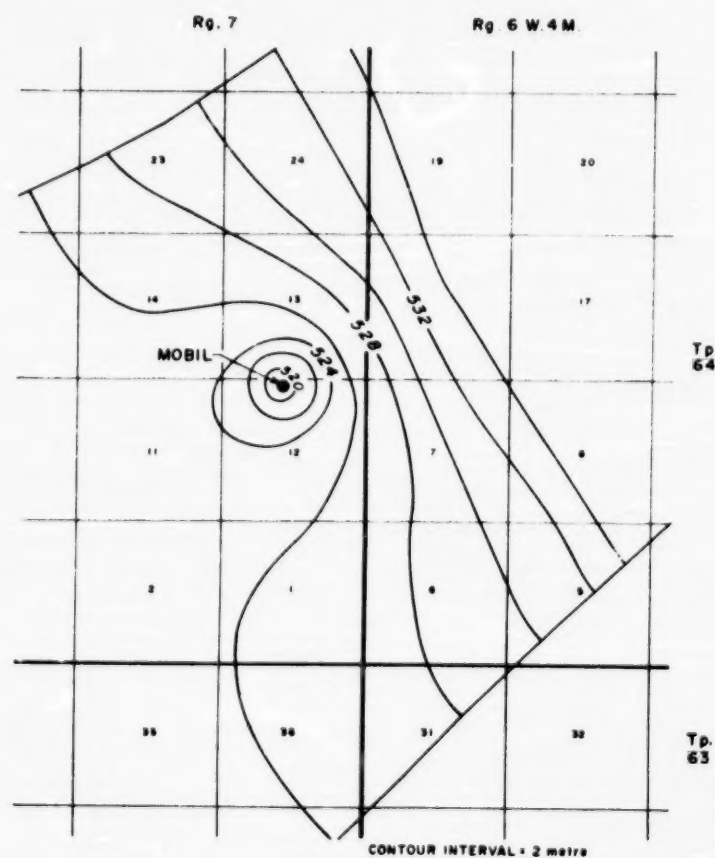
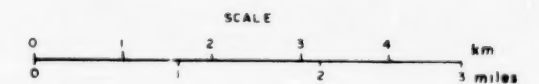
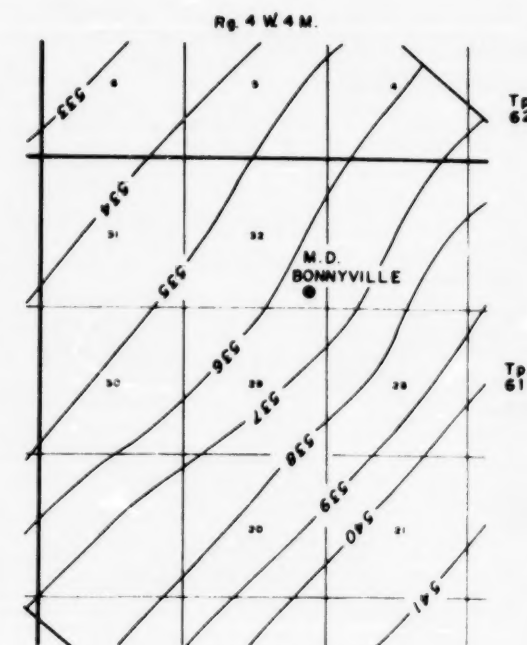
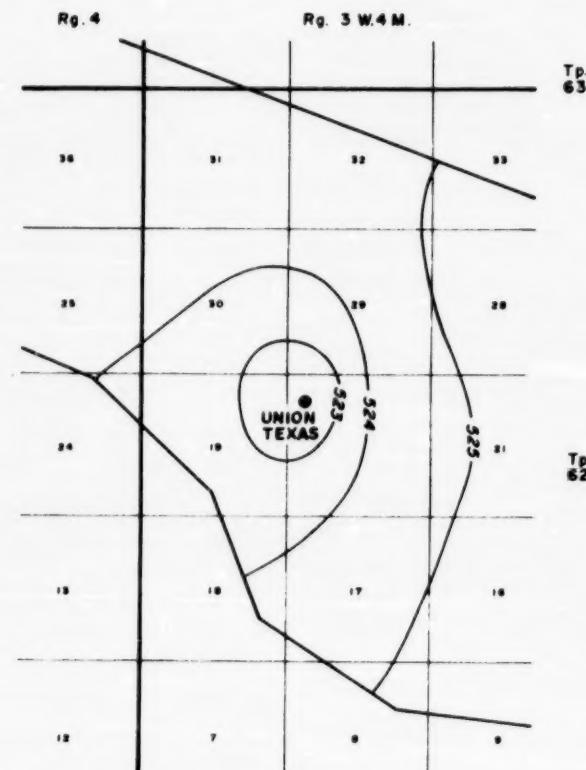
- MOBIL
- - -●- - UNION TEXAS
- TOP OF AQUIFER AT THE WITHDRAWAL LOCATION

 EARTH SCIENCES DIVISION ENVIRONMENT		GROUNDWATER MODELING STUDY-COLD LAKE BASIN	
		DEMAND RESPONSE CURVES FOR MOBIL AND UNION TEXAS	
SUBMITTED <i>[Signature]</i>	DESIGNED Y. CHAU	SCALE —	SHEET OF
DATE 83-08-05	CHECKED	DATE JULY 1983	FIGURE No. 6.4
APPROVED <i>[Signature]</i>	DRAWN <i>[Signature]</i>		
DATE 1983 08 05	CHECKED		

The steady state piezometric distribution of the aquifer under the feasible withdrawal scheme is presented in figure 6.5. The general flow pattern is similar to figure 6.2 which gives the steady state head distribution at the beginning of the simulation period. Due to higher withdrawal rates associated with the feasible scheme, a steep hydraulic gradient prevails near pumping centers and the regional flow pattern is also altered. This is clearly illustrated in figure 6.6 which shows detailed piezometric contours around pumping centers for the feasible groundwater withdrawal scheme. Comparing this figure with figure 6.7, showing hydraulic contours around pumping centers at the beginning of 1980, the change in hydraulic gradient is quite noticeable.



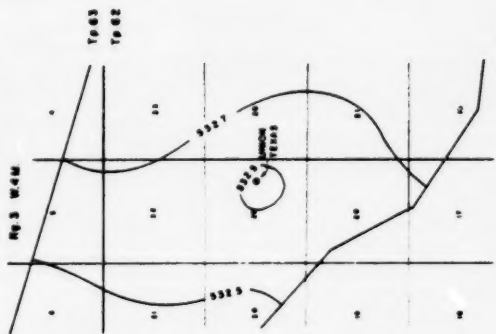
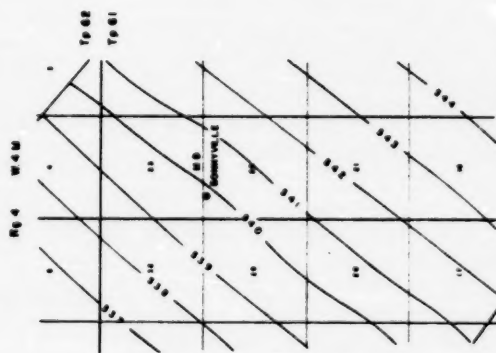




PIEZOMETRIC CONTOURS AROUND PUMPING CENTRES

- LEGEND
- PUMPING CENTRE
 - 655 — PIEZOMETRIC CONTOUR
CONTOUR INTERVAL = 1 metre UNLESS OTHERWISE STATED
 - AQUIFER BOUNDARY

		GROUNDWATER MODELING STUDY - COLD LAKE BASIN	
		RESPONSE OF DURLINGVILLE AQUIFER TO FEASIBLE WITHDRAWAL SCHEME	
SUBMITTED <i>[Signature]</i>	DESIGNED T. CHAU	SCALE AS SHOWN	SHEET OF
DATE 83-08-05	CHECKED	DATE 1983 JULY	FIGURE No. 6 6
APPROVED <i>[Signature]</i>	DRAWN H.O.		
DATE 1983 08 05	CHECKED		



PIEZOMETRIC CONTOURS AROUND PUMPING CENTRES

LEGEND:

- PUMPING CENTRE
- - - - - PIEZOMETRIC CONTOUR
- AQUIFER BOUNDARY



SUBMITTED BY: <i>[Signature]</i>		DUNSMITH MODELING STUDY - OLD LAKE BASIN	
DATE: <i>21.09.83</i>		RESPONSE OF DUNSMITH AQUIFER	
APPROVED BY: <i>[Signature]</i>		AT THE BEGINNING OF 1980	
DATE: <i>1983</i>		SCALE AS SHOWN	
DATE: <i>1983</i>		DATE: <i>JULY 1983</i>	
DATE: <i>1983</i>		SHEET OF	
DATE: <i>1983</i>		FIGURE	
DATE: <i>1983</i>		No. 6.7	

Chapter 7

CONCLUSIONS

Conclusions reached during various stages of this modelling study are summarized below:

- (1) The data collection and interpretation phase indicated that hydrogeologic information was scarce compared to the relatively large amount of geological data. The observation well network in each formation was inadequate to define the regional flow pattern. The same difficulty was encountered in determining the spatial distribution of aquifer properties.
- (2) The model was designed to simulate (approximately) the overall dynamic response of the aquifer system and was not intended to produce a particle by particle comparison for the complex hydrogeologic system. However, the magnitude of discrepancies between observed and simulated responses at certain locations in the basin indicate definite areas where additional data should be obtained to improve the understanding of the aquifer system. For example, the large difference in heads at Amoco and Canada Cities wells suggested more data is needed to identify a possible source of outflow from the Helina Channel. This data requirement was further supported by the sensitivity study, showing that it was not possible to reduce the large discrepancy simply by tuning existing parameters and boundary conditions.
- (3) Based on the existing observation well network and numerous simulations, inferences are drawn on the general flow pattern in the Empress Aquifer. Flow from the northern Sinclair Channel is divided at the junction of three channels (Sinclair, Beverly and Helina). The main component flows east through the Helina Channel into Cold Lake, while the other component flows west along the

Helina Channel until it encounters the outflow boundary discussed previously. This boundary further acts as a discharge zone for flow coming from the west of the Helina Channel. That component flowing south along the Beverly Channel terminates at the geologic discontinuity zone (north of the Belvedere Development). On the south side of this discontinuity, flow along the Beverly Channel is from the west to east towards the Bronson Lake Channel.

- (4) The parameter sensitivity study with the model illustrated the relative importance of different parameters in affecting the aquifer response. Results showed that the hydraulic head was insensitive to transmissivity changes. It was affected much more by the change in the storage coefficient. If interest is focused on the steady state behaviour of the aquifer system, as in the prediction phase, then errors in estimating aquifer properties would only slightly affect the accuracy of the output. In that case, rather than going into a costly pumping test field program for aquifer properties identification, efforts should be directed instead towards better estimation of aquifer stresses (pumpage and recharge) and specification of reliable boundary head conditions.
- (5) In the predictive phase, the Durlingville Formation was shown to have higher potential for groundwater development than the Empress Formation. The feasible groundwater withdrawal scheme for water users in each aquifer was summarized in tables 5.5 and 6.3. This assessment was made based on our present understanding of the aquifer system.
- (6) The magnitude of recharge can be an important factor in deciding the groundwater allocation potential of the aquifer. The recharge, determined during the calibration phase of the simulation, can be quite sensitive to the change in boundary conditions. If the discharge zone in the Helina Channel, within the Empress Formation,

does exists, the estimated recharge for the Empress Aquifer may increase accordingly. By the same token, the estimated recharge for the Durlingville Aquifer could be excessive because the computed outflow into the Beaver River from the aquifer was on the high side. The excessive outflow resulted primarily from a lack of information on the locations of river-aquifer interaction and the river level. This indicates the necessity of specifying such reasonable boundary conditions.

- (7) The numerical model is an useful tool to evaluate long-term safe yield of an aquifer because of its capability to include various types of boundary conditions. Comparison of safe yield results obtained by the numerical and analytical means should be made on the basis of the assumptions and criteria used to derive them.

Chapter 8

RECOMMENDATIONS

Based on the results of this study, the following recommendations are presented:

- (1) A field program is essential to identify a possible discharge location for the Empress Aquifer along that reach (63-8-W4) of the Helina Channel. Detailed geologic interpretation, supplemented with more data from a field program, is needed to identify locations of hydraulic connections between the Beaver River and the Durlingville Aquifer. This together with a river gauging station for water level measurement would provide a better means for specifying proper boundary conditions in the model to simulate the process of river-aquifer interaction.
- (2) Lack of reliable data to specify piezometric heads along inflow and outflow boundaries of the modelling domain was experienced. To identify flow direction in the Sinclair Channel, observation wells should be installed in the Empress and Durlingville Formations. Observation wells to define the piezometric boundary head in the Big Meadow and Bronson Lake Channels are essential to provide a quantitative assessment of regional flow in the Durlingville Formation. These wells would further serve to monitor the hydrologic impact on the aquifer system resulting from future groundwater mining.
- (3) In this study, the constraint in maintaining the steady state piezometric head above the top of the formation was used to arrive at feasible pumping schemes. Using a linear programming formulation, it is conceptually possible to introduce other constraints (economic or physical) and objective functions into the flow model to achieve optimal aquifer management. Future modelling effort should combine the finite element and linear programming methods to study the groundwater allocation in the basin.

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APPENDIX A

MATHEMATICAL FORMULATION OF THE NUMERICAL MODEL

A two dimensional finite element aquifer flow model (AQUIFEM), developed by Townley and Wilson (1980), was used for this study. The model is based on the Galerkin formulation and uses linear interpolation functions with triangular elements. The finite element formulation of the governing equation and the solution technique used to solve the system of matrix equations are presented in this Appendix.

A.1 Governing Equation and Boundary Conditions

The numerical model used for this investigation solves the following differential equation, describing two-dimensional, nonsteady flow in a non-homogenous anisotropic aquifer:

$$S \frac{\partial h}{\partial t} = \frac{\partial}{\partial x_i} \left(T_{ij} \frac{\partial h}{\partial x_j} \right) + Q + \frac{K'}{B'} (h_a - h) \quad i, j = 1, 2 \quad (A.1)$$

where

- S is the aquifer storage coefficient,
- h is the hydraulic head (L),
- T_{ij} is the transmissivity tensor ($L^2 T^{-1}$),
- Q is the vertical flux into the aquifer from point sources or sinks (LT^{-1}),
- K' is the vertical permeability of the semi-pervious confining layer (LT^{-1}),
- B' is the thickness of the confining layer (L),
- h_a is the hydraulic head in an adjacent aquifer separated from the main aquifer by the semi-pervious confining layer (L).

Equation (A.1) is derived under several main assumptions. Darcy's Law is considered to be valid. The assumption of essentially horizontal

flow in the aquifer is applicable. Vertical flow exists only in the semi-pervious layer overlying the aquifer and the effect of time dependent storage in this layer is not considered. Aligning the x and y axes along the principal directions of the transmissivity tensor, off diagonal terms of the transmissivity tensor are zero ($T_{12} = T_{21} = 0$, $T_{11} = T_{xx}$, $T_{22} = T_{yy}$).

Equation A.1 is to be solved in the aquifer modelling domain subject to the following three types of boundary conditions and an initial condition:

$$h = h_1(x, y, t) \quad \text{on } \Gamma_1 \quad (\text{A.2})$$

$$- (T_{xx} \frac{\partial h}{\partial x} n_x + T_{yy} \frac{\partial h}{\partial y} n_y) = Q_s(x, y, t) \quad \text{on } \Gamma_2 \quad (\text{A.3})$$

$$- (T_{xx} \frac{\partial h}{\partial x} n_x + T_{yy} \frac{\partial h}{\partial y} n_y) = \frac{K''W''}{B''} (h_3 - h) \quad \text{on } \Gamma_3 \quad (\text{A.4})$$

$$h = h_0(x, y, t_0) \quad \text{in } \Omega \quad (\text{A.5})$$

where

$\Gamma_1, \Gamma_2, \Gamma_3$ are the boundary segments of the aquifer domain Ω , on each of which the essential, natural and mixed boundary conditions are respectively specified,

h_1 is the prescribed head on Γ_1 (L),

Q_s is the prescribed lateral flux on Γ_2 (L^2T^{-1}),

n_x, n_y are the x and y components of the unit inward-pointing normal vector $\underline{n}$ on Γ_2 or Γ_3 ,

T_{xx}, T_{yy} are transmissivities in x and y directions on Γ_2 and Γ_3 (L^2T^{-1}),

K'' is the equivalent hydraulic conductivity of the leaky layer on Γ_3 (LT^{-1}),

B'' is the thickness of the leaky layer on Γ_3 (L),
 W'' is width perpendicular to the inflow on Γ_3 (L),
 h_0 is the prescribed head in the aquifer domain Ω at time t_0 (L).

A.2 The Galerkin Finite Element Formulation

The governing equation A.1 can be written in the form

$$L(h) - f = 0 \quad (A.6)$$

$L(h)$ is a linear differential operator on the piezometric head h in the aquifer domain Ω and f is the function representing known sources, sinks and leakage. This gives

$$L(h) = S \frac{\partial h}{\partial t} - \frac{\partial}{\partial x} (T_{xx} \frac{\partial h}{\partial x}) - \frac{\partial}{\partial y} (T_{yy} \frac{\partial h}{\partial y}) + \frac{K'}{B'} h \quad (A.7)$$

$$f = Q + \frac{K'}{B'} h_a \quad (A.8)$$

The aquifer domain is discretized into triangular elements with nodal points. By introducing a set of approximating functions of the form

$$\hat{h} \approx h = \sum_{j=1}^{NEQ} N_j(x,y) h_j(t) + \sum_{j=NEQ+1}^{NUMNP} N_j(x,y) g_j(t) \quad (A.9)$$

where

$N_j(x,y)$ is the j th shape function with value 1 at node j and value 0 at all other nodes, $1 \leq j \leq NUMNP$,
 $h_j(t)$ is the approximate value of h at node j at time t , $1 \leq j \leq NEQ$,
 $g_j(t)$ is the specified head at node j on Γ_1 , at time t , $(NEQ+1) \leq j \leq NUMNP$,
 $NUMNP$ is the number of nodal points,
 NEQ is the number of nodal points at which head is unknown.

Substitution of this approximating function $\hat{h}$ into equation A.6 does not satisfy exactly the equation. Instead, it gives rise to an error or residual ϵ which is given as

$$L(\hat{h}) - f = \epsilon \quad (\text{A.10})$$

Introducing weighting functions $W_i(x,y)$, $i=1, \dots, \text{NEQ}$, which equal to zero on the prescribed head boundary Γ_1 , the method of weighted residuals (Finlayson, 1972) requires

$$\int_{\Omega} (L(\hat{h}) - f) W_i d\Omega = 0 \quad (\text{A.11})$$

This is equivalent to forcing the error of the approximate differential equation A.10 to zero in an average sense. There are a number of ways to choose the weighting functions. In the Galerkin method, the shape functions $N_i(x,y)$, $i=1, \dots, \text{NEQ}$, are selected as the weighting functions. The Galerkin method of approximation to the governing equation A.1 generates the following sets of integral equations:

$$\int_{\Omega} (L(\hat{h}) - f) N_i d\Omega = 0 \quad (\text{A.12})$$

Equations A.7 and A.8 are substituted into equation A.12. The Green's theorem is then used to reduce the second order derivatives in $L(\hat{h})$ to first order and to introduce the natural boundary condition. Replacing $\hat{h}$ in equation A.12 with the approximating function of equation A.9, the following system of simultaneous ordinary differential equations in time for unknown nodal hydraulic heads are derived by Wilson, et al. (1979):

$$\underline{\dot{M}}\underline{\dot{h}} + \underline{K}\underline{\dot{h}} = \underline{\dot{f}} - (\underline{\dot{M}}^* \underline{\dot{q}} + \underline{K}^* \underline{q}) = \underline{\dot{f}}^* \quad (\text{A.13})$$

where

$$\underline{h} = \{h_j\} \quad j=1, \dots, \text{NEQ}, \text{ is a vector of unknown nodal values,}$$

$$\underline{\dot{h}} = \frac{d}{dt} (\underline{h})$$

- $\underline{q} = \{q_j\}$ $j=(NEQ+1), \dots, NUMNP$, is a vector of specified nodal values of head on Γ_1 ,
 $\dot{\underline{q}} = \frac{d}{dt} (\underline{q})$
 $\underline{\underline{M}} = \{m_{ij}\}$ $i,j=1, \dots, NEQ$, is the capacity or storage matrix representing storage effects,
 $\underline{\underline{M}}^* = \{m_{ij}^*\}$ $i=1, \dots, NEQ$ and $j=(NEQ+1), \dots, NUMNP$,
 $\underline{\underline{K}} = \{k_{ij}\}$ $i,j=1, \dots, NEQ$, is the conductivity or stiffness matrix representing transmissive effects and some effects of leakage and mixed type boundaries,
 $\underline{\underline{K}}^* = \{k_{ij}^*\}$ $i=1, \dots, NEQ$ and $j=(NEQ+1), \dots, NUMNP$,
 $\underline{f} = \{f_i\}$ $i=1, \dots, NEQ$, is the flux vector representing the effects of flux boundaries, sources/sinks, and part of the contribution of leakage and mixed type boundaries,
 $\underline{f}^* = \{f_i^*\}$ $i=1, \dots, NEQ$ is the right-hand side vector in Eq. A.13 which includes the flux vector $\underline{f}$ and the effect of prescribed head boundaries.

The individual terms of the matrices $\underline{\underline{M}}$ and $\underline{\underline{K}}$ and vector $\underline{f}$ are

$$m_{ij} = \int_{\Omega} S N_i N_j d\Omega \quad (A.13a)$$

$$\begin{aligned}
 K_{ij} = \int_{\Omega} (T_{xx} \frac{\partial N_i}{\partial x} \frac{\partial N_j}{\partial x} + T_{yy} \frac{\partial N_i}{\partial y} \frac{\partial N_j}{\partial y} + \frac{K'}{B'} N_i N_j) d\Omega \\
 + \int_{\Gamma_3} \frac{K''W''}{B''} N_i N_j d\Gamma_3 \quad (A.13b)
 \end{aligned}$$

$$f_i = \int_{\Gamma_2} N_i Q_s d\Gamma_2 + \int_{\Gamma_3} N_i \frac{K''W''}{B''} h_3 d\Gamma_3 + \int_{\Omega} N_i (Q + \frac{K'}{B'} h_a) d\Omega \quad (A.13c)$$

The single and double underlines are used to present a vector and a matrix respectively.

A.3 Solution Technique

In this study, both steady state and transient solutions of equation A.13 were sought for a confined aquifer. Under steady state condition ($\frac{\partial h}{\partial t} = 0$), a boundary valued problem arises and equation A.13 is reduced to

$$\underline{K} \underline{h} = \underline{f}^* \quad (\text{A.14})$$

which is a system of simultaneous linear algebraic equations. The Crout's algorithm, taking advantage of the banded structure of $\underline{K}$, is used to solve the unknown heads.

Under unsteady condition, an initial valued problem results. A transient, linear (transmissivity is independent of head for a confined aquifer) solution of equation A.13 is therefore required. Using a finite difference temporal scheme to approximate $\underline{h}$, the following set of algebraic equations that are solved at each time step are derived by Wilson, et al. (1979).

$$\left(\frac{1}{\alpha \Delta t} \underline{M} + \underline{K}\right) \underline{h}_{n+1} = \underline{f}_{n+1} - \underline{M}^* \underline{\dot{q}}_{n+1} - \underline{K}^* \underline{q}_{n+1} + \frac{1}{\alpha \Delta t} \underline{M} \underline{\tilde{h}}_{n+1} \quad (\text{A.15})$$

The subscript $n+1$ denotes the $n+1$ th time step. α is a weighting coefficient which determines the stability and accuracy of the solution. The algorithm described by equation A.15 is unconditionally stable for $0.5 \leq \alpha \leq 1$. $\underline{\tilde{h}}_{n+1}$, a predictor of the vector $\underline{h}_{n+1}$, is

$$\underline{\tilde{h}}_{n+1} = \underline{h}_n + (1-\alpha) \Delta t \underline{v}_n \quad (\text{A.16})$$

and $\underline{v}_n$ is defined as

$$\underline{v}_n = \frac{\underline{h}_n - \underline{\tilde{h}}_n}{\alpha \Delta t} \quad (\text{A.17})$$

Computation of $\underline{h}$ in time proceeds in the following manner. $\underline{h}_0$ is given as an initial condition at time t_0 . $\underline{v}_0$ is obtained from equation A.13 as $\underline{v}_0 = M^{-1} (\underline{f}_0 - K \underline{h}_0)$. This completes the initialization step for computing $\underline{v}_0$. Since $\underline{h}_0$, $\underline{v}_0$ are known, equation A.16 is used to compute $\underline{\tilde{h}}_1$ which is then substituted in equation A.15 to obtain $\underline{h}_1$. $\underline{v}_1$ is calculated next from equation A.17. The same computation procedure for $\underline{h}$ applies to the rest of the time steps.

The steady and transient responses of the aquifer system were obtained with the model using the numerical technique outlined in this Appendix.

APPENDIX B

FEASIBILITY OF 3-DIMENSIONAL MODELLING

The geological description of the study area, presented in Chapter 2, suggests that a multilayered or a three dimensional (3-D) flow model provides the best means of representing complicated multilayered, surficial aquifer systems. Although much progress and continuing research has been made in the theoretical development of 3-D or multilayered models, many of these models have only been verified with analytical solutions and small hypothetical systems (Javandel and Witherspoon, 1968; Freeze, 1971). Very few have been applied sufficiently to real basins under actual field conditions. The reason is that, with most 3-D simulations of real basins, there is often insufficient hydrogeologic information available for model input and calibration.

Gupta and Tanji (1976) applied a 3-D finite element flow model using isoparametric elements to the analysis of flow through multiaquifers in Sutter Basin, California. In that study, the model was directed towards steady state application. Hence no consideration was given to include a movable water table boundary in the formulation, a condition necessary for the simulation of the dynamic response in a natural basin. Frind and Verge (1978) applied a transient, saturated-unsaturated finite element model to study the 3-D flow in a hypothetical system and a real basin. This transient model requires the specification of the groundwater surface as the atmospheric boundary condition. It is difficult to obtain enough accurate data for a real basin to provide this top boundary condition for model input. Another aspect that needs to be considered for a 3-D basin simulation is the computing cost which is much higher than the comparable 2-D simulation. The high computing cost as a limiting factor in 3-D modelling was pointed out by Appel (1975) in his review on groundwater models. Lastly, the

manpower involved in data interpretation, grid design and preparation of input data is another economic factor to be considered. The practicality of 3-D aquifer modelling for the study area was evaluated prior to the study. Review of the presently available hydrogeologic data showed that the data base was inadequate to define the 3-D system and to calibrate the model. A costly aquifer identification field program will be required to serve this purpose. Within the economic and time constraints imposed on this study, it was decided to proceed with 2-D modelling of the Empress and Durlingville Aquifers, treating the aquifers independently of each other. A two dimensional finite element aquifer flow model was used to simulate horizontal groundwater flow in each aquifer under the assumption that vertical flow was negligible compared to the horizontal component.

MAPS NOT

FILMED

CARTES

OMISES

POOR

QUALITY

ORIGINAL